# Methods for Predicting Truck Speed Loss on Grades 

Final Report
Contract Number DTFH61-83-C-00046

## Thomas D. Gillespie

November 1985

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| 1. Report No. | 2. Govommen Accostion No. | 3. Rocipiomi's Cotalog No. |
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| METHODS FOR PREDICTING TRUCK SPEED LOSS ON GRADES -Final Technical Report |  | 5. Report Doie $\begin{gathered}\text { November } 1985 \\ \text { Nover }\end{gathered}$ |
|  |  | 6. Pastorming Organi zation Code |
|  |  | Orgomi zation Report |
| 7. Authors) Thomas D. Gillespie |  | UMTRI-85-39/2 |
| 9. Performing Orgenizetion Name and Address Transportation Research Institute The University of Michigan 2901 Baxter Road Ann Arbor, Michigan 48109 |  | 10. Work Unit No. |
|  |  | 11. Conntract or Grant No. DTFH61-83-C-00046 |
|  |  | 13. Typo of Report and Period Covered |
| 12. Sponsoring Agency Neme and Address <br> Federal Highway Administration <br> U.S. Department of Transportation Washington, D.C. 20590 |  | Final |
|  |  | 7/83-11/85 |
|  |  | 14. Sponsoring Agency Code |
| 15. Supplemantery Notes |  |  |
| 16. Abstract <br> Truck speed loss on grades reduces highway capacity and increases the risk of accidents. The rational design of a truck climbing lane as a solution to this problem requires means for predicting truck speed changes on grades. <br> Experimental measurements of the speed loss of trucks operating on highways were conducted at 20 sites throughout the country. These data were analyzed to compare performance to present guidelines for highway design embodied in the AASHTO Policy on Geometric Design of Highways and Streets. The performance of the straight truck and tractor-trailer population is notably better than that reflected in the AASHTO publication. <br> Methods were developed for modeling the hill-climbing performance of the four major truck classes at the 12.5 and 50 percentile population level using empirically determined weight-topower values. Speed-distance plots are provided for each class on constant grades, along with a simple computer program for calculating speed versus distance on arbitrary grades defined by the user. These speed-loss models are recommended as alternatives to the AASHTO standard for highways carrying primarily straight trucks and tractor-trailers. <br> Trucks pulling trailers, and doubles and triples are the truck classes with lowest hill-climbing performance. For the limited data obtained, the AASHTO model appears to provide a reasonable performance prediction for the 12.5 percentile population. <br> Methods for estimating performance at the 12.5 percentile level for mixed truck populations are presented. The need of a rationale for making design decisions with mixed truck populations is recognized, and suggested as a future research topic. |  |  |
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| 17. Kay Words <br> climbing lanes, speed weight-to-power ratio acceleration performa | on grades18. Distribution <br> No restri <br> available <br>  <br> National <br> Springfie | 18. Distribution Statement <br> No restrictions. This document is available to the public through the National Technical Information Service, Springfield, Virginia 22161 |  |
| :---: | :---: | :---: | :---: |
| 19. Socurity Cleasif. (of this repert) NONE | 20. Security Clessif. (of this poge) NONE | $\begin{gathered} \text { 21. No. of Poges } \\ 169 \end{gathered}$ | 22. Price |

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Background

This document is the final report for the FHWA study, "Truck Tractive Power Criteria," Contract Number DTFH61-83-C-00046, performed over the period July 1983 to October 1985. The study focuses on the problem of predicting the speed loss of trucks encountering grades on our nation's highways.

For purposes of this project, the term "truck" refers to any combination of single- or multi-unit vehicles having at least one axle with dual wheels. Vehicles of this type normally have a gross vehicle weight rating (GVW) of $10,000 \mathrm{lb}$ or more, and are thus separated from the much larger population of light trucks (pickups), which are similar in hill-climbing performance to passenger cars. The trucks considered in the project then range from the smaller 2-axle straight trucks with GVW ratings over $10,000 \mathrm{lb}$, to tractor-semitrailers, and doubles or triples combinations with GVW ratings to the maximum allowable on the highways.

Trucks characteristically exhibit the lowest level of hillclimbing performance of all vehicles using the nation's highways. Thus, at uphill grades of sufficient length and steepness their speed loss may be great enough that they impede the traffic flow, reducing the capacity of the highway to carry traffic, and creating possible hazards to other vehicles. To counteract these influences, climbing lanes may be added along the uphill grade section. The additional construction and maintenance costs, however, warrant careful consideration with regard to when climbing lanes are needed, and over what portion of the grade.

To aid highway designers in making decisions on this and other matters, the American Association of State Highway and Transportation Officials (AASHTO) publishes a Policy on Geometric Design of Highways and Streets. ${ }^{(1)}$ The Policy addresses the issue of truck uphill
performance and the need for climbing lanes. In brief, a truck's weight-to-power (W/P) ratio is considered to be the most important characteristic affecting hill-climbing performance, with a value of 300 $\mathrm{lb} / \mathrm{hp}$ taken as the representative $\mathrm{W} / \mathrm{P}$ value for design purposes. Plots of speed versus distance on constant grades are presented for a typical truck of $300 \mathrm{lb} / \mathrm{hp}$ as a tool for the highway engineer to estimate truck speed losses on a proposed design. Studies are referenced that indicate that truck accident frequency increases with differential in speed, thus climbing lanes are advantageous when excessive speed differentials are anticipated. A speed difference of $10 \mathrm{mi} / \mathrm{h}(16 \mathrm{~km} / \mathrm{h})$ is suggested as a limit at which point a given grade is of the "critical" length justifying consideration for a climbing lane.

The decision to add a climbing lane carries with it an economic penalty, and in many cases complicates the overall design. For determination of where on the grade the climbing lane must start, the characterization of truck performance is very critical. The basis for characterizing truck performance by $a \mathrm{~W} / \mathrm{P}$ of $300 \mathrm{lb} / \mathrm{hp}$ derives from a number of past studies ranging in time from 1945 to 1978. $(2,3,4,5,6)$ Other and more recent data on truck performance is available. $(7,8,9,10,11,12)$ Yet, there is need for a more comprehensive study examining truck hill-climbing performance in a more general wayconsidering the possible differences in geography, road type, and, particularly, the temporal changes in truck properties.

Objectives

This study addressed the broad issue of how truck hill-climbing performance could be best characterized, and what methods could or should be applied by the highway engineer to quantitatively estimate truck speed losses for a particular design. The individual objectives may be stated as follows:

1) To determine how to model or characterize hill-climbing performance in a way that is most useful for the highway design process.
2) To determine the primary variables affecting hill-climbing performance that may be specific to a site (i.e., truck class, grade, speed, road classification, and location).
3) To develop guidelines and/or procedures for the highway engineer that can be used to quantitatively estimate hill-climbing performance of the general truck population at a site, taking into account the above variables.

Methods

As reflected in the AASHTO's Policy on Geometric Design of Highways and Streets, weight-to-power ratio has been adopted as the means of characterizing trucks for their hill-climbing performance. (1) Other representations are possible. Which is best depends on the performance measure to be predicted and the ease with which it can be applied.

In order to determine means for predicting hill-climbing performance, an experimental data base of measurements of actual trucks on the nation's highways is needed. Furthermore, the experimental data must be collected over a broad range of conditions and geographic locations, so that the significant variables affecting performance can be extracted. Thus, the foundation of the research program was a program of data collection in the field, by which to examine hillclimbing performance of present-day trucks. Based on economic and other factors, a program of field tests at 20 sites throughout the country was conducted. In those tests, the hill-climbing performance of a sample of trucks was determined, along with descriptions of the vehicles making up the population of vehicles using the road.

This data base was analyzed to determine the averages and distributions of performance properties for the trucks at each site. By selecting sites with appropriate representation of geographic location and road class, differences in performance attributable to these
variables could be determined. Within each site, the classification by vehicle allowed inquiry into differences between classes of vehicles.

At the same time, the overall measures of hill-climbing performance allowed examination of the typical behavior over a large sample of vehicles, so that past assumptions as to how trucks decelerate on a grade could be critically tested.

## Report Organization

Chapter 2 of this report provides a background on how hillclimbing performance can be characterized. Certain key issues are identified which establish a direction in evaluating the results observed in the experimental measurements of hill-climbing performance obtained in this study. In chapter 3 the performance capabilities of modern trucks are examined, using the data base of experimental measurements. The relationships between performance and truck type on different road classes are examined to identify which variables should be considered by the highway engineer in attempting to predict speed loss in a design analysis. Chapter 4 presents the application of the information in the form of suggested means for predicting hill-climbing performance for highway design purposes. In Chapter 5, the overall findings from the project are summarized in the form of conclusions and recommendations. The appendices provide background information on the methods employed to collect data in the field, and summaries of the data that were collected.

## CHARACTERIZATION OF HILL-CLIMBING PERFORMANCE

Mechanics of Truck Accelerations

Choosing a "best" means to quantify hill-climbing performance must start with a basic understanding of the mechanics involved. The ability for a truck to accelerate on the road depends on the summation of the forces acting on the vehicle. The propulsive effort (drive force) is derived from the engine. This acts to overcome the drag forces due to aerodynamic and rolling resistance at the particular speed of travel. Any reserve in drive force available from the engine may be used either to accelerate the vehicle or to overcome the drag arising from road grade. When encountering a grade greater than the available drive force, the deficiency is made up by a deceleration of the vehicle.

Governing Equations. The governing equation for the forward travel of any motor vehicle when it encounters a grade is determined by the summation of forces on the vehicle in the longitudinal direction. The equational form is:

$$
\begin{equation*}
W(1+e) A_{x}=F_{d}-F_{r}-F_{a}-W G_{r} \tag{1}
\end{equation*}
$$

where

```
\(\mathrm{W}=\) the vehicle gross weight
\(e=e f f e c t i v e\) weight of all rotating components normalized by W
\(A_{x}=\) the instantaneous acceleration in g's
\(F_{d}=\) engine drive force at the ground
\(F_{r}=\) rolling resistance force
\(F_{a}=\) aerodynamic drag force
\(G_{r}=\) road grade (expressed in radians or percent/100)
```

At high speeds, the effective weight of the rotating components is small (on the order of a few percent of the gross vehicle weight). At speeds below $20 \mathrm{mi} / \mathrm{h}(32 \mathrm{~km} / \mathrm{h})$ it may increase to a significant fraction of the gross weight, but to simplify the discussion at this point it will be neglected. Then this equation can be written in an alternate form in which all terms are normalized by the weight:

$$
\begin{equation*}
A_{x}+G_{r}=F_{d} / W-\left(F_{r}+F_{a}\right) / W \tag{2}
\end{equation*}
$$

This equation accounts for the instantaneous acceleration of the vehicle on the grade. The right side of the equation represents the normalized drive force, less the normalized drag forces. At any instant in time the acceleration (in g's) plus the grade must equal this total force. When the grade is large, the acceleration must be small (or even negative) in order for the equation to be satisfied.

In order to use the equation to predict velocity as a function of time, the equation is integrated over the desired interval beginning from a set of initial conditions (an entry velocity at the grade entry point). In general the forces will be a function of velocity and the grade may be a function of distance traveled. Reduction to a closedform analytical expression is difficult due to the complexity of the expressions for the forces acting on the vehicle, and due to the influence of transmission shifts on speed maintenance. (Closed-form solutions have been obtained for some of the simpler forms of the equation. For example, in vehicle coastdown tests the engine power term is zero and transmission shifting does not occur. (13) ) However, the equation can be solved readily on a small desktop computer, or approximate solutions can be performed on a calculator.

Forces Acting on a Vehicle. The exact solution obtained in any particular case is dependent on the expressions and values used to describe the various forces acting on the vehicle. Figure 1 shows the nature of the various forces acting on the vehicle as a function of speed.


Figure 1. Forces acting on a vehicle as a function of speed.

Drive force-The power available from the engine represents an absolute upper bound on the drive force as a function of speed. Power is force times velocity, hence the power limit of the engine plots as a hyperbola in the figure. In actuality, only a portion of that power is available because of the inefficiency of the drive train, the efficiency factor lowering the level of the hyperbola. Maximum power is available from the engine only at a specific engine speed. To allow the engine to operate near this limit, various gear ratios are provided in the transmission. Within each gear the drive force available is then simply the image of the engine torque curve. Acceleration (or deceleration) over a wide speed range will require that the transmission be shifted from one gear ratio to the next. The majority of heavy trucks have manual transmissions. When the shift is made, the engine power is disengaged from the drive train for the shift interval. Typical time intervals of 1 to 2 seconds are assumed for shifting.

Rolling resistance-The drag force arising from the tires is generally accepted to consist of a constant value, plus a smaller component that increases linearly with speed. The absolute magnitude of the rolling resistance is directly proportional to the load carried; hence, rolling resistance is represented by a coefficient times the gross vehicle weight.

Aerodynamic resistance-The drag due to aerodynamic interaction with the surrounding air is dependent on the square of the relative wind speed. In the absence of ambient wind, the square of the vehicle speed is used. The absolute magnitude of the drag at any speed is proportional, as well, to the frontal area of the vehicle, its drag coefficient, and the local air density.

When all of these forces are added together, the available drive force at any speed is as shown in figure 2. The ordinate in this plot is the drive force divided by weight. It represents the ability for the vehicle to accelerate at full engine power. The numerical scale on the ordinate represents " $g$ 's" of acceleration (longitudinal acceleration/gravitational acceleration). Thus it might be appropriately called the "acceleration reserve," (AR), and the AR may be


Figure 2. Graph of $\mathrm{P} / \mathrm{W}$ versus speed for 1953 Road-Test Data [8].
interpreted as the net force available to accelerate the vehicle, normalized by its weight. The acceleration can be applied either to changing the speed of the vehicle, or counteracting the acceleration component of gravity when the vehicle is on a grade. At the point where the curve intersects the abscissa, there is no acceleration reserve, thus the vehicle cannot accelerate beyond this speed on a level surface, and it represents the theoretical maximum speed determined by engine power. (The actual maximum speed may be less than this due to the gearing selected for the driveline.)

On a grade, the drag force is equivalent to the gross vehicle weight times the grade percentage divided by 100. Because the drag is not dependent on speed, grades can be represented by horizontal lines on the plot. The intersection between a particular grade and the acceleration reserve represents the steady-state speed (final climbing speed) that the vehicle can maintain on that grade. At other speeds, the acceleration or deceleration that will be experienced is equivalent to the difference between the grade line and the AR line.

This plot characterizes the acceleration ability of a truck on a grade while the engine power is applied. It does not represent directly the performance during shifting intervals when the engine is disengaged.

Definitions of Terms. Throughout the rest of this report, many references will be made to the "power" of a truck, often used in the context of a weight-to-power ratio. As seen above, the power available to motivate the truck is different at various points on the vehicle (especially differing between the engine and the drive wheels), and it is helpful for clarity in the discussion to establish certain definitions. Three power symbols will be defined.
$P_{1}$-Engine size may be characterized by its "rated power," either gross or net, the latter including allowances from losses associated with the driven accessories. The $P_{1}$ designation will be used to identify power at the engine, as would be quoted by the truck owner or driver.
$\mathrm{P}_{2}$-For certain purposes it becomes necessary to estimate the average or "effective power" being delivered at the flywheel of the engine, based on the performance observed. The performance mode of interest here will be hill-climbing. $P_{2}$ will be lower than $P_{1}$ because of accessory losses, ambient conditions, the maintenance condition of the engine, shifting losses, or inability of the driver to maintain the engine at its maximum power operating point.
$P_{3}$--Refers to the power available to accelerate the vehicle or overcome grade. It will be lower than $P_{2}$ because of losses in the drive train, rolling resistance losses, and aerodynamic drag. $P_{3}$ is the "drive power," and is the net force, represented in the right-hand side of equation 2 , times the forward speed.

## Characterization of Hill-Climbing Performance

In the past, the highway community has characterized trucks by a weight-to-power ratio for purposes of modeling hill-climbing performance. Other methods can be used. Each involves different levels of comprehensiveness with which the behavior is predicted, the more comprehensive approaches usually carrying a burden of greater complexity in their utilization. The different alternatives are reviewed here as background for identifying the best choice for particular applications.

Simulation Models. The most comprehensive means to characterize a truck is simply to take the approach of analytical prediction using a detailed "simulation" model of a truck climbing a grade. This approach is reflected in a number of computer simulations that calculate speed versus time and distance by integration of the governing equation, such as equation 1. Appropriate descriptions of the aerodynamic and rolling resistance forces are developed for the calculation process. With this approach the effect of transmission shifts can be incorporated directly in the calculations to provide a more realistic estimation of performance. Overall, this approach requires an extensive list of parameters to describe the vehicle in the necessary detail. In return, the calculations yield velocity plots that can closely match the
performance of typical trucks. Figure 3 shows the form of the velocitydistance relationships obtained from simulation of a typical vehicle of $300 \mathrm{lb} / \mathrm{hp}$, where the net engine horsepower is used. Of course, every vehicle will be slightly different. Even the same vehicle with different gearing will produce different results. The multiple plots in figure 3 are obtained from the same vehicle with different sets of gearing, which alters the speeds at which shifts are made. For comparison, the figure also shows the computed performance presuming an infinitely variable transmission, which would not require shifting, but would allow the engine to always operate at maximum power.

Weight-to-(Effective) Power Ratio. For many years the highway community has used an approach based on the simulation method described above for characterizing hill-climbing performance. $(1,6)$ For this purpose, typical parameter values are assumed to describe the truck and the drag losses. The key variable quantifying truck performance is the estimate of the weight and the effective power $\left(P_{2}\right)$ available from the engine. Weight-to-power values that have been used over the years have been selected on the basis of what was known about truck weights and engine power values, and the agreement between predicted and observed hillclimbing performance. This approach takes into account the changes in drag force with speed, rationalizing the use of only one power value to describe the truck, although its value is dependent on the estimates of drag used in its determination. The variations in performance due to shifting (see figure 3) are overcome by arbitrarily smoothing the curves. The predictions of performance obtained are illustrated in the AASHTO curves, shown in figure 4.

Semi-Empirical Equations. Semi-empirical equations for the effective acceleration of a truck on grades have been developed by some researchers. (10) The effective acceleration is a function of road speed. At any particular speed, the value is determined by solution of the force equations, like that of equation 1 , but yielding an acceleration value that is averaged over the period which includes the gear shifting interval. Given the same vehicle and road parameters, the semi-empirical equations simply generate a "smoothed" form of the


Figure 3. Speed-distance plots obtained from simulation of a typical truck on a 6 percent grade.

Deceleration (on Percent Upgrades Indicated)


Figure 4. Speed-distance curves for a typical heavy truck of $300 \mathrm{lb} / \mathrm{hp}$ for deceleration (on percent upgrades). [1]
velocity-time or velocity-distance curves that would be obtained using the simulation models described previously.

Acceleration Reserve. The acceleration reserve described in the section entitled Forces Acting on a Vehicle is another means of representing the performance capabilities of a truck as a function of speed. It is the most direct method for quantifying climbing performance because it is a direct expression of the combination of deceleration and grade. Although analytical predictions of this quantity, based on assumptions for truck properties, will be no more accurate than the three methods described previously, AR values determined from experimental measurements are the most direct characterization of the truck. No assumptions need to be made with regard to drag losses, efficiencies, or other factors, and the reduction in effective climbing ability due to shifting is directly reflected in the AR value observed. From equation 2 , AR can be defined as:

$$
\begin{equation*}
A R=A_{x}+G_{r}=F_{d} / W-\left(F_{r}+F_{a}\right) / W=f(V) \tag{3}
\end{equation*}
$$

At any speed and grade condition the AR then determines the deceleration that will be observed.

$$
\begin{equation*}
d V / d t=A_{x} g=\left(A R-G_{r}\right) g \tag{4}
\end{equation*}
$$

where,

```
\(t=\) time
\(\mathrm{g}=\) gravitational constant
```

Because the velocity, $V$, equals $d X / d t$ ( $X$ being the distance along the road), the equation can also be written:

$$
\begin{equation*}
\mathrm{dV} / \mathrm{dX}=\left(\mathrm{AR}-\mathrm{G}_{\mathrm{r}}\right) \mathrm{g} / \mathrm{V} \tag{5}
\end{equation*}
$$

The equations can be integrated to obtain $V$ as a function of time or distance, presuming $A R$ is known as a function of speed. Note from figure 1 that for speeds above $20 \mathrm{mi} / \mathrm{h}(32 \mathrm{~km} / \mathrm{h})$ the acceleration
reserve is nearly linearly related to speed. In that case equation 2 can be rewritten as:

$$
\begin{equation*}
A R=A_{x}+G_{r}=C_{1}+C_{2} V \tag{6}
\end{equation*}
$$

where

```
\(A_{x}=\) longitudinal acceleration (g's)
```

$G_{r}=$ upgrade $(\% / 100)$
$C_{1}, C_{2}=$ truck characterization coefficients
$\mathrm{V}=\mathrm{velocity}(\mathrm{fps})$

This method is attractive for its directness in describing the acceleration capability on a grade. Only two coefficients are needed to characterize the truck, and no assumptions need be made about the truck. The AR is seen as a means to empirically characterize a truck. There is no direct analytical means to adjust the $A R$ for losses incurred during shifting; however, empirical measurements of the AR will produce an effective value that includes shifting losses.

Using the accleration reserve function of equation 5, velocitydistance curves can be generated by integrating to obtain the velocity as a function of distance. Figure 5 shows the form of the curves obtained on constant grades.

Weight-to-(Drive) Power Ratio. Similar to the AR function, a truck may be characterized by the ratio of weight to drive power $\left(P_{3}\right)$. This method is attractive because a weight-to-power value is more intuitive than AR. This characterization is simply an alternate form of the AR. From equation 3:

$$
\begin{equation*}
A R=A_{x}+G_{r}=F_{d} / W-\left(F_{r}+F_{a}\right) / W=\left(P_{3} / W\right) / V \tag{7}
\end{equation*}
$$

or:

$$
\begin{equation*}
\mathrm{W} / \mathrm{P}_{3}=550 /(\mathrm{AR} \mathrm{~V}) \tag{8}
\end{equation*}
$$



Figure 5. Speed-distance plots calculated from an $A R$ function that is linearly dependent on speed.
where:
$P_{3}=$ Drive horsepower

A constant $W / P$ value implies a hyperbolic shape for the acceleration reserve of the vehicle as a function of speed; in fact, we observe that it is more likely to be linear. At high speed, characterization by a constant may be a poor representation for the steady-state acceleration reserve, which has a linear form. However, at low speed, the constant W/P more closely matches the characteristic shape of the acceleration reserve function.

To accommodate the inconsistency at high and low speeds, it may be anticipated that two $W / P_{3}$ values may be needed to characterize typical truck performance-one value to quantify the high-speed decelerations on entry to a grade, and one value to quantify the final climbing speed. Like the $A R$, the $W / P_{3}$ representation does not directly account for the shifting losses as a truck decelerates on a grade, although these effects will be reflected in the $W / P_{3}$ values determined from empirical measurements. Figure 6 shows the form of the speed-distance curves obtained on a constant grade from calculation with a fixed value of $W / P_{3}$.

## Evaluation of Characterization Methods

The choice of what constitutes the best method for characterizing the truck should be made with first priority given to its ability to reasonably match the performance of typical trucks. The format in which the performance is evaluated assumes critical importance. For example, for the prediction of instantaneous acceleration of a particular 4vehicle, the computer simulation method provides the most detailed record of actual speeds at an arbitrary time, yet the "smoothed" curves of the $A R$ and $W / P$ methods are more appealing for representing the


Figure 6. Speed-distanee plots resulting from a constant $W / P_{3}$ value.
average performance of a sample of trucks. Thus one must ask, what performance predictions are most critical to the highway designer.

For determining critical length of grade, the change of velocity with distance at high speed has assumed the greatest importance. A speed loss of $10 \mathrm{mi} / \mathrm{h}(16 \mathrm{~km} / \mathrm{h})$ is recognized as the threshold of increase in accident frequency. On open highways, where truck entry speeds will be near $55 \mathrm{mi} / \mathrm{h}(89 \mathrm{~km} / \mathrm{h})$, the distances required for speeds to drop to 45 or $40 \mathrm{mi} / \mathrm{h}(64$ or $72 \mathrm{~km} / \mathrm{h}$ ) are the most important for determining where a climbing lane should start. On steep grades the AASHTO curves imply a rather linear relationship between speed and distance, thus the gradient is the most important. On the other hand, on the more shallow grades, the prediction of final climbing speed (and whether it is more than 10 or $15 \mathrm{mi} / \mathrm{h}$ ( 16 or $24 \mathrm{~km} / \mathrm{h}$ ) below mean traffic speed) assumes great importance in determining whether a climbing lane will be needed at all. Again, the predictions of truck speeds in the range of 40 to $45 \mathrm{mi} / \mathrm{h}$ ( 64 to $72 \mathrm{~km} / \mathrm{h}$ ) is the most important. Accurate predictions at lower speeds may not be as critical. Certainly, roads on which mean traffic speeds are 35 to $40 \mathrm{mi} / \mathrm{h}(56-64 \mathrm{~km} / \mathrm{h})$ are less frequent than those with higher speeds, and are less likely to involve long, steep grades.

From the standpoint of estimating highway capacity, the speed-time relationship and final climbing speeds assume greater importance. The integral of speed reduction over time represents the impediment to the free flow of traffic.

Comparing figures 4 and 5 indicates that different speed-distance relationships are obtained from each method of characterization. The AR representation of a vehicle's ability to overcome grade yields a continuous curve. Representation by constant engine power, as in the AASHTO method, results in a nearly bilinear speed-distance relationship, at least when starting from high speeds on steep grades. It is not clear which method more accurately represents actual performance.

In addition to the issue of parameters for characterizing a vehicle, there is also the question of which vehicle to characterize.

The existing AASHTO guidelines describe a single "typical" truck of 300 $\mathrm{lb} / \mathrm{hp}$ used in the context of a "design truck." Inasmuch as the population of trucks using a road encompasses a broad range of performance capabilities, there is no "typical" performance representative of all. The nature of the problem is illustrated in figure 7, which shows the cumulative distribution of tractor-trailer decelerations measured near the beginning of a grade on five different roads with different grade values. Trucks near the top of the distribution, which are decelerating very little or not at all, are not impediments to other traffic. It is the trucks from the midpoint of the curves and down that impact on traffic flow. The midpoint can be represented by the $50^{\text {th }}$ percentile truck, or the average. In general, the averages will differ somewhat from the 50 percentile, reflecting a skewness in the distribution, especially on sites such as "Coyote" identified in the figure. The trucks at the bottom of the distribution (experiencing the greatest decelerations) are the vehicles creating the greatest traffic impedance.

The relationships and models that have been established to link truck speed loss to its impact on traffic safety and highway capacity do not provide an adequate basis to deal with the issue of these performance variations in the truck population. Applying the $10 \mathrm{mi} / \mathrm{h}$ ( $16 \mathrm{~km} / \mathrm{h}$ ) criterion to the real world, where decelerations of the truck population on a given grade exhibit this distribution of performance, a "no-risk" design is not practical. The extremes of performance would dictate ultra-conservative design practices. Given limited resources, the highway engineer must choose to minimize the risk over the whole network, which means minimizing the frequency with which the $10 \mathrm{mi} / \mathrm{h}$ ( 16 $\mathrm{km} / \mathrm{h}$ ) rule is violated on the overall road system. On a lightly traveled road, a higher percentage of the truck traffic at this threshold would equate with a lower percentage on a more heavily traveled road, and the highway managers must ultimately incorporate this risk-taking assessment in their decision process. To do so requires that the distribution of deceleration performance be known. The distribution of decelerations for tractor-trailers shown in figure 7 tends to be rather linear from the midpoint (median truck) down to the


Figure 7. Probability distributions of spatial accelerations for tractor-trailers on five interstate road sites.
12.5 percentile level. Thus a feasible means for characterizing the distribution (suitable for use in more formal and sophisticated decision-making models that will presumably be developed in the future) is to characterize the performance of interest by both a 12.5 and 50 percentile value. Thence, performance at any other percentile level can be predicted by assuming the linear shape. Studies in the State of California have emphasized the 12.5 percentile truck, thus its use allows comparison with that data base. (11) Further, the 12.5 percentile level is reasonable because it falls near the bottom of the linear range and is a "real" value that can be determined directly from experimental observations.

Although vehicles below the 12.5 percentile depart markedly in their performance, these vehicles may be considered atypical, and they would be unreasonable to use as a benchmark for highway design. Included in this group would be over-weight and/or over-width trucks operating by special permit, those with engine problems, or those that are recognized by owners or operators as marginal for highway use.

With these questions in mind, a study of truck hill-climbing performance was conducted, involving both experimental measurements and analyses to identify suitable methods for characterizing the performance observed.

In order to provide answers to some of the questions posed in chapter 2, experimental measurements of the climbing performance of over 4,000 trucks were made throughout the country. Appendix A details the methods that were used. From 20 sites distributed both in the East and West, the speed loss of trucks was measured on grades from 2 to 6 percent, along with descriptive data about the trucks. Individual trucks were tracked through the grades, and at some sites additional data on weight and power were obtained while they were stopped at nearby weigh stations. This base of data allows many types of analyses to answer questions about hill-climbing performance. In the sections that follow, analyses of the key issues will be discussed with the objective of providing more quantitative data on hill-climbing performance.

Final Climbing Speeds

On constant grades of sufficient length a truck will decelerate to a steady speed, of ten called the "final climbing" speed. Final climbing speed is significant both because of its influence on highway capacity, and because of what it tells about truck performance capabilities. At this operating condition, shifting is no longer required and the speed achieved represents a balance between engine tractive effort and the drag forces acting on the truck. On steep grades the primary drag is that due to grade which can be determined independently by measurement of the grade angle. This contrasts with measurements during the deceleration phase at the beginning of grade where deceleration levels must also be determined to quantify performance.

Examination of the final climbing speed is selected as the first step in presentation of experimental results because it can be compared directly with data provided in the AASHTO guide, and it provides a simple format for illustrating the distribution of truck population.

Figure 8 shows the final climbing speed of tractor-trailers as a function of grade observed on the 20 sites. Tractor-trailers are selected for the plot because they tend to represent one of the most homogeneous classes in the population (with the least data scatter). Especially on shallower grades, some tractor-trailers have sufficient power to climb the grade at normal traffic speed. Thus the "average" speeds tend to be higher than those for the median ( 50 percentile) vehicles. This is an indication of an asymmetric population distribution, and the use of an "average" reflects a bias when compared to the median. Alternately, the properties of trucks at the lower end of the performance range can be characterized by the velocity of lower percentile vehicles. The 12.5 percentile value has been used by the California Department of Transportation. (11) This precedent and the fact that it generally falls on the linear portion of the probability distribution of decelerations (see figure 7) makes it a reasonable choice for use here. Superimposed on the plot is the curve of speed versus grade corresponding to the AASHTO values obtained from reference 6.

The general slope of the data points for all three measures is similar, closely matching that of the AASHTO curve. The data points do not fall exactly along a constant weight-to-power (W/P3) curve, although the random scatter in the data points is larger than the deviation between a trend line and a constant power line.

Figure 9 shows the 12.5 percentile values for final climbing speed by truck class and road class. As would be expected, the experimental data points reflect a variation in the performance of trucks at different sites. Several interpretations can be applied to the data. On the one hand, one could establish a "trend" line that best fits the data points, minimizing mean square errors, or such. This would be an estimate of typical 12.5 percentile performance for which a variance is still required to characterize the limit. A special problem that will be encountered in many cases with this approach is that the limited data will result in a trend that does not relate properly to the independent variable (grade in this case). For example, the best fit line may show


Figure 8. Average, median, and 12.5 percentile of final climbing speeds for tractor-trailers.


Figure 9a. Final climbing speeds of straight trucks (12.5 percentile level).


Figure 9b. Final climbing speeds of trucks with trailers (12.5 percentile level).


Figure 9c. Final climbing speeds of tractor-trailers
(12.5 percentile level).


Figure 9d. Final climbing speeds of doubles and triples (12.5 percentile level).
final climbing speed increasing with grade, which conflicts with the mechanics involved.

An alternative approach is to attempt to bound the experimental observations with a limit that reasonably matches the mechanics involved. In figure 9a this would be equivalent to shifting the AASHTO curve upward to the level of the lowest data points, using the AASHTO curve as a reasonable reflection of how final climbing speed should vary with grade. As will be seen with much of the experimental data, this approach can provide a very good match to the data. In effect the bound represents a performance limit--the nominal limit of performance at which the owners or drivers choose to operate the vehicles. At whatever percentile may be chosen, this is a conservative estimate of performance. By and large, at any arbitrary site on the highway network, truck performance should be at least as good as the limit selected.

The AASHTO values for final climbing speed are clearly conservative in estimating the performance of trucks and tractortrailers. They are roughly equivalent to perhaps a 5 percentile vehicle in those cases. On the other hand, the curve closely approximates the 12.5 percentile limit for trucks with trailers (figure $9 b$ ) and for doubles and triples combinations (figure 9d). Only one data point, a western primary for the trucks with trailer (figure 9b), falls significantly below the AASHTO curve, and then, only 16 vehicles were in the sample from which this 12.5 percentile point was determined. To reflect performance of all vehicles at the 12.5 percentile level, the AASHTO speeds would have to be increased by about $3 \mathrm{mi} / \mathrm{h}(5 \mathrm{~km} / \mathrm{h}$ ) for straight trucks and tractor-trailers.

Figure 9 shows that the distinction between final climbing speeds on different road classes is not especially significant. For straight trucks, the final climbing speeds tend to be somewhat lower on Eastern roads than on Western roads (figure 9a). A slight indication of the same trend is seen also with tractor-trailers. The same tendency is not seen for straight trucks with trailers, or for doubles and triples.

The final climbing speeds observed here can be related directly to a weight-to-power ratio. From equation 7, a relationship can be derived as follows:

$$
\begin{equation*}
U_{f c}=375 /\left(W / P_{3} * G_{r}\right) \tag{9}
\end{equation*}
$$

where

```
\(\mathrm{U}_{\mathrm{fc}}=\) Speed (MPH)
\(G_{r}=\) Fractional grade (\%/100)
```

Decelerations at Speed

Truck decelerations at high speed on a grade are of primary importance in determining where a climbing lane should start. The AR and $W / P_{3}$ values (both being related) are direct measures of high-speed performance. The values may be determined from the observations of deceleration and speed, using a discrete form of equation 5. That is, by noting the change in speed between two points on a known grade and the average speed, the $A R$ can be calculated. The $W / P_{3}$ is obtained from equation 8. The three speed measurements in the entry portion of the grade yield two values. An additional value is obtained from the final climbing speed where the acceleration is zero and the AR is simply equivalent to the grade. For the convenience of the reader, the more familiar $W / P_{3}$ form will be used in subsequent discussion.
$\mathrm{A} W / \mathrm{P}_{3}$ to characterize a truck population can be determined in several ways. Values for individual vehicles can be calculated, and then the population properties established for that sample. Two values from each vehicle will be obtained from the three speed measurements. Thus the median vehicle in the first set of traps may not be the median vehicle in the second set, or at the final climbing point. Also the vehicles with the largest decelerations (and highest apparent $\mathrm{W} / \mathrm{P}_{3}$ ) may tend to be the vehicles traveling at the highest speed because of the higher aerodynamic drag acting on the vehicle.

An alternate way to associate a $\mathrm{W} / \mathrm{P}_{3}$ with a grade site is to determine the speed population, like that of figure 7, at various points along the grade. The deceleration properties of the truck population between those two points can then be inferred, and the $\mathrm{W} / \mathrm{P}_{3}$ calculated on that basis. This method is preferable for characterizing speed changes along a grade, although it should be recognized that deceleration used in the calculations is not that of a particular truck (at a given percentile, a different truck is seen at each point in the grade), rather it is that of the population.

The procedure used is to determine the probability distribution of the speeds at each measurement point. Then, at a given percentile level, the drop in speed from point to point along the grade is used to establish the spatial deceleration ( $d V / d X$ ) for which a $W / P_{3}$ is calculated. Because the $W / P_{3}$ values are likely to be speed dependent, the average speed must also be calculated. Thus the 12.5 percentile $W / P_{3}$ value indicates the rate at which the 12.5 percentile speeds are decreasing on a given grade from a given initial speed, and answers the needs of the highway designer in estimating speed changes of the truck traffic stream along the grade.

It might be expected that the two independent variables most affecting $W / P_{3}$ will be the speed and grade. At high speed the aerodynamic and rolling resistance forces are greatest, elevating its value. In turn, on steep grades where the decelerations are greatest, the need to continuously shift the transmission is likely to lower the effective power being extracted from the engine, with an associated decrease in the average drive power.

Figures 10 to 13 show the 12.5 percentile $W / P_{3}$ values on different road classes. Figure 10 covers trucks, Figure ll--trucks with trailers, Figure 12-tractor-trailers, and Figure 13--doubles and triples.

Also shown on these plots is an "AASHTO curve." It is difficult to associate a specific $\mathrm{W} / \mathrm{P}_{3}$ value with the AASHTO predictions of truck performance during the deceleration phase, because multiple values exist as a result of the arbitrary way in which speed-distance curves have


Figure 10a. 12.5 percentile $\mathrm{W} / \mathrm{P}_{3}$ values for straight trucks on Eastern interstate road sites.


Figure 10b. 12.5 percentile $\mathrm{W} / \mathrm{P}_{3}$ values for straight trucks on Western interstate road sites.


Figure 10c. 12.5 percentile $W / P_{3}$ values for straight trucks on Eastern primary road sites.


Figure 10d. 12.5 percentile $W / P_{3}$ values for straight trucks on Western primary road sites.


Figure 1la. 12.5 percentile $W / P_{3}$ values for trucks with trailers on Western interstate road sites.


Figure 1lb. 12.5 percentile $W / P_{3}$ values for trucks with trailers on Western primary road sites.


Figure 12a. 12.5 percentile $W / P_{3}$ values for tractor-trailers on Eastern interstate road sites.


Figure 12 b . 12.5 percentile $W / P_{3}$ values for tractor-trailers on Western interstate road sites.


Figure 12c. 12.5 percentile $W / P_{3}$ values for tractor-trailers on Eastern primary road sites.


Figure 12d. 12.5 percentile $W / P_{3}$ values for tractor-trailers on Western primary road sites.


Figure 13a. 12.5 percentile $W / P_{3}$ values for doubles and triples on Eastern interstate road sites.


Figure 13b. 12.5 percentile $W / P_{3}$ values for doubles and triples on Western interstate road sites.


Figure 13c. 12.5 percentile $W / P_{3}$ values for doubles and triples on Western primary road sites.
been smoothed. In the absence of shifting, $\mathrm{W} / \mathrm{P}_{3}$ values can be calculated using the equations for truck performance given in reference 6. These represent the lower limit of $W / P_{3}$ as a function of speed. But the truck simulation algorithm used for computation of speed-distance performance curves includes shifting intervals during which there is complete loss of engine power. The shifting losses vary with calculations for each grade condition; thus, at a given speed multiple values for $W / P_{3}$ exist, one for each grade. For example, at 40 $\mathrm{mph}\left(64 \mathrm{~km} / \mathrm{h}\right.$ ) the steady-state $\mathrm{W} / \mathrm{P}_{3}$ value will be $537 \mathrm{lb} / \mathrm{hp}$; on the other hand, the slopes of the speed-distance curves at the same speed reflect $\mathrm{W} / \mathrm{P}_{3}$ values ranging from about 680 to $930 \mathrm{lb} / \mathrm{HP}$ (the different values depending on which grade curve was taken on the AASHTO plot). The steady-state values of $W / P_{3}$ were used for the AASHTO curve in these figures. Thus it can be interpreted as a conservative choice.

Consider first figure 10. In each plot three points for each site are shown connected by straight lines (the lines shown only for convenience in associating the data points for a site). The two data points at the highest speeds usually represent performance calculated for the intervals between the first and second speed measurements, and between the second and third. The third data point at the lowest speed is derived from the final climbing speed measurement.

In figure $10 a$, six sites are shown, labeled in the legend according to the city nearest the site. The sites are listed in the legend in order of increasing grade at the final climbing point (which is not necessarily the same as at the beginning of grade). With the exception of "Wheeling," all data points fall below the AASHTO curve. Thus the 12.5 percentile speed changes at these sites were representative of trucks with a lower weight-to-power ratio than used for the AASHTO predictions. The Wheeling data are peculiar for no explanable reason and will be excluded from the discussion. Otherwise, the data appear to show a slight trend of $\mathrm{W} / \mathrm{P}_{3}$ rising with speed. A trend of this nature would be expected simply from the mechanics of the forces acting on trucks.

Examining the plots for straight trucks on the other types of roads, it is clear that the AASHTO assumptions on $W / P_{3}$ are very conservative. The general level of the AASHTO curve could be dropped by $50 \mathrm{lb} / \mathrm{hp}$ and still have the majority of data points fall below its level.

The same is true for tractor-trailer combinations shown in figure 12. The tractor-trailers generally show more consistent performance in every case with no profound differences in performance between the East and West or between interstate and primary roads.

Straight trucks with trailers (figure 11) are remarkably different. Data are shown only for Western sites (interstate and primary), because there were insufficient vehicles in this class at the Eastern sites to determine a 12.5 percentile. The AASHTO curve falls near the midpoint of the data spread. The fact that more consistent performance was observed with tractor-trailers on each of these same sites would suggest that the variability is associated with the vehicles rather than being due to site factors.

Figure 13 shows the performance of doubles and triples. No data are shown for primary eastern sites because of the few number of doubles encountered on these roads. The AASHTO curve is generally a good estimate of the minimum performance of these vehicles, with only a few of the data points exceeding its value.

## Performance Characterization

It is clear from the previous figures that the AASHTO curves for decelerations on grades are overly conservative for several types of vehicles, since they do not account for some of the differences between vehicle classes. The dilemma that arises with availability of more detailed data on truck performance is how to characterize those observations. The characterization problem involves two dimensions; what percentile truck should be chosen and what functional relationship to use.

In chapter 2 the rationale for use of the 12.5 and 50 percentile values was presented as a means to characterize the population distribution. From these, predictions of performance at any other percentile value can be made based on the assumption of linearity in the critical range of the distribution. This does not, however, solve the problem of which percentile value to use for setting performance limits. In the absence of a recognized basis for making such a choice, it is arrived at by default. In the interest of choosing limits that are more conservative than those of the median population, the 12.5 percentile value is reasonable. The 12.5 percentile truck is one truck in eight. Other choices, such as the 10 percentile (one truck in ten), may also seem reasonable from the intuitive viewpoint, although it is less desirable from the practical viewpoint. The 10 percentile value falls closer to the curved ends of the distribution (see figure 7). Thus, finding 10 percentile performance carries with it greater risk of misrepresenting the true slope of the distribution. Even though the 12.5 percentile is chosen as a limit in this report, the results and conclusions that are presented can be adjusted to reflect any other percentile point once a rationale is developed to justify its choice.

The rationale for choosing a functional form to represent performance limits is also steeped in utility. The decelerations implicit in the speed-distance curves used by AASHTO (see figure 4) are obtained by "smoothing" the speed-distance curves calculated for a "typical" truck. Thus their shape is based on arbitrary assumptions with regard both to the parameters used to characterize the typical truck, and to the method used to smooth the resultant curves. Although the curves were adjusted to ensure overall agreement with what was known about truck performance at the time of their development, the decelerations at any speed and grade condition may not necessarily be representative of any fraction of the truck population.

The experimental data obtained in this project have been reduced to values for the effective power available to accelerate the truck at any condition of speed and grade $\left(\mathrm{P}_{3} / \mathrm{W}\right)$. With this measure it is not necessary to make any assumptions with regard to the losses due to drag forces acting on the vehicle or the losses due to shifting. It is a
direct measure of performance impacting on speed loss on a grade. $P_{3} / W$ will vary with speed. The functional form should be as follows:

$$
\begin{equation*}
P_{3} / W=P_{2} / W-A V-B V^{2}-C V^{3} \tag{10}
\end{equation*}
$$

The first term on the right-hand side, $\mathrm{P}_{2} / \mathrm{W}$, is the normalized power available at the engine, which is nominally constant. The second and third terms are, respectively, the constant and speed-dependent portions of the rolling resistance power loss. The last term represents power loss from aerodynamic forces. A precise functional relationship between $\mathrm{P}_{3} / \mathrm{W}$ and speed would involve all of these terms. Evaluating all constants, however, would require more experimental data than that available here.

Lacking the necessary information to evaluate all terms, a good approximation is to assume $P_{3} / W$ is a linear function of speed. That is:

$$
\begin{equation*}
P_{3} / W=C_{1}+C_{2} v \tag{11}
\end{equation*}
$$

The linear function can exactly match the higher order function at two speeds. By carefully selecting these speeds, a good approximation of the higher order function is obtained over a limited range. For hill-climbing characterization the speeds of $25 \mathrm{mi} / \mathrm{h}$ and 50 mph ( 40 and $80 \mathrm{~km} / \mathrm{h}$ ) are the logical choices. A good match at $25 \mathrm{mi} / \mathrm{h}(40 \mathrm{~km} / \mathrm{h})$ ensures that final climbing speed is accurate, and a good match at 50 $\mathrm{mi} / \mathrm{h}(80 \mathrm{~km} / \mathrm{h})$ ensures that the high-speed decelerations are accurate.

Although this simplified representation of truck performance does not properly represent two of the speed-dependent terms, as will be seen, it provides a reasonable match to experimental observations. It is likely that the losses integral to the higher order terms are insignificant when compared to the influence of shifting losses. Despite the fact that this is an approximation, it should be noted that it does not require making assumptions for truck parameters or curve smoothing as used in development of the present AASHTO curves.

Perhaps the most important consideration in using this characterization method is the ease with which it can be used to relate
to experimental observations. Given a large number of experimental data points, it is impossible to choose a set of vehicle parameters which will constitute a truck with performance matching the observations.

Characterization of Tractor-Trailer Performance

Tractor-trailers have been selected as the first vehicle class to characterize because they are the most homogeneous in performance, and they illustrate the application of the method with the least confusion from outlier data points. Figures $12 a$ to $d$ showed the $W / P_{3}$ values for the 12.5 percentile decelerations of tractor-trailers on all sites measured. Although the individual data points exhibit a degree of variation, the majority fall below an upper bound similar in shape to the AASHTO curve. There is no systematic difference between interstate and primary roads, nor between Eastern and Western sites.

Figures 14 a and 14 b show the collective data for all sites plotted for the 12.5 and 50 percentile decelerations. On the 50 percentile plot the upper limit of $\mathrm{W} / \mathrm{P}_{3}$ is clearly evident. At $25 \mathrm{mi} / \mathrm{h}(40 \mathrm{~km} / \mathrm{h})$ the upper bound is approximately $250 \mathrm{lb} / \mathrm{hp}$. Assuming a $\mathrm{W} / \mathrm{P}_{3}$ value of 475 $\mathrm{lb} / \mathrm{hp}$ at $50 \mathrm{mi} / \mathrm{h}(80 \mathrm{~km} / \mathrm{h})$ and that $\mathrm{P}_{3} / \mathrm{W}$ is linearly dependent on speed as in equation 10 , produces the 50 percent limit curve shown. Its shape is nonlinear because $W / P_{3}$ is the inverse of the linear $P_{3} / W$. Most importantly, the limit has a shape that reflects the proper functional relationship to speed. It is comparable to the AASHTO curve, and its level and slope can be matched to the data points by choice of the $\mathrm{W} / \mathrm{P}_{3}$ values at 25 and $50 \mathrm{mi} / \mathrm{h}$ ( 40 and $80 \mathrm{~km} / \mathrm{h}$ ). In a comparable fashion the 12.5 percentile limit is obtained by selection of 375 and $550 \mathrm{lb} / \mathrm{hp}$ at the speeds of 25 and $50 \mathrm{mi} / \mathrm{h}$ ( 40 and $80 \mathrm{~km} / \mathrm{h}$ ).

Choosing a boundary for the data is a subjective judgment, but it is perhaps more straightforward than the judgments implicit in the methods used previously for development of AASHTO guidelines. In the 50 percent plot the single point for the interstate-east that falls above the limit has been arbitrarily ignored as an outlier simply because it does not appear to fit the bounds appropriate to the other data points.


Figure 14a. 12.5 percentile $W / P_{3}$ values for tractor-trailers on all roads.


Figure 14b. 50 percentile $\mathrm{W} / \mathrm{P}_{3}$ values for tractor-trailers on all roads.

The same issue arises in the plot for the 12.5 percentile data. Exclusion of outlier points is more easily rationalized in the 12.5 percentile data because we are already dealing with the extreme of the population.

The selection of a performance limit as shown here may appear to be somewhat tenuous with uncertain implications. Its validity can be assessed by looking more explicitly at the performance that it attempts to model. Specifically, the objective is to provide a reasonable estimate of the decelerations in speed and the final climbing speeds. The decelerations will be a function of both speed and grade, and the final climbing speed will be a function of grade. The spatial deceleration is calculated as follows:

$$
\begin{equation*}
\mathrm{dU} / \mathrm{dX}=0.465\left(375 \mathrm{P}_{3} /(\mathrm{W} \mathrm{U})-\mathrm{G}_{\mathrm{r}}\right) \mathrm{g} / \mathrm{U} \tag{12}
\end{equation*}
$$

where
$\mathrm{U}=$ velocity in mph
$X=$ distance along the grade in feet
$P_{3} / W=$ horsepower per pound
$G_{r}=$ grade fraction (\%/100)
$g=$ gravitational constant ( $32.2 \mathrm{ft} / \mathrm{sec}^{2}$ )

The final climbing speed is also obtained from this equation when $d U / d X$ equals zero. Thus it is determined by solution for the speed at which the term within the parentheses on the right-hand side becomes equal to zero.

The equation may be solved for any assumed form of $P_{3} / W$. For the 12.5 percentile tractor-trailer ( $\mathrm{W} / \mathrm{P}_{3}$ values of 375 and $550 \mathrm{lb} / \mathrm{hp}$ at speeds of 25 and $50 \mathrm{mi} / \mathrm{h}$ ( 40 and $80 \mathrm{~km} / \mathrm{h}$ ), respectively):

$$
\begin{equation*}
\mathrm{P}_{3} / \mathrm{W}=.001(3.515-.0339 \mathrm{U}) \tag{13}
\end{equation*}
$$

Spatial decelerations were calculated for grades of 3, 4, 5, and 6 percent. These are plotted in figure $15 \mathrm{a}-\mathrm{d}$. Also shown are the decelerations extracted from the AASHTO speed-distance curves. They were obtained by evaluating the slope of the curve for each grade at a series of speeds. For comparison, the spatial decelerations for 12.5 percentile tractor-trailers were determined for the speed measurement points at all sites. These represent experimental data points. A grade value is associated with each data point, although not precisely equal to $3,4,5$, or 6 percent. Thus they were grouped into ranges of 2.4 to $3.4,3.5$ to $4.4,4.5$ to 5.4 , and 5.5 to 6.5 . These data points are entered, respectively, on the $3,4,5$, and 6 percent plots. Because we are attempting to bound the performance, the experimental data should fall under the curves to be valid. The plots clearly illustrate that the 12.5 percent limit is a more reasonable boundary than that of the AASHTO curves. The intercept of the 12.5 percent limit with the abscissa determines the final climbing speed for each grade. Its proximity to at least one data point on the abscissa in each plot shows it to be a much more reasonable estimate of final climbing speed than the current AASHTO curves. Throughout the plots the data points at higher speeds approach, but do not exceed, the 12.5 percent limit. They are not all expected to fall on the curve because it is, in fact, a limit intended to bound performance. The higher level of the AASHTO deceleration indicates that it is a more conservative estimate of performance limits for modern trucks--one that is perhaps inappropriately conservative.

## Characterizing Straight Truck Performance

The experimental data show that the performance of straight trucks is more variable. The $\mathrm{W} / \mathrm{P}_{3}$ values that were shown in figure 10 appear more dependent on the road class, and they are slightly less consistent than those for tractor-trailers.

For trucks on interstate routes, the 12.5 and 50 percentile $W / P_{3}$ data are shown in figure 16. Eastern and Western sites are


Figure 15a. Decelerations on $3 \%$ grades, 12.5 percentile tractor-trailers.


Figure 15b. Decelerations on $4 \%$ grades, 12.5 percentile tractor-trailers.


Figure 15c. Decelerations on $5 \%$ grades, 12.5 percentile tractor-trailers.


Figure 15d. Decelerations on $6 \%$ grades, 12.5 percentile tractor-trailers.


Figure 16a. 12.5 percentile $W / P_{3}$ values for straight trucks on interstate roads.


Figure 16b. 50 percentile $W / P_{3}$ values for straight trucks on interstate roads.
distinguished in the plots by the symbol used. The distinction between East and West is a little more obvious with straight trucks than with tractor-trailers. The Western data points generally exhibit a limit that is about $50-75 \mathrm{lb} / \mathrm{hp}$ lower than that for the east.

The 12.5 percentile limit used for tractor-trailers fits the eastern data points for this vehicle class. That is, the curve established by $W / P_{3}$ values of $375 \mathrm{lb} / \mathrm{hp}$ at $25 \mathrm{mi} / \mathrm{h}(40 \mathrm{~km} / \mathrm{h})$ and 550 $\mathrm{lb} / \mathrm{hp}$ at $50 \mathrm{mi} / \mathrm{h}(80 \mathrm{~km} / \mathrm{h})$ yields a reasonable bound for the Eastern straight truck data. The actual expression for the $P_{3} / W$ is presented in a summary at the end of this chapter. Although one might independently come up with a somewhat different limit, as will be seen later, there is great advantage to being able to apply the same limit to both types of vehicles. Certainly, it is difficult to say that the straight trucks are significantly different from the tractor-trailers to justify a different limit. Note that in the 12.5 percentile plots for interstate routes the two data points above the limit have been treated as outliers based on the subjective judgment that they do not appear consistent with the remainder of the data.

The Western data in this figure for the 12.5 percentile trucks fall somewhat below the limit just selected for the Eastern data, indicating that straight trucks operating on the Western interstates have a generally higher performance level (lower $W / P_{3}$ ). A second limit is shown for these points based on 290 and $500 \mathrm{lb} / \mathrm{hp}$.

The 50 percentile limit for tractor-trailers also matches well the data for straight trucks on Eastern interstate routes. That boundary is established from $\mathrm{W} / \mathrm{P}_{3}$ values of $250 \mathrm{lb} / \mathrm{hp}$ at $25 \mathrm{mi} / \mathrm{h}(40 \mathrm{~km} / \mathrm{h})$ and 475 $\mathrm{lb} / \mathrm{hp}$ at $50 \mathrm{mi} / \mathrm{h}(80 \mathrm{~km} / \mathrm{h})$. For the Western data a limit based on 200 and $400 \mathrm{lb} / \mathrm{hp}$ is more appropriate.

Straight trucks on primary roads tend to be higher in performance than on interstates (lower $W / P_{3}$ values). The explanation may be that they tend to be more lightly loaded. Straight trucks operating on interstates are presumably traveling for longer distances, and for economic reasons are loaded more heavily. The 12.5 and 50 percentile
performance is presented in figure 17. The limits used for tractortrailers are a little high to closely match the straight truck performance on primary roads. The 12.5 percentile limit is based on $\mathrm{W} / \mathrm{P}_{3}$ values of 350 and $500 \mathrm{lb} / \mathrm{hp}$ at 25 and $50 \mathrm{mi} / \mathrm{h}$ ( 40 and $80 \mathrm{~km} / \mathrm{h}$ ). Those for the 50 percentile are based on 150 and $300 \mathrm{lb} / \mathrm{hp}$. The 50 percentile exhibits an especially clear boundary. The 12.5 percentile is not so clear and has one data point that falls above the limit. The presence of data points from both the East and the West near the limit suggests that there is no geographic distinction between straight truck performance on primary roads.

Characterizing Straight Trucks with Trailers

Characterizing the performance limits of straight trucks with trailers is difficult because of the absence of conclusive data. On Eastern sites very few were encountered, resulting in samples of a halfdozen or less at many sites. Although a median can be inferred from measurements of only a few trucks, a 12.5 percentile cannot. Thus the 12.5 percentile performance could only be determined for some of the Western sites. Their performance is shown in figure 18a. The limit is based on $525 \mathrm{lb} / \mathrm{hp}$ at $25 \mathrm{mi} / \mathrm{h}(40 \mathrm{~km} / \mathrm{h})$ and $625 \mathrm{lb} / \mathrm{hp}$ at $50 \mathrm{mi} / \mathrm{h}$ ( 80 $\mathrm{km} / \mathrm{h})$. The data are consistent enough to state that trucks with trailers are much lower in performance than straight trucks without trailers and should be recognized as a separate class of vehicles.

Comparisons between East and West and between interstates and primaries can only be made at the 50 percentile level. Figure 18 b shows the 50 percentile performance data. The distribution of data points would seem to justify a distinction between performance in the East and West. Thus two limits are shown in the plot. For the East, the limit is established by $W / P_{3}$ values of 350 and $1200 \mathrm{lb} / \mathrm{hp}$ at 25 and $50 \mathrm{mi} / \mathrm{h}$ ( 40 and $80 \mathrm{~km} / \mathrm{h}$ ), respectively. For the West, the limits are based on 325 and $550 \mathrm{lb} / \mathrm{hp}$.

In light of the fact that the Eastern trucks with trailers are so much lower in performance at the 50 percentile level, it is likelly that


Figure 17a. 12.5 percentile $W / P_{3}$ values for straight trucks on primary roads.


Figure 17b, 50 percentile $\mathrm{W} / \mathrm{P}_{3}$ values for straight trucks on primary roads.


Figure 18a. 12.5 percentile $W / P_{3}$ values for trucks with trailers on Western interstate roads.


Figure 18b. 50 percentile $W / P_{3}$ values for trucks with trailers on all roads.
the 12.5 percentile limit would be much lower than that for the West. Although Eastern trucks with trailers are bounded by a much lower performance limit even at the 50 percentile level, note that the actual data points tend to be more broadlyl distributed in the plot. The implication is that trucks with trailers are much more variable in the East.

Characterizing Performance of Doubles and Triples

Experimental data for doubles and triples suffered from the same problems as that for straight trucks with trailers. Only a marginal number of vehicles were encountered at some sites. Nevertheless, the number of doubles was sufficient to assess 12.5 and 50 percentile performance on interstates in the East and West, and on primary roads in the West.

The majority of vehicles encountered were doubles comprised of two short trailers. The short trailers are nominally 27 ft ( 8 m ) in length, producing a combination vehicle length of about $65 \mathrm{ft}(20 \mathrm{~m})$. In the West, a long and a short trailer may be combined into a unit frequently called a "Rocky Mountain Doubles." Several of these were encountered, but were insufficient in number to allow assessment of their hillclimbing performance. Thus the data on doubles vehicles has been limited to the $65-f t(20-m)$ combination.

Also in the West, 12 triples were included in the measurements, 10 at one site. Ten vehicles provides a sample large enough to calculate 12.5 and 50 percentile values for comparison to performance of the doubles, although one site is not sufficient to generalize about the population as a whole.

Figures 19 a and b show the performance plots for doubles at the 12.5 and 50 percentile levels. The 12.5 percentile limit is established by 475 and $800 \mathrm{lb} / \mathrm{hp}$ at 25 and $50 \mathrm{mi} / \mathrm{h}(40$ and $80 \mathrm{~km} / \mathrm{h}$ ). The two data points at the lowest speeds fall slightly above this boundary, but were not taken as justification for raising the boundary line. Eastern and


Figure 19a. 12.5 percentile $W / P_{3}$ values for doubles and triples on all roads.


Figure 19b. 50 percentile $W / P_{3}$ values for doubles and triples on all roads.

Western interstates and the Western primary roads are all represented near the boundary, thus there is no distinction by geographic location or road type.

Also shown on the plot are three data points (the data from one site) for triples operating on a Western interstate road. These are included to show the performance observed with the triples, even though only ten vehicles were included in the sample. Although no concrete conclusions can be drawn, these data would indicate that the performance of triples is comparable to that of $65-\mathrm{ft}(20-\mathrm{m})$ doubles.

The 50 percentile limit shown in figure 19 b is established by 350 and $700 \mathrm{lb} / \mathrm{hp}$ at 25 and $50 \mathrm{mi} / \mathrm{h}$ ( 40 and $80 \mathrm{~km} / \mathrm{h}$ ). The Eastern and Western interstates are both near the boundary, indicating no geographic differences. The Western primaries fall further from the boundary, indicating that slightly better performance is obtained at the median level. Data points for the triples are near the 50 percentile limit shown.

## Summary of Performance Characteristics

In all the discussion that has preceded, it is difficult to keep a clear picture of the performance characteristics that have been concluded with regard to vehicle classes, road classes, and 12.5 versus 50 percentiles. For convenience, the results are summarized in tables 1 and 2.

[^0]Table 1. $W / P_{3}$ values ( $1 \mathrm{~b} / \mathrm{hp}$ ) at 25 and $50 \mathrm{mi} / \mathrm{h}$ ( 40 and $80 \mathrm{~km} / \mathrm{h}$ ) by vehicle and road class.

|  | Interstate |  | Primary |  |
| :---: | :---: | :---: | :---: | :---: |
|  | East | West | East | West |
| Straight Trucks |  |  |  |  |
| 12.5\% | 375, 550 | 290, 500 | 350, 500 | 350, 500 |
| 50.0\% | 250, 475 | 200, 400 | 150, 300 | 150, 300 |
| Trucks with Trailers |  |  |  |  |
| 12.5\% | --- | 525, 625 | -- | 525,625 |
| 50.0\% | 350, 1200 | 325, 550 | 350, 1200 | 325, 550 |
| Tractor-trailers |  |  |  |  |
| 12.5\% | 375, 550 | 375, 550 | 375, 550 | 375, 550 |
| 50.0\% | 250, 475 | 250, 475 | 250, 475 | 250, 475 |
| 65-ft Doubles |  |  |  |  |
| 12.5\% | 475, 800 | 475, 800 | - | 475, 800 |
| 50.0\% | 350, 700 | 350, 700 | --m | 350, 700 |

## Table 2. $\mathrm{P}_{3} / \mathrm{W}$ equations by vehicle and road class.

## Interstate

## Primary

Straight Trucks

| $12.5 \%$ East | $\mathrm{P}_{3} / \mathrm{W}=(3.52-.0339 \mathrm{U}) / 1000$ | $\mathrm{P}_{3} / \mathrm{W}=(3.71-.0343 \mathrm{U}) / 1000$ |
| :--- | :--- | :--- |
| $12.5 \%$ West | $\mathrm{P}_{3} / \mathrm{W}=(4.90-.0579 \mathrm{U}) / 1000$ | $\mathrm{P}_{3} / \mathrm{W}=(3.71-.0343 \mathrm{U}) / 1000$ |
| $50.0 \%$ East | $\mathrm{P}_{3} / \mathrm{W}=(5.89-.0758 \mathrm{U}) / 1000$ | $\mathrm{P}_{3} / \mathrm{W}=(10.0-.1333 \mathrm{U}) / 1000$ |
| $50.0 \%$ West | $\mathrm{P}_{3} / \mathrm{W}=(7.50-.1000 \mathrm{U}) / 1000$ | $\mathrm{P}_{3} / \mathrm{W}=(10.0-.1333 \mathrm{U}) / 1000$ |

Trucks with Trailers
12.5\% East

| $12.5 \%$ West | $\mathrm{P}_{3} / \mathrm{W}=(2.21-.0122 \mathrm{U}) / 1000$ | $\mathrm{P}_{3} / \mathrm{W}=(2.21-.0122 \mathrm{U}) / 1000$ |
| :--- | :--- | :--- | :--- |
| $50.0 \%$ East | $\mathrm{P}_{3} / \mathrm{W}=(4.88-.0809 \mathrm{U}) / 1000$ | $\mathrm{P}_{3} / \mathrm{W}=(4.88-.0809 \mathrm{U}) / 1000$ |
| $50.0 \%$ West | $\mathrm{P}_{3} / \mathrm{W}=(4.36-.0504 \mathrm{U}) / 1000$ | $\mathrm{P}_{3} / \mathrm{W}=(4.36-.0504 \mathrm{U}) / 1000$ |

## Tractor-trailers

| 12.5\% East \& West | $\mathrm{P}_{3} / \mathrm{W}=(3.52-.0339 \mathrm{U}) / 1000$ | $\mathrm{P}_{3} / \mathrm{W}=(3.52-.0339 \mathrm{U}) / 1000$ |
| :---: | :---: | :---: |
| 50.0\% East \& West | $\mathrm{P}_{3} / \mathrm{W}=(5.89-.0758 \mathrm{U}) / 1000$ | $\mathrm{P}_{3} / \mathrm{W}=(5.89-.0758 \mathrm{U}) / 1000$ |

65-ft Doubles

| 12.5\% East | $\mathrm{P}_{3} / \mathrm{W}=(2.96-.0342 \mathrm{U}) / 1000$ | - |
| :---: | :---: | :---: |
| 12.5\% West | $\mathrm{P}_{3} / \mathrm{W}=(2.96-.0342 \mathrm{U}) / 1000$ | $\mathrm{P}_{3} / \mathrm{W}=(2.96-.0342 \mathrm{U}) / 1000$ |
| 50.0\% East | $\mathrm{P}_{3} / \mathrm{W}=(4.29-.0571 \mathrm{U}) / 1000$ | - --- |
| 50.0\% West | $\mathrm{P}_{3} / \mathrm{W}=(4.29-.0571 \mathrm{U}) / 1000$ | $\mathrm{P}_{3} / \mathrm{W}=(4.29-.0571 \mathrm{U}) / 1000$ |

resistance properties, aerodynamic properties, gearing, tire size, and drive line efficiencies.

Population characteristics of the weights of trucks operating on the road system are generally available to the highway community through the routine measurements made at weigh stations. Getting a reasonable picture of the power available to accelerate a truck is more difficult. The Truck Inventory in Use (TIU) survey conducted periodically by the Department of Commerce includes an inquiry on the power installed in each truck. (13) This "reported" power, of course, is not the same as that available at the wheels. However, if it could be related to the power available for hill-climbing, then the TIU survey results could be utilized in conjunction with weight survey results to estimate how truck performance is changing.

In order to address this issue, more comprehensive data were acquired at certain of the field test sites. Two each of the Eastern and Western sites were selected because of close proximity to a truck weigh station. In addition to the measurements of hill-climbing performance, other data were obtained at the weigh station. Gross vehicle weights were obtained from the weight measurements. The driver was interrogated to obtain a figure for the power of the engine. Most drivers know the rated power of the engine in a truck, a figure which should compare closely with that obtained from the owner in the TIU survey. The vehicle type, factors related to its frontal area, the presence of aerodynamic aids, and the type of tires (radial or bias) were also noted. Vehicle descriptions allowed the data from the weigh station to be linked to that obtained on the grade.

The raw averages of the weight and power figures are the first items of interest. Table 3 shows the "actual" values by truck type and road class. The numbers in parentheses following the road class listing indicate the number of vehicles sampled. The weight-to-power figures shown are equivalent to $W / P_{1}$. That is, the power figure is based on installed, rather than, effective horsepower. The values are determined from the average weight divided by average power.

Table 3. Average weights and power values for trucks.

Weight (1b) Power (HP) Weight/Power

| Straight Trucks |  |  |  |
| :---: | :---: | :---: | :---: |
| Interstate - East (14) | 15233 | 219 | 70 |
| Interstate - West (6) | 35050 | 267 | 131 |
| Primary - East (6) | 16575 | 273 | 75 |
| Trucks with trailers |  |  |  |
| Interstate - East (2) | 12300 | 193 | 64 |
| Interstate - West (7) | 48430 | 346 | 140 |
| Primary - East (1) | 76780 | 400 | 192 |
| Tractor-trailers |  |  |  |
| Interstate - East (157) | 54452 | 328 | 166 |
| Interstate - West (233) | 64775 | 370 | 175 |
| Primary - East (134) | 57487 | 330 | 174 |
| 65-ft Doubles |  |  |  |
| Interstate - West (19) | 64920 | 331 | 196 |

The weight-to-power ratio for the individual trucks was also calculated and averaged to see if it resulted in a different figure that would indicate some bias due to interaction between weight and power. Essentially the same $\mathrm{W} / \mathrm{P}_{1}$ averages were obtained both ways. .This would indicate that it is valid to obtain average weights and average power levels for modern trucks and determine the average $\mathrm{W} / \mathrm{P}_{1}$ from their ratios.

The weight-to-power values seen here do not exhibit the same trends as have been observed for the overall populations in the previous sections. For example, straight trucks in the East have a lower W/P ratio than tractor-trailers, although the 12.5 percentile limits were found to be comparable. Several reasons are possible explanations. First, these are averages for one or two sites, not 12.5 percentiles for many. Second, the sample sizes for straight trucks here are small and marginally significant. The reasons for the small sample size for straight trucks, trucks with trailers, and doubles is their small representation in the truck population at the measurement sites, and the fact that the complete data, as needed here, were only captured on a fraction of those vehicles passing the site. These differences in $\mathrm{W} / \mathrm{P}$ values do not prevent this data from being meaningful. The purpose here is to examine a few trucks in detail to determine how their performance relates to what would be expected.

The weight-to-power values for the trucks sampled in this study are lower than those projected from the TIU data. Figure 20 is a plot from reference 14 showing the weight-to-power ratios for trucks compiled from studies over the years. The triangles show data from the 1977 TIU study based on maximum weight and reported horsepower. Added to the figure are data points obtained from table 3. Data points for the Eastern trucks with trailers have been excluded from the plot because of the small sample size. The data points show a trend that falls significantly below the TIU line. In operation, the trucks have a lower weight-to-power ratio than the TIU data would suggest. Tractortrailers, which are nominally in the $60,000-$ to $80,000-1 \mathrm{~b}$ weight class, appear to operate on the average at about $60,000-$ to $65,000-1 b$ gross vehicle weight. The average horsepower from this study is approximately


Figure 20. Trends in weight-to-power since 1949 [14].

350, up 25 percent from the 282 hp average for comparable vehicles from the 1977 TIU survey. Thus, the major reason for reduced weight-to-power ratios is the increase in horsepower. Inasmuch as eight years have elapsed since the TIU study, it is likely that the statistics seen in table 3 are more representative of modern trucks even though they are derived from a much smaller sample size.

The data were analyzed in depth to estimate an "effective" power being extracted from the engine during the grade-climbing experience. The estimate is derived from the measured speed and speed loss on grade, to which are added additional power consumption estimates for rolling resistance and aerodynamic drag. Parameters for estimating these contributions were obtained from the additional data acquired on the truck at the weigh station. Rolling resistance was estimated from the SAE equations as follows:

$$
\begin{array}{ll}
C_{r r}=.001(4.1+.041 \mathrm{U}) & \text { for radial tires } \\
C_{r r}=.001(5.3+.044 \mathrm{U}) & \text { for mixed tires } \\
C_{r r}=.001(6.6+.046 \mathrm{U}) & \text { for bias-ply tires } \tag{14c}
\end{array}
$$

The aerodynamic drag forces were estimated from the familiar equation:

$$
\begin{equation*}
\mathrm{F}_{\mathrm{a}}=0.5 \mathrm{DC}_{\mathrm{d}} \mathrm{AV}_{2} \tag{15}
\end{equation*}
$$

where
$D=$ air density, corrected for altitude

$$
\begin{aligned}
& C_{d}=\text { drag coefficient ( } 0.7 \text { with aero-aids, } 0.8 \text { without) } \\
& A=\operatorname{area}\left(100 \mathrm{ft}^{2} \text { for van bodies, } 75 \mathrm{ft}^{2}\right. \text { for cab only) }
\end{aligned}
$$

Thus the effective power estimated is that which is available from the engine at the drive wheels. Losses due to drive line efficiency, shifting, engine maintenance condition, or accessories are not included. It is a modified form of $P_{2}$ in that these last items are not included.

The "effective" power calculated in this manner can be compared to the "actuals" (table 3) to determine a factor characterizing the utilization of the power that is theoretically available in the vehicle. Separate utilization factors can be determined for performance in the deceleration portion of the grade and at the final climbing condition. The method generally yielded comparable "effective" power values in both phases of the climbing process, typically within 10 to 20 percent. The utilization factors obtained are listed in table 4.

Note that a fairly consistent pattern emerges showing about the same utilization in the deceleration and final climbing stages of the grade. The straight trucks are least consistent, varying from about 40 percent to 60 percent utilization. The generally low values may be indicative of high representation of vehicles powered by gasoline engines in this class. It is reasonable to expect a much higher engine power utilization with diesel power plants than with gasoline because it is routine to run a diesel near maximum r/min (approximately $2,000 \mathrm{r} / \mathrm{min}$ ), which is the power peak. On the other hand, fewer drivers would climb a long grade with a gasoline engine running near its maximum power as that speed is normally about 4,000 to $4,500 \mathrm{r} / \mathrm{min}$. It is not only unpleasantly loud, but it verges on the point of being abusive of the engine.

From table 4, reasonable utilization factors can be estimated. For straight trucks in the East, utilization factors of about 45 percent of engine power are reasonable. Straight trucks in the West, however, run at about 65 percent of rated power. Highway tractors used with semitrailers or multiple trailers (doubles) generally yield utilization factors of about 80 percent, indicating that the drivers are very effective at using the power available from the engine. Data for trucks with trailers were only available for Western sites. A utilization factor of about 70 percent is indicated.

As average vehicle weights or engine power levels change in the future fleet, these results would suggest that a reasonable estimate of the changes in hill-climbing performance can be made. The installed power can be corrected to an effective value at the drive wheels by

Table 4. Power utilization factors (effective/actual)

|  | Straight <br> Trucks | Trucks - <br> Trailers | Tractor- <br> Trailers | $65-f t$ <br> Doubles |
| :---: | :---: | :---: | :---: | :---: |
| Final Climbing |  |  |  |  |
| Interstate - east | 0.40 | --- | 0.75 | --- |
| Interstate - west | 0.65 | 0.74 | 0.86 | 0.85 |
| Primary - east | 0.43 | ---- | 0.79 | --- |


| Deceleration |  |  |  |  |
| :--- | :--- | :--- | :--- | :--- |
| Interstate - east | 0.45 | $-\infty$ | 0.68 | $--\infty$ |
| Interstate - west | 0.62 | 0.63 | 0.88 | 0.81 |
| Primary - east | 0.44 | $-\infty$ | 0.84 | $--\infty$ |

multiplying by the utilization factor. The power available for acceleration ( $P_{3}$ ) is then obtained from this by subtracting of $f$ aerodynamic and rolling resistance losses. In the event changes in aerodynamic or rolling resistance losses are projected (from greater use of aerodynamic aids, or radial tires), their impact on the $P_{3}$ power can be applied directly. That is, presuming the effective power at the drive wheels is unchanged, the increase in $P_{3}$ is simply equivalent to the decrease in these other losses.

## INTERPRETATION AND APPLICATIONS

The experimental observations of truck speed loss on grades in this project clearly show the AASHTO speed-distance curves to be a very conservative basis for design of climbing lanes. Yet to use the new information, methods must be defined for predicting speed losses on grades at the design synthesis stage.

## Calculations of Speed Loss

The formulation of the $\mathrm{P}_{3} / \mathrm{W}$ function to characterize performance provides a very simple and easily applied method for calculating speed losses on grades for a particular class of vehicle. The method is contained in equation 12 , which is of the form:

$$
\begin{equation*}
d U / d X=0.465\left(375\left(P_{3} / W\right) / U-G_{I}\right) g / U \tag{12}
\end{equation*}
$$

where

```
\(\mathrm{U}=\) speed \((\mathrm{mi} / \mathrm{h})\)
\(X=\) distance (ft)
\(G_{r}=\) road grade (percent/100)
\(g=\) gravitational constant \(=32.2 \mathrm{ft} / \mathrm{sec}^{2}\)
```

The $\mathrm{P}_{3} / \mathrm{W}$ functions used in the equation are obtained from those listed in table 1 for the particular class of vehicle of interest. The equation itself cannot be readily integrated to provide a closed-form solution; however, it is simple enough to be programmed on the smallest desktop microcomputer. Figure 21 lists a Basic-language program to calculate speed-distance curves for an arbitrary grade. The initial speed, $\mathrm{W} / \mathrm{P}_{3}$ values for 25 and $50 \mathrm{mi} / \mathrm{h}$ ( 40 and $80 \mathrm{~km} / \mathrm{h}$ ), and elevationdistance (grade) parameters are set within the program. Running the

```
10 REM
20 REM
30 REM
4 0 ~ R E M
50 REM
Program for calculating speed-distance curves
    Select entry speed in line 100
    Select weight-to-power values in line 110
    Define grade by distance-elevation values in lirie 300
        ........by T. D. Gillespie, 1985
90 pi=100: REM Sets distance intervals at which values print out
100 ENTRSPED=55: U=ENTRSPED: REM Set entry speed to desired value
110 WP25=375: WP50=550: REM Choose W/P3 values at 25 and 50 MPH
120 B=(1/WP50-1/WP25)/25: A=1/WP25-B*25
130 READ DIST,ELEV: REM Read grade on initial segment
140 GR=ELEV/DIST: XL=DIST: YL=ELEU
150 PRINT "Distance (Ft) Speed (MPH)": PRINT USING "#####.##"; X,U
160 DELU=,464876*(375*(A+B*U)/U-GR)*32.2/U*10
170U=U+DELU
180 X=X+10
190 IF X)XL THEN 200 ELSE 220
200 READ DIST,ELEV
210 GR=(ELEU-YL)/(DIST-XL): XL=DIST: YL=ELEU
220 IF X MOD pi<1 THEN 230 ELSE 160
230 PRINT USING "######,##"; X, U: GOTO 160
300 REM Enter grade data here in distance, elevation values (feet)
310 DATA 500,30
320 DATA 1000,60
330 DATA 1500,90
340 DATA 2000,120
350 DATA 2500,150
360 DATA 10000,600
```

Figure 21. Basic-language program for computing speed-distance curves from $W / P_{3}$ values.
program produces a listing of speed versus distance along the arbitrarily defined grade.

Plots of speed-distance are also provided in figure 22 for the various classes of vehicles on constant grades. These may be useful for those without access to a computer, in which case they can be used in a way comparable to that applied to the earlier AASHTO curves. That is, an initial speed is assumed, and the arbitrary grade profile is broken up into sections of constant grade. Then the curves are used to estimate speed loss along each section, producing a speed profile from entry point to final climbing point.

More importantly, the plots in figure 22 provide a visual framework in which to compare the speed-distance performance observed in this project to that in the AASHTO guide. Figure 22a is perhaps the most important in this regard as it applies to the 12.5 percentile tractor-trailers. Tractor-trailers are the most numerous heavy vehicles of any class encountered on many roads, and the AASHTO speed-distance curves were based on performance of tractor-trailers. The predictions for "critical length of grade" for these vehicles in figure 22a make an interesting comparison to the AASHTO data. In an absolute sense, the differences are minor on steep grades. For example, the critical length of grade for a $10 \mathrm{mi} / \mathrm{h}(16 \mathrm{~km} / \mathrm{h})$ speed loss on a 6 percent grade is nominally $600 \mathrm{ft}(183 \mathrm{~m})$. In figure 22 a a distance of about 700 ft (213 m) is indicated. However, on a shallow grade of 3 percent the AASHTO distance is $1,400 \mathrm{ft}(427 \mathrm{~m})$, compared to about $2,100 \mathrm{ft}(640 \mathrm{~m})$ in figure 22 a . The $700-\mathrm{ft}(213-\mathrm{m})$ difference represents a major change in highway design. The differences become even more profound near 2 percent; where the AASHTO guide indicates a $2,500-f t$ ( $762-\mathrm{m}$ ) critical length, figure 22 a shows $6,000 \mathrm{ft}(1,829 \mathrm{~m})$. Clearly the performance levels reflected by this new data indicate that longer values for critical length of grade are appropriate.


Figure 22a. Speed loss for vehicles at $W / P_{3}$ values of 375 and $550--$ 12.5\% tractor-trailers on all roads, $12.5 \%$ straight trucks on Eastern interstates, and $12.5 \%$ straight trucks on all roads (optional).


Figure 22b. Speed loss for vehicles at $W / P_{3}$ values of 290 and 500-$12.5 \%$ straight trucks on Western interstates.


Figure 22c. Speed loss for vehicles at $W / P_{3}$ values of 350 and 500 . $12.5 \%$ Straight trucks on primary roads


Figure 22d. Speed loss for vehicles at $W / P_{3}$ values of 525 and $625-$ $12.5 \%$ trucks with trailers on Western roads.


Figure 22e. Speed loss for vehicles at $W / P_{3}$ values of 475 and $800-$ $12.5 \%$ doubles and triples on ali roads.


Figure 22f. Speed loss for vehicles at $W / P_{3}$ values of 250 and 500 -$50 \%$ tractor-trailers on all roads, $50 \%$ straight trucks on Eastern interstates, and $50 \%$ straight trucks on all roads (optional.


Figure 22g. Speed loss for vehicles at $W / P_{3}$ values of 200 and $400--$ $50 \%$ straight trucks on Western interstates.


Figure 22 h . Speed loss for vehicles at $W / \mathrm{P}_{3}$ values of 150 and $300--$ $50 \%$ straight trucks on primaries.


Figure 22i. Speed loss for vehicles at $W / P_{2}$ values of 325 and 550 -$50 \%$ trucks with trailers in the West.


Figure $22 j$. Speed loss for vehicles at $W / P_{3}$ values of 350 and $1200^{--}$ $50 \%$ trucks with trailers in the East.


Figure 22 k . Speed loss for vehicles at $W / P_{\imath}$ values of 350 and $700-$ $50 \%$ doubles and triples on all roads.

Dealing with Traffic Mixes

The experimental observations clearly show distinctive differences in performance among different classes of vehicle and roads. To use this information constructively, methods must be developed for estimating performance of a mixed population.

It has been argued previously that the frequency of vehicles operating at the critical speed on a grade is a measure of hazard created. Thus the traffic density and the distribution of speed deficiencies among the trucks are the determinants of that frequency. The distribution of speeds (more accurately, speed changes) for an arbitrary mix of trucks is somewhat complicated to calculate analytically.

To do so, a deceleration distribution (similar to that shown in figure 9) must be calculated for the mix of vehicles expected to use the site. The procedural steps are as follows:

1) Assume values for the vehicle mix, initial speed, and initial grade.
2) Calculate the spatial deceleration, $\mathrm{dU} / \mathrm{dX}$, for the 12.5 and 50 percentile vehicles in each truck class using equation 12 as illustrated in the example below.
3) Plot the distribution of spatial deceleration for each vehicle class as a fraction of the total population.
4) Determine the distribution for the total population by summing the values for each vehicle class at specific levels of deceleration. Then from the distribution for the total population, the deceleration for the 12.5 percentile of the traffic mix (or any other percentile of choice) can be read from the graph.

As an example consider an assumed mix of 20 percent doubles and 80 percent tractor-trailers on an interstate of 4 percent grade, where the entry speed is expected to be $55 \mathrm{mi} / \mathrm{h}(88 \mathrm{~km} / \mathrm{h})$. These assumptions are step 1 in the procedure.

For step 2, the spatial decelerations are calculated. The $P_{3} / W$ functions given in table 2 for each truck class are different, so the decelerations will differ. The spatial deceleration will be given by the equation:

$$
\begin{equation*}
\mathrm{dU} / \mathrm{dX}=0.465\left(375\left(P_{3} / W\right) / \mathrm{U}-\mathrm{G}_{\mathrm{r}}\right) \mathrm{g} / \mathrm{U} \tag{12}
\end{equation*}
$$

where

$$
\begin{aligned}
& \mathrm{P}_{3} / \mathrm{W}=(3.52-.0339 \mathrm{U}) / 1000-12.5 \% \text { Tractor-trailers (table 2) } \\
& \mathrm{P}_{3} / \mathrm{W}=(5.89-.0758 \mathrm{U}) / 1000-50 \% \text { Tractor-trailers (table 2) } \\
& \mathrm{P}_{3} / \mathrm{W}=(2.96-.0342 \mathrm{U}) / 1000-12.5 \% \text { Doubles (table 2) } \\
& \mathrm{P}_{3} / \mathrm{W}=(4.29-.0571 \mathrm{U}) / 1000-50 \% \text { Doubles (table 2) }
\end{aligned}
$$

From this equation, spatial deceleration values at $55 \mathrm{mi} / \mathrm{h}$ ( 88 $\mathrm{km} / \mathrm{h}$ ) are calculated with the following results:

| $12.5 \%$ Tractor-trailers | $-7.82 \mathrm{mi} / \mathrm{h}$ per 1000 ft |
| :--- | :--- |
| $50 \%$ Tractor-trailers | $-7.70 \mathrm{mi} / \mathrm{h}$ per 1000 ft |
| $\mathbf{1 2 . 5 \%}$ Doubles | $-8.89 \mathrm{mi} / \mathrm{h}$ per 1000 ft |
| $50 \%$ Doubles | $-8.70 \mathrm{mi} / \mathrm{h}$ per 1000 ft |

After these are calculated, the deceleration is plotted for step 3 as shown in figure 23.

The tractor-trailers represent 80 percent of the population, thus, their distribution establishes the decelerations for that fraction of the vehicles. The 12.5 percentile tractor-trailer is the 10 percentile of the population (. $125 \times 80$ percent). Thus its deceleration (the value of -7.82 ) is plotted at the 10 percent point, as shown in figure 23a. The 50 percentile tractor-trailer is the 40 percentile of the population (. $4 \times 80$ percent). Thus its deceleration (the value of -7.70 ) is plotted at the 40 percent point. The actual distribution for the


Figure 23a. Plot of deceleration distribution for tractor-trailers.


23b. Addition of deceleration distribution for doubles.


Figure 23c. Deceleration distribution for
the total population.
tractor-trailers can then be approximated by drawing a straight line through these points from zero to the 80 percent level on the ordinate.

A similar procedure is used to plot the estimated distribution for the doubles in figure 23 b , using the 20 percent level on the ordinate because the doubles represent that fraction of the vehicles. That is, points are established at -8.89 and 2.5 percent (. $125 \times 20$ percent), and at -8.70 and 10 percent (. $5 \times 20$ percent). Then a straight line is drawn through these points from zero to 20 percent.

As the last step, the distribution for the total population is determined by summing values for the doubles and the tractor-trailers at specific levels of deceleration. The resultant curve is the distribution for the total population as shown by the bold line in figure 23c. Now presuming that the need for a climbing lane will be based on the 12.5 percentile decelerations, the 12.5 percentile value from the total population would be used for estimating speed loss at that point on the grade. In this case it will be dominated by the doubles, because the complete population of doubles decelerates more rapidly than the tractor-trailers. The 12.5 percentile for the total vehicle population is equivalent to the 62.5 percentile doubles.

As the speed changes along the grade, the same process must be repeated to estimate spatial decelerations at subsequent points. A similar process is required to estimate the distribution of speeds at the final climbing point.

The process can be simplified somewhat by making some reasonable assumptions and approximations. Presuming the entry speed is $55 \mathrm{mi} / \mathrm{h}$ ( $88 \mathrm{~km} / \mathrm{h}$ ), and a speed drop of $10 \mathrm{mi} / \mathrm{h}(16 \mathrm{~km} / \mathrm{h})$ is the critical value, the calculations can be made for an assumed speed of $50 \mathrm{mi} / \mathrm{h}(80 \mathrm{~km} / \mathrm{h})$. Thence, the resultant deceleration may be assumed correct for that first region of the grade, and the critical length determined on that basis.

The differences between vehicle classes are not so critical when only straight trucks and tractor-trailers are involved because their performance is reasonably comparable. However, trucks with trailers, or doubles represent classes of vehicles with much lower performance. A
simple approach would be to design on the basis of the lower performing vehicles, although that could be overly conservative in some cases. If the lower performing vehicles make up more than 12.5 percent of the truck population on the road, then in most cases their spatial deceleration distribution will determine that for the 12.5 percentile level of the total population. However, to determine the 12.5 percentile deceleration properly, the method in figure 23 should be used.

If the lower performing vehicles represent much less than 12.5 percent of the population, then the deceleration distribution for the larger fraction of vehicles will determine the deceleration for the 12.5 percentile level of the population. However, it will occur at the larger class percentile level equivalent to 12.5 minus the percent of the lower performing vehicles.

Once the 12.5 percentile deceleration level has been determined, the critical length of grade is calculated by dividing the acceptable speed reduction (i.e., 10 or $15 \mathrm{mi} / \mathrm{h}$ ) ( 16 or $24 \mathrm{~km} / \mathrm{h}$ ) by the deceleration level.

All this presents a rather complicated picture for estimating 12.5 percentile performance of a mixture of truck traffic. The methodology grows even more complicated in the case of arbitrarily varying grade, or cases where different entry speeds would be expected for different classes of vehicles. Simpler rules of thumb can be applied in some cases.

Speed-Distance for Truck and Tractor-Trailer Mixed Traffic

Because of the close similarity of the performance of straight trucks and tractor-trailers, one simplification is to use the speeddistance plots of figure 22a for traffic of this mix. Straight trucks in the East and on Western interstates exhibited somewhat better performance (less speed loss) than indicated here. Thus, the critical lengths of grade determined from this plot will be conservative in these
geographic areas. Inasmuch as some judgment must always be applied in the decision-making process, the other appropriate speed-distance plots from figure 22 can be referenced to estimate the range in variation of the "critical length of grade" that might be possible by analysis of the separate vehicle classes. On steep grades ( 4 to 8 percent), the differences in critical length will be on the order of $100 \mathrm{ft}(30 \mathrm{~m})$ or less. Only on the shallow grades ( 2 to 3 percent) do the differences stretch out to several hundred feet.

A second benefit from using a single plot for both straight trucks and tractor-trailers is that it is not necessary to know beforehand the actual mix of vehicles on the highway. Were one to try to take advantage of the better performance of straight trucks using the method in the previous section, their representation in the traffic mix would have to be estimated.

Final Climbing Speeds
The final climbing speed is of general interest in determining whether climbing lanes are warranted and the impact of grades on traffic speeds and capacity. The final climbing speeds for the 12.5 percentile vehicles will differ by vehicle class. For the case of straight trucks, it has been found that some differences in performance exist depending on road class and geographic locale. However, the presumption of straight truck performance equivalent to that of tractor-trailers is warranted for reducing the complexity of dealing with traffic mixes. In final climbing speeds the difference between the various straight truck limits is on the order of 2 to $3 \mathrm{mi} / \mathrm{h}$ ( 3 to $5 \mathrm{~km} / \mathrm{h}$ ). Thus they are not treated separately in summarizing the final climbing speed results. Table 5 lists the final climbing speeds for the 12.5 percentile vehicles by vehicle class. All straight trucks are assumed to be equivalent to tractor-trailers in this table. Note that on 1.5 percent grades all vehicles can maintain speed within $15 \mathrm{mi} / \mathrm{h}(24 \mathrm{~km} / \mathrm{h})$ of the $55 \mathrm{mi} / \mathrm{h}$ ( 89 $\mathrm{km} / \mathrm{h}$ ) national speed limit with doubles at the limit just marginal for consideration of a climbing lane if the number of vehicles on the road

Table 5. Final climbing speeds (mi/h), $12.5 \%$ vehicles.

| Grade (\%) | Straight | Trucks wi | Tractor- | 65-ft |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | Trucks | Trailers | Trailers | Doubles | AASHTO |
| 1.5 | 47.5 | 42.3 | 47.5 | 39.9 | -- |
| 2 | 40.3 | 33.7 | 40.3 | 33.8 | ---- |
| 3 | 30.9 | 24.0 | 30.9 | 25.9 | 26.5 |
| 4 | 25.0 | 18.6 | 25.0 | 21.0 | 22.0 |
| 5 | 21.0 | 15.2 | 21.0 | 17.7 | 18.4 |
| 6 | 18.1 | 12.8 | 18.1 | 15.2 | 15.5 |
| 7 | 15.9 | 11.1 | 15.9 | 13.4 | 13.8 |
| 8 | 14.2 | 9.8 | 14.2 | 12.0 | 12.2 |
| 9 | 12.8 | 8.8 | 12.8 | 10.8 | 10.6 |

warrant it. By 2 percent grades, straight trucks and tractor-trailers are down by $15 \mathrm{mi} / \mathrm{h}(24 \mathrm{~km} / \mathrm{h})$, as well. If there is significant representation of trucks with trailers or doubles in the traffic mix the 12.5 percentile speed will be down by more than $15 \mathrm{mi} / \mathrm{h}(24 \mathrm{~km} / \mathrm{h})$.

Estimating a distribution of final climbing speeds is performed in a manner similar to that for the spatial decelerations. Distributions for each vehicle class are constructed from the 12.5 and 50 percentile values, and the distribution for the total population is determined from their sum. For this purpose, table 6 lists the final climbing speeds for the 50 percentile vehicles. The speeds shown for the trucks with trailers are based on $W / P_{3}$ values for the West, as was data for the 12.5 percentile speeds shown in table 5 .

Table 6. Final climbing speeds (mi/h), $50 \%$ vehicles.

| Grade (\%) | Straight | Trucks with | Tractor- | 65-FT |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | Trucks | Trailers (W) | Trailers | Doubles | AASHTO |
| 1.5 | 50.9 | 48.0 | 50.9 | 44.1 | --- |
| 2 | 45.7 | 41.8 | 45.7 | 38.8 | -m-- |
| 3 | 37.8 | 33.3 | 37.8 | 31.3 | 26.5 |
| 4 | 32.3 | 27.6 | 32.3 | 26.2 | 22.0 |
| 5 | 28.2 | 23.6 | 28.2 | 22.5 | 18.4 |
| 6 | 25.0 | 20.6 | 25.0 | 19.7 | 15.5 |
| 7 | 22.5 | 18.3 | 22.5 | 17.6 | 13.8 |
| 8 | 20.4 | 16.4 | 20.4 | 15.8 | 12.2 |
| 9 | 18.7 | 14.9 | 18.7 | 14.4 | 10.6 |

The main objective in this project was to obtain experimental measurements of the hill-climbing performance of modern trucks, and develop methods for predicting speed loss of the general truck population on arbitrary grades. The data and methods have significance as potential aids in the decision-making process with regard to the need for, and design of, truck climbing lanes. The work has resulted in some significant conclusions with regard to truck performance prediction:

1) The AASHTO curves for speed versus distance on different grades are conservative estimates of truck performance, nominally equivalent to the 12.5 percentile of the lower performing truck classes (trucks with trailers, and doubles). The performance limits for 12.5 percentile straight trucks and tractor-trailers are somewhat higher than the AASHTO values. For these vehicles the final climbing speeds are 2 to $4 \mathrm{mi} / \mathrm{h}$ ( 3 to $6 \mathrm{~km} / \mathrm{h}$ ) higher. The rate of speed loss on grades (spatial decelerations) observed for straight trucks and tractortrailers was lower than that of the AASHTO speed-distance curves. Thus, the "critical length of grade" indicated in the AASHTO guide is shorter than warranted for these vehicles. On a 6 percent grade the "critical length" based on AASHTO is approximately 100 feet shorter than necessary. On a 3 percent grade it is about 700 feet shorter.
2) Measurable differences in performance were observed among certain truck classes, road classes, and geographic locations. Tractortrailers exhibited consistent performance throughout the country on both interstate and primary roads. Straight trucks had slightly better performance on primary roads, and on interstates in the West. Trucks pulling trailers and doubles are significantly lower in performance than trucks and tractor-trailers.
3) A simplified means of predicting truck hill-climbing performance was developed based on characterization of the available power for accelerating and overcoming grade (denoted by the symbol " $P_{3}$ "). The ratio of available power to weight $\left(P_{3} / W\right)$ is speed
dependent, but it provides an easy means for calculating truck speed profiles on arbitrary grades. Appropriate $P_{3} / W$ ratios, representative of the 12.5 and 50 percentile of most vehicle classes, was determined from the experimental data acquired in the project.
4) The recognition that performance variations exist within vehicle classes, and between vehicle classes, brings to focus a need for more comprehensive methods for decision making on climbing lane design. Minimizing the frequency of trucks operating below a critical speed on the highway network is suggested as the goal in a decision model. The performance of the 12.5 percentile truck in a population has been suggested as a benchmark for conservatively estimating critical length of grade. Methods for determining performance of the 12.5 percentile vehicle in a mixed population of truck classes is provided.

Although the project was successful at answering many of the questions posed at the outset, and clarifying many of the issues involved, it has become obvious that there are many areas of need for data and methodology by which to refine the climbing-lane design process. Extensive data were obtained on tractor-trailer vehicles and reasonable samples were obtained for straight trucks. The homogeneity observed with tractor-trailer vehicles suggests that their characterization is well founded. The more limited data on trucks, and the differences observed on interstate and primary highways would argue that more experimental data should be acquired to refine the estimates of their performance limits. In the meantime, it is recommended that the speed-distance relationships for the 12.5 percentile vehicle given in figure $22 a$ be used for prediction of straight truck and tractortrailer performance. This figure should be considered as an alternative to the AASHTO speed-distance curves on roads where essentially all truck traffic is of these two classes.

The data on straight trucks pulling trailers, and doubles and triples are so limited that the performance limits determined here should be taken only as estimates of the population as a whole. More experimental data on these particular vehicle classes are warranted before performance limits can be confidently assessed. The speed loss
on grade for the 12.5 percentile of both of these vehicle classes appears comparable to that in the current AASHTO guide. Thus, the AASHTO is still appropriate for characterizing these vehicles, pending more experimental data to improve predictions of their performance. For optimal design, the AASHTO guidelines should not be applied casually to highways simply because truck traffic of these vehicle classes is present. Consideration of the performance for the overall traffic mix may allow a longer critical length of grade at the 12.5 percentile performance level.

The characterization of performance within truck and road classes, as has been determined in this work, results in a more complex decisionmaking process for the rational design of climbing lanes. There is need for improved methodology to guide the decision-making process which properly considers the distribution of vehicle performance on a grade. Insights from this work have been suggested. The notion that the goal in the decision process is to minimize the frequency of encounters with low-speed trucks in a highway network points to the need for treatment from a probabilistic approach. The 12.5 and 50 percentile performance levels, plus the observation that deceleration distributions are approximately linear, provides a basis for describing the distributions of performance among vehicles. Further research in this area is recommended.

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## APPENDIX A

## FIELD DATA COLLECTION ON HILL-CLIMBING PERFORMANCE

The objective of the field data collection exercise was to acquire data on a variety of trucks throughout the country, by which to characterize their hill-climbing performance. A primary interest was to determine whether their performance was variable with geographic location within the country, and with road type. That objective dictates that field measurements be carried out in various regions of the country. Yet, a truly random sample throughout the country is not economically feasible. Instead, a purposeful random sampling method was used.

Sites

In the purposeful sample, sites were selected to achieve stratification in the variables of geography, interstate/primary road classes, and urban/rural locations. Inasmuch as long grades greater than 2 percent in slope are required to get measurements that include a final climbing speed condition, the sites are necessarily going to be located primarily in the eastern and western mountain regions.

Inquiries were sent to state highway departments and transportation agencies in both regions requesting candidate sites for measurement. Respondees were requested to complete a data form on each proposed site covering such essentials as route, location, road classification, grade, average daily truck traffic, number of lanes, and roadside conditions. Also, candidate sites in close proximity to a truck weigh station were requested to allow collection of more detailed data on truck parameters at these sites.

State personnel proved very cooperative and provided lists of approximately 100 sites. These were reviewed and site selections were
made to obtain a balanced representation at each level of stratification. Thus 10 Eastern and 10 Western sites were chosen, including 2 weigh scale sites in each region. The eight remaining sites in each region were then chosen to provide two sites each in the categories of:

- Interstate urban
- Interstate rural
- Primary urban
- Primary rural

In the selection process, consideration was given to obtaining representation of grades over the range of 3 to 8 percent; and preference was given to sites for which an alternate was located in close proximity. The identification of alternate sites in close proximity proved to be an advantageous feature for this type of operation, as many of the selected sites often proved unsatisfactory from the standpoint of visibility, traffic interferences from on-ramps, etc. Overall, many of the sites that were first choice were not used, and suitable sites with grades above 6 percent were not found. The list of sites where data were collected is provided in table 7. The interpretation of what constitutes an urban site, in contrast to a rural site, leaves much room for judgment. In the descriptions shown, those indicated as urban sites were not just close to a city, but also carried what appeared to be local traffic. Only four sites closely matched this intention. Although that disrupts the balance of rural/urban samples, they were balanced in that two each were in the East and West, and a primary and interstate road was obtained in each case. In the original plan, it was the intention as well to try and classify traffic in the local/long distance categories. As it turned out, the state personnel had no information of this nature, and it was not possible to classify thusly in the data collection, so that objective had to be dropped.

Table 7. List of sites for truck hill-climbing performance measures.

| Route | Nearest city | Location | Weigh Scales | Grade(\%) ${ }^{1}$ |
| :---: | :---: | :---: | :---: | :---: |
| I-81 | Hazelton, PA | Rural |  | 2.4, 2.5, 3.6 |
| I-80 | Milesburg, PA | Rural |  | 3.3, 3.5, 2.9 |
| I-64 | Waynesboro, VA | Rural |  | 2.5, 2.9, 3.9 |
| I-77 | Wytheville, VA | Rural | X | $4.0,4.0,4.0$ |
| I-70 | Wheeling, WV | Urban |  | 4.7, 5.1, 5.0 |
| I-48 | Cheat Lake, WV | Rural |  | 6.1, 6.4, 6.1 |
| I-8 | Coyote, CA | Rural |  | 5.2, 5.3, 5.9 |
| I-17 | Camp Verde, AZ | Rural |  | 2.8, 3.2, 4.8 |
| I-25 | Trinidad, CO | Rural | X | 4.5, 5.2, 6.4 |
| I-70 | Denver, C0 | Urban |  | 4.6, 5.9, 6.2 |
| I-84 | Bliss, ID | Rural | X | 3.1, 4.0, 4.0 |
| I-80 | Wells, NV | Rural |  | 5.4, 4.7, 5.3 |
| SR22 | Duncansville, PA | Rural |  | 4.7, 5.8, 4.9 |
| SR12 | Utica, NY | Urban |  | 4.7, 4.9, 5.0 |
| SR15 | Blossburg, PA | Rural | X | 6.3, 4.7, 5.8 |
| SR23E | Bean Station, TN | Rural |  | 5.1, 4.9, 4.4 |
| SR152 | San Luis, CA | Rural |  | $4.9,4.9,5.9$ |
| SR87 | Payson, AZ | Rural |  | 5.8, 6.1, 5.9 |
| SR44 | Bernallilo, NM | Rural |  | 3.3, 3.4, 3.8 |
| US395 | Carson City, NV | Urban |  | 5.6, 5.7, 5.8 |

${ }^{1}$ For Traps 1 and 2, Traps 2 and 3, and at Final Climbing location

For this experiment, procedures were used by which individual trucks could be tracked thoughout their climb up the grade. Philosophically, the intent was to obtain samples of vehicle speed over the initial portion of the grade where the first 10 to 20 mph ( 16 to 32 $\mathrm{km} / \mathrm{h}$ ) was lost, and then catch the final climbing speed of the vehicle. No attempt was made to observe the actual entry speed into the grade (at the level tangent point), because it was desired that the trucks be under full power during all measurements. Thus, first measurements were obtained at a distance of 500 to $1,000 \mathrm{ft}$ ( 152 to 305 m ) up the grade, where the experimenters were assured that the engine was fully applied.

For reliability over these multi-week expeditions, tapeswitch speed traps were devised for the speed measurements in the initial portion of the grade. Radar was excluded at the entry region of the grade for fear that it would cause drivers (especially those at higher speeds) to voluntarily slow down. Radar was used for final climbing speed measurements (typically a mile further up the road) because driving patterns would not be influenced at this point.

A typical site layout is illustrated in figure 24. Three speed measurement traps were placed in the initial part of the grade. An instrumentation van was located at approximately the midpoint of the three traps. Wires connected each of the tapeswitches to a timer system located in the van. Each trap consisted of two tapeswitches placed 40 ft ( 12 m ) apart--far enough that measurement errors due to inaccuracies in placement were negligible, yet, not so far that other vehicles could interfere with the measurement. The traps were separated by a distance of 900 to 1000 ft ( 274 to 305 m ). Average grades between the traps were measured with a surveyor's transit. At a point much farther up the hill where grade was constant, and the vehicles appeared to be settled into a final climbing speed, an experimenter was stationed with a radar to measure that speed.

The data collection procedure specified that the first truck (a vehicle with at least one axle with dual wheels) entering the traps,

Figure 24.. Typical Site Layout.
when the experimenters were free to accept a vehicle, be taken. This was done to avoid biasing the data by the natural tendency to always take a larger truck when two choices are presented. The tapeswitch traps were "armed" as the truck approached, and the travel time through the trap was measured and recorded. The vehicle was visually tracked, and the time (speed) to travel across each of the subsequent traps was measured similarly. As the vehicle passed, the experimenters noted the type of vehicle (number of axles, number of units, and size) and color and make identification of the power unit. Figure 25 shows the data entered for each vehicle. Prominent identification features of the vehicle were listed in the description. The number of units established whether it was a truck, truck with trailer, tractor-semitrailer, double and triple combination. The gross body size (in front silhouette view) was indicated as maximum, intermediate, or minimum. The number of axles on each unit, and whether a trailer was long (generally over 30 ft [ 9 m]) or short was entered in the appropriate location. The descriptive information on each vehicle was transmitted via radio link to the observer in the final climbing area. When the vehicle passed that area, the final climbing speed was reported back on the radio and entered on the data sheet. Thus three speeds during the initial deceleration phase (derived from the times $T 1, T 2$, and $T 3$ ) and a final climbing speed ( $V_{S S}$ ) were measured for each truck, along with its identification and classification. With this procedure the same sample of trucks was always represented in measurements at each point on the grade.

Because of the length of grade required, at least two uphill lanes were present at nearly every site. As a consequence, some trucks (generally those with better hill-climbing capability) would take the left-hand lane precluding measurement. When time permitted, the experimenter at the uphill location would take a 100 percent classification sample for some period of the day to get an idea of the number of vehicles being missed in the measurements. Depending on location, the sampling captured from 60 to 90 percent of the trucks passing the site. There did not appear to be any strong bias in the distribution of trucks among classes as a result of those vehicles that were missed. Figure 26 shows the distribution of the total population


Figure 25. Data recording form used at the uphill measurement sites.


Figure 26 a . Total population and sampled population obtained at Bliss site.


Figure 26b. Total population and sampled population obtained at Carson City site.
by truck class passing the site and the distribution of the sampled vehicles for a rural interstate site in Idaho and an urban primary road site in Nevada. The coding on the abscissa identifies the vehicles by straight truck (STR), tractor-semitrailer (SEMI), doubles (DOUB) and triples (TRIP), with the number of axles indicated by the numeral following the abbreviation. The charts illustrate that the sample population very closely matched the total population by truck class. Comparing the two charts gives an overview of the way in which the types of trucks vary by location. Traffic on the rural interstate site is dominated by five-axle tractor-trailers, presumably representing long distance transport. The urban primary route was selected specifically because of the expectation of a different traffic mix in such locations, borne out by the high percentage of straight trucks seen in the chart.

Data were collected at each site until a total of 200 or more trucks were sampled, expecting to obtain a reasonable number in each truck class. Normally two long days were required at each site. When completed, all data were reviewed and checked for errors or inconsistencies. On all except the urban sites, tractor-trailers dominated the sampling numerically, with most of these of the five-axle type. Although the number of straight trucks sampled was marginal in many cases, no effort was made to alter this situation because of the desire to have a "random" sample at each site.

At some point in the test operations at a site, a site survey was made recording relevant geometric information about the site. The distances identifying the speed trap locations were recorded and a surveyor's transit was used to determine the average vertical angle between traps and at the top of the hill.

At the weigh scale sites, additional data was obtained. An observer was stationed at the scale to obtain the gross vehicle weight on all vehicles passing through. The observer inquired of the driver as to the engine horsepower, and noted the vehicle size, identification, types of tires (bias or radial) and what, if any, aerodynamic aids were present on the vehicle. At the end of each day the data sheets from the weigh scale and the measurements on grade were compared, and the
individual trucks were matched by identification and time. The procedure proved very successful, generally matching 90-95 percent of the vehicles. Thus for these sites, hill-climbing performance and truck weight and power data were available.

On return to UMTRI, the data were entered into computer files for subsequent processing and analysis.

## APPENDIX B

## SUMMARY OF FIELD DATA

The following pages provide a summary of the data on truck performance collected at the field sites. Each page covers a separate site, identified by name on the first line. The second line lists
a) The distance (in feet) between the first and second, and between the second and third speed measurement points, and
b) The grades (\%/100) in each of the first two deceleration intervals and at the final climbing point.

The first page for each site provides data summaries for three classes of vehicles--straight trucks, trucks with trailers, and tractortrailers. On the second page a summmary is provided for the various types of doubles and triples. The distinctions relate to whether the trailers are "long" ( 40 to 45 ft [13 to 14 m$]$ ) or "short" (27 to 28 ft [8 to 9 m ]). The classes are divided into $65-\mathrm{ft}$ doubles (a tractor with 2 short trailers), Rocky Mountain doubles (a long and a short trailer), turnpike doubles (2 long trailers), and triples (3 short trailers). Under each class the first group of information indicates the speeds ( $\mathrm{ft} / \mathrm{sec}$ and $\mathrm{mi} / \mathrm{h}$ ) at the $12.5 \%$ and median (50\%) level. The number in parenthesis is the number of data samples. The second summary group under each vehicle class is the calculated weight-to-power values, derived from the speeds compiled previously. If there was insufficient sample size to permit these calculations, the weight-to-power summary is omitted.

MILESEURG
906.896990 .8000
0.0326
0.0346
8.0296

| Trucke | No. | 12.5 Fercentile |  | Median |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Ft/sec | MPH | Ft/sec | MPH |
| Trapl | ( 37) | 56.5039 | 38.52539 | 69.26453 | 47.22582 |
| Trap 2 | ( 37) | 52.67663 | 35.91588 | 64 | 43.63637 |
| Trap 3 | ( 35.) | 50.1785 | 34.21261 | 63.33697 | 43.1843 |
| Fnl Clm | 35 ) | 46.01667 | 31.375 | 57.2 | 39 |

$12.5 \%$ Weight/Fower
At MPH of
Median Weight/Fower
At MPH of

Trieps 1-2 Traps 2-3 Fnl Clmbe $397.1622 \quad 354.7169412 .376$ $37.22064 \quad 35.06425 \quad 31.375$
$403.1846 \quad 260.7745 \quad 331.7512$ $45.43109 \quad 43.4103339$


12.5\% Weight/Power At MPH of
Median Weight/Fower At MPH of

Traps 1-2 Traps 2-3 Fnl Clmbe $351.5946 \quad 310.6978 \quad 386.2178$ $39.05193 \quad 37.68668 \quad 33.5$ $289.3399 \quad 275.0296 \quad 294.0522$ 51.9670149 .9523544


HAZELTON
$900.0000 \quad 900.0080 \quad 0.0244 \quad 0.0363$

| Trucks | No. | 12.5 | F | Medi an |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1 | 33 | 79.8279 | 48.29175 | 79.44397 | 54.16635 |
| Trap 2 | 33 | 69.87489 | 47.89651 | 76.41053 | 52.09809 |
| Traf 3 | 33 | 69.61345 | 47.46372 | 77.44441 | 52.80301 |
| Fol Clm | 33 | 54.81667 | 37.375 | 73.33334 | 50 |


| 12.5\% Weight/Power. |  | rafs 1-2 | 3018361 | 2760648 |
| :---: | :---: | :---: | :---: | :---: |
|  |  | 396.2682 | 301.8361 | 276.0648 |
| At MPH of |  | 47.69413 | 47.28012 | 37.375 |
| Median Weight |  | 435.184 | 257.7996 | 206.3585 |
| At MPH of |  | 53.13222 | 52.45055 | 50 |
| Trucks with trailers |  | Percentile | - Median |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1 --- (2) | $\theta$ | 0 | 77.66991 | 52.95675 |
| Trap 2--- (2) | 6 | 0 | 70.29877 | 47.93097 |
| Trip 3--- (2) | 0 | 0 | 72.85974 | 49.6771 |
| Fnl Clmbg-- 2 ) | 0 | 0 | 64.53333 | 44 |
| Tractor trailersNo. | 12.5 | Percentile | Median |  |
|  | Ft/sec | MF- | Ft/sec | MPH |
| Traf 1--- ( 162) | 76.33588 | 52.04719 | 84.38816 | 57.5374 |
| Trap 2 --- ( 164 ) | 72.99271 | 49.76776 | 79.68128 | 54.32814 |
| Trap 3 --- ( 162 ) | 70.29943 | 47.93143 | 77.82101 | 53.05978 |
| Fnl Cimbg- ( 159) | 46.93334 | 32 | 63.86667 | 43 |
|  |  | Traps 1-2 | Traps 2-3 | Fnl Clmbe |
| 12.5\% Weight/Power |  | 467.2667 | 418.7971 | 322.4351 |
| At MPH of |  | 50.90747 | 48.84959 | 32 |
| Median Weight/Power |  | 606.5193 | 350.3654 | 239.9517 |
| At MPH of |  | 55.93278 | 53.69396 | 43 |

hazeltan



```
    12.5\% Weight/Power
        At MPH of
Median Weight/Power
        At MPH of
```

                                    Trape 1-2 Traps 2-3 Fnl Clmbe
                                    \(926.7988 \quad 516.6955 \quad 357.3307\)
                                \(50.5374 \quad 46.98721 \quad 28.875\)
                                \(753.3721 \quad 448.1832 \quad 343.9308\)
                                \(54.58441 \quad 51.55688 \quad 30\)
    Racky Mountain Doutiles 12.5 Fercentile
No. $\quad$ Ft/sec MPH
Ft/sec Man MPH

Traf 1--- ( $\theta$ ) 0
Trap $2--(0)$
Trap 3 - ( 0 ) 0
Fil Cimber- (0) 0 0

| 0 | 0 |
| :--- | :--- |
| 0 | 0 |
| 0 | 0 |
| 0 | 0 |



| Triples | No. |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Trap 1 | (0) | 0 | 0 | 0 | 0 |
| Trap 2 | (0) | 0 | 0 | 0 | 0 |
| Trap 3 | (0) | 0 | 0 | 0 | 0 |
| Fnl Clmb | ( 0 ) | 0 | 0 | 0 |  |

WAYNESEGRO
$900.0060 \quad 900.00400 .02500 .0294039$

| Trucks | 12.5 |  | Percentile | Median |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap | ( 62 | 72.07486 | 49.14195 | 80 | 54.5454 |
| Trap 2 | 62 | 69.56521 | 47.43083 | 78.89546 | 53.7923 |
| Trap 3 | 61 | 62.56399 | 42.65727 | 76.84983 | 52.3976 |
|  | 60 | 38.86667 | 26.5 | 61.6 | 42 |

12.5\% Weight/Fower
At MPH of
Median Weight/Power
At MFH of

Traps 1-2 Trape 2-3 Fnl Clmbg $411.8731 \quad 620.1473 \quad 360.3099$ $48.28639 \quad 45.84405 \quad 26.5$ $315.2366 \quad 295.6627 \quad 227.3384$ $54.16891 \quad 53.0949842$

Trucks with trailers
12.5 Percentile Median Ne.
$\begin{array}{lllllll}\text { Trap } 1 & -0-(5) & 76.69241 & 52.29029 & 83.019389 & 56.65492\end{array}$
Trap 2 --- ( 5 ) $74.1177 \quad 50.5348 \quad 82.31708 \quad 56.12528$

| Traf $3---(4)$ | 70.95047 | 48.37532 | 80.32129 | 54.76452 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |

Fil Clmbg- (5) $44.91667 \quad 30.625 \quad 62.33334 \quad 42.50001$
12.5\% Weight/Power At MPH af
Median Weight/Power At MFH of

Traps 1-2 Traps 2-3 Fnl Clmbg $398.8082 \quad 353.3764 \quad 311.7784$ $51.41254 \quad 49.45506 \quad 30.625$ $292.033 \quad 284.3625 \quad 224.6638$ $56.3901 \quad 55.4449 \quad 42.50001$

Tractor trailers
12.5 Percentile

## Median

No. Ft/sec MPH Ft/sec MPH
Trap 1-2- (143) $75.02935 \quad 51.15637 \quad 81.54953 \quad 55.60195$
Trap 2 --- ( 143 ) $72.267849 .273578 .81781 \quad 53.73942$
Trap 3 --- ( 143 ) $67.9983846 .3625476 .78189 \quad 52.29674$
Fril Clmbg- ( 143 ) $39.6 \quad 27 \quad 52.8 \quad 36$
12.5\% Weight/Power

At MFH of
Median Weight/Power At MPH of

Traps 1-2 Traps 2-3 Fnl Clmbe $415.562 \quad 411.6065 \quad 353.6375$ $50.21494 \quad 47.81802 \quad 27$ $393.5223 \quad 298.3474 \quad 265.2282$ $54.67068 \quad 53.1180836$



| $\begin{aligned} & \text { WYTHEUILLE } \\ & \qquad 900.00000 \quad 900.0 \end{aligned}$ | 960.80800 | 0.03987 | 0.13957 | 0.03557 |
| :---: | :---: | :---: | :---: | :---: |
| 65 foot Doubles | 12.5 | Fercentile | Median |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- (1) | 9.615385 | 6.555945 | 38.46154 | 26.22378 |
| Trap $2--\infty$ ( 1 ) | 8.064516 | 5.498534 | 32.25807 | 21.99414 |
| Trap 3 --- ( 1 ) | 6.849315 | 4.669987 | 27.39726 | 18.67995 |
| Fril C.lmbg-- 1 ) | . 3666667 | . 25 | 1.466667 | 1 |
|  |  | Traps 1-2 | Traps 2-3 | Fril Clmbg |
| 12.5\% Weight/Power |  | 1579.358 | 1878.771 | 37906.73 |
| At MPH af |  | 6.027239 | 5.084261 | . 25 |
| Median Weight/Fower |  | 481.5878 | 533.4225 | 9476.682 |
| At MPH of |  | 24.10896 | 20.33704 | 1 |
| Rocky Mouritain Doubles 12.5 |  | Percentile | Median |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- ( ) | 0 | 0 | 0 | 0 |
| Trap 2 --- (a) | 0 | 0 | 0 | 0 |
| Traf $3-\cdots$ ( 0 ) | 0 | 0 | 0 | 0 |
| Fnl Clmbg--( 0 ) | 0 | 0 | 0 | 0 |
| Turnpike Doubles | 12.5 | Fercentile | Media |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- ( 0 ) | 0 | 0 | 0 | 0 |
| Trap 2 --- (0) | 0 | 0 | 0 | 0 |
| Trap 3 --- ( ) | 0 | 0 | 0 | 0 |
| Fril Clmbg-- (0) | 0 | 0 | 0 | 0 |
| Triples | 12.5 | Percentile | Media |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- ( 0 ) | 6 | 0 | 6 | 0 |
| Trap 2 --- ( $\operatorname{Tr}^{\text {a }}$ ) | 0 | 0 | 0 | 0 |
| Trap 3 --- ( 0 ) | 0 | 0 | 0 | 0 |
| Fnl Clmbg-- ( 0 ) | 0 | 0 | 0 | 0 |


| WHEELING |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| 1100.80080800 .0 | 0800 | 0.04653 | 0.85889 | 0.05040 |
| Trucks | $\begin{aligned} & 12.5 \\ & \mathrm{Ft} / \mathrm{sec} \end{aligned}$ | Percentile | Median |  |
|  |  | MPH | Ft/sec | MPH |
| Trap 1--- (11) | 62.50947 | 42.62009 | 72.27646 | 49.27941 |
| Trap 2 --- (12) | 43.34057 | 29.55039 | 62.1311 | 42.36211 |
| Trap 3--- (12) | 27.33659 | 18.63859 | 55.55556 | 37.87879 |
| Fnl Clmbg--( 12 ) | 35.93334 | 24.5 | 46.93334 | 32 |
|  |  | Traps 1-2 | Traps 2-3 | Fnl Clmbg |
| 12.5\% Weight/Power |  | 581.0395 | 537.967 | 306.1433 |
| At MPH of |  | 36.08524 | 24.09449 | 24.5 |
| Median Weight/Power |  | 300.0188 | 260.6116 | 234.3909 |
| At MPH of |  | 45.82076 | 40.12045 | 32 |
| Trucks with trailers 12.5 |  | Percentile | Median |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- ( 3 ) | 0 | 0 | 66.78969 | 45.53843 |
| Trap 2 --- ( 3 ) | 8 | 0 | 57.71606 | 39.35186 |
| Trap 3--w ( 3) | 0 | 0 | 49.39738 | 33.68043 |
| Fnl Clmbg--( 3) | 0 | 0 | 44.73334 | 30.5 |
| Tractor trailers No. | $\begin{aligned} & 12.5 \\ & F t / s^{2} \end{aligned}$ | $\begin{gathered} \text { Percentile } \\ \text { MPH } \end{gathered}$ | Medi an |  |
| Trap 1--- ( 155) | 66.65985 | 45.4499 | 75.25166 | 51.30795 |
| Trap 2 --- ( 168 ) | 50.88412 | 34.69372 | 59.04931 | 40.26089 |
| Trap 3--- (170) | 41.12479 | 28.03963 | 51.24264 | 34.93816 |
| Fnl Clmbg-- 161) | 35.2 | 24 | 44 | 30 |
|  |  | Traps 1-2 | Traps 2-3 | Fnl Cimbg |
| 12.5\% Weight/Power |  | 459.8351 | 357.341 | 312.5213 |
| At MPH of |  | 40.87181 | 31.36667 | 24 |
| Median Weight/Power |  | 518.0385 | 291.8515 | 250.017 |
| At MPH of |  | 45.78442 | 37.59952 | 30 |

WHEELING

$$
\begin{array}{llll}
800.00000 & 0.04653 & 0.05089 & 0.05000
\end{array}
$$






| CHEAT LAKE |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Trucks No. | 12.5 | Percentile | - Median |  |
|  | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- ( 49) | 59.58862 | '40.6286 | 77.59514 | 52.90578 |
| Trap 2 --- ( 48 ) | 48.93565 | 33.36522 | 69.61365 | 47.46385 |
| Trap 3--- (49) | 40.871 | 27.86659 | 64.77839 | 44.16709 |
| Fnl Clmbg--( 49) | 35.2 | 24 | 59.4 | 40.5 |
|  |  | Traps 1-2 | Traps 2-3 | Fil Clmbg |
| 12.5\% Weight/Power |  | 266.5348 | 255.2251 | 255.9627 |
| At MPH of |  | 36.99691 | 30.6159 | 24 |
| Median Weight/Power |  | 198.4503 | 164.9567 | 151.6816 |
| At MPH of |  | 50.18482 | 45.81547 | 40.5 |
| Trucks with trailers 12.5 |  | Percentile Median |  |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1 --- ( 6 ) | 62.93663 | 42.91134 | 77.33953 | 52.7315 |
| Trap $2--\infty$ ( 6) | 53.89187 | 36.74446 | 57.92904 | 39.49707 |
| Trap 3--- (6) | 34.57122 | 23.57128 | 49.51721 | 33.76174 |
| Fnl Clmbg--( 6) | 6.049999 | 4.124999 | 44 | 30 |
| 12.5\% Weight/Power |  | Traps 1-2 Traps 2-3 |  | Fnl Clmbg 1489.238 |
|  |  | 235.341 | 470.1 |  |
| At MPH of |  | 39.8279 | 30.15787 | 4.124999 |
| Median Weight/Power |  | 926.8251 | 232.3361 | 204.7702 |
| At MPH of |  | 46.11428 | 36.6294 | 30 |
| Tractor trailersNo. 12.5Traper |  | $\begin{aligned} & \text { Percentile } \\ & \text { MPH } \end{aligned}$ | Median |  |
|  |  | Ft/sec | MPH |  |
| Trap 1 - - ( 153) | 68.37914 |  | 46.62214 | 78.42368 | 53.47869 |
| Trap 2 --- (158) | 54.83979 | 37.39077 | 66.78911 | 45.53803 |
| Trap $3---(159)$ | 43.24961 | 29.48837 | 55.47923 | 37.82675 |
| Fnl Clmbg--( 158 ) | 33.36667 | 22.75 | 45.46667 | 31 |
| 12.5\% Weight/Power At MPH of |  | Traps $1-2$320.7522 | Traps 2-3 Fnl Clmbg |  |
|  |  | 287.788 | 270.0266 |  |
|  |  | 42.08646 | 33.43957 | 22.75 |
| Median Weight/Power |  |  | 276.359 | 267.854 | 198.1647 |
| At MPH of |  | 49.50436 | 41.68239 | 31 |



| BLISS |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| 1000.000081080 .00080 |  | 0.83106 | 0.84030 | 0.84830 |
| Trucks | $\begin{aligned} & 12.5 \\ & \mathrm{Ft} / \mathrm{sec} \end{aligned}$ | Percentile | Median |  |
| No. |  | MPH | Ft/sec | MPH |
| Trap 1--- (15) | 68.12126 | 46.44631 | 82.66721 | 56.36401 |
| Trap 2 --- (15) | 62.92365 | 42.98249 | 78.81781 | 53.73942 |
| Trap 3-m ( 15 ) | 54.26264 | 36.99725 | 75.40599 | 51.41318 |
| Fnl Cimbg--( 14 ) | 50.6 | 34.5 | 71.86667 | 49 |
|  |  | Traps 1-2 | Traps 2-3 | Fnl Clmbg |
| 12.5\% Weight/Power |  | 409.8632 | 382.4686 | 269.6948 |
| At MPH of |  | 44.6744 | 39.94987 | 34.5 |
| Median Weight/Fower |  | 318.2021 | 221.9702 | 189.8871 |
| At MPH of |  | 55.85171 | 52.5763 | 49 |
| Trucks with trailers |  | Percentile | Median |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- (12) | 74.66401 | 50.90728 | 87.14598 | 59.41771 |
| Trap $2---(12)$ | 66.52295 | 45.35656 | 82.81574 | 56.46528 |
| Trap 3--- (12) | 56.30976 | 38.39301 | 77.97271 | 53.16321 |
| Fnl Clmbg--( 12 ) | 44 | 30 | 71.86667 | 49 |
|  |  | Traps 1-2 | Traps 2-3 | Fil Cimbg |
| 12.5\% Weight/Power |  | 589.717 | 430.0626 | 310.149 |
| At MPH of |  | 48.13191 | 41.87478 | 30 |
| Median Weight/Power |  | 329.6783 | 242.4994 | 189.8871 |
| At MPH of |  | 57.94149 | 54.81425 | 49 |
| Tractor trailers | 12.5 | Percentile | Medi a |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- (199) | 78.23975 | 53.34528 | 85.83691 | 58.52517 |
| Trap 2 --- ( 204 ) | 73.19311 | 49.90439 | 81.63265 | 55.65862 |
| Trap 3--- ( 200 ) | 64.41224 | 43.91744 | 74.62686 | 50.88195 |
| Fnl Clmbg--( 201 ) | 50.85 | 34.125 | 63.06667 | 43 |
|  |  | Traps i-2 | Traps 2-3 | Fnl Clmbg |
| 12.5\% Weight/Power |  | 378.4744 | 371.1026 | 272.6584 |
| At MPH of |  | 51.62484 | 46.91091 | 34.125 |
| Median Weight/Power |  | 326.3526 | 302.07 | 216.383 |
| At MPH of |  | 57.89191 | 53.27029 | 43 |


| ELISS |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| 1000.000001000 .00000 |  | 0.03106 | 0.04030 | 0. 04030 |
| 65 foot Doubles | $\begin{gathered} 12.5 \\ \mathrm{Ft} / \mathrm{sec} \end{gathered}$ | Percentile | Median |  |
|  |  | MPH | $\mathrm{Ft} / \mathrm{sec}$ | MFH |
| Trap 1--- ( 12) | 79.05138 | 53.89867 | 83.85745 | 57.17553 |
| Trap 2 --- ( 12 ) | 73.66483 | 50.22602 | 80.16032 | 54.65476 |
| Trap 3--- (12) | 60.78705 | 41.44571 | 69.56521 | 47.43083 |
| Fnl Clmbg--( 12 ) | 46.2 | 31.5 | 55.73334 | 36 |
| 12.5\% Weight/Power |  | Trafs 1-2 | Traps 2-3 Fnl Clmbg |  |
|  |  | 393.9014 | 609.7542 | 295.38 |
| At MPH of |  | 52.06235 | 45.83587 | 31.5 |
| Median Weight/Fower |  | 309.8662 | 468.8331 | 244.8545 |
| At MPH of |  | 55.91515 | 51.04279 | 38 |
| Rocky Mountain Doubles 12.5 |  | Fercentile | Median |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- ( 3 ) | 33.18584 | 22.62671 | 90.33073 | 61.58914 |
| Trap 2 --- ( 3 ) | 28.03738 | 19.1164 | 79.84985 | 53.89762 |
| Trap 3--- ( 3 ) | 25.12563 | 17.13111 | 74.5686 | 50.84223 |
| Fnl Clmbg--( 3 ) | 20.9 | 14.25 | 68.2 | 46.5 |
|  |  | Traps 1-2 | Traps 2-3 | Fnl Clmbg |
| 12.5\% Weight/Power |  | 686.6801 | 545.9456 | 652.9452 |
| At MPH of |  | 20.87155 | 18.12375 | 14.25 |
| Median Weight/Power |  | 4674.376 | 241.7997 | 200.0961 |
| At MPH of |  | 57.74338 | 52.36992 | 46.5 |
| Turnpike Doutiles | 12.5 | Percentile | Medi a |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- (0) | 0 | 0 | 0 | 0 |
| Trap 2 --- ( 0 ) | 0 | 0 | 8 | 0 |
| Trap 3 --- ( 0 ) | 0 | 0 | 0 | 0 |
| Fnl Clmbg--( 0 ) | 0 | 0 | 0 | 0 |
| Triples | 12.5 | Percentile | Medi a |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- (1) | 9.363296 | 6.384066 | 37.45319 | 25.53626 |
| Trap 2 --- (1) | 8.417509 | 5.739211 | 33.67084 | 22.95684 |
| Trap 3--- ( 1 ) | 6.887052 | 4.695718 | 27.54821 | 18.78287 |
| Fril Clmbg--( 1 ) | 8.983334 | 6.125 | 35.93334 | 24.5 |
|  |  | Traps 1-2 | Traps 2-3 | Fnl Clmbg |
| 12.5\% Weight/Power |  | 2098.688 | 1799.572 | 1519.097 |
| At MPH af |  | 6.061639 | 5.217464 | 6.125 |
| Median Weight/Power |  | 575.3448 | 521.0701 | 379.7742 |
| At MPH of |  | 24.24655 | 20.86986 | 24.5 |




| WELLS |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| 880.00180 1980.00 | 08908 | 0.85350 | 0.04681 | 0.85263 |
| Trucks No. | $\begin{aligned} & 12.5 \\ & \mathrm{Ft} / \mathrm{sec} \end{aligned}$ | Percentile | Median |  |
|  |  | MPH | Ft/sec | MPH |
| Trap 1--- ( 28 ) | 57.51927 | 39.21768 | 78.58546 | 53.581 |
| Trap 2 --- ( 28 ) | 54.20542 | 36.95824 | 73.5294 | 50.13368 |
| Trap 3--- ( 28 ) | 46.02803 | 31.37729 | 68.72851 | 46.86835 |
| Fnl Clmbg--( 27 ) | 38.68334 | 26.375 | 56.46667. | 38.5 |
|  |  | Traps 1-2 | Traps 2-3 | Fnl Clmbg |
| 12.5\% Weight/Power |  | 209.6298 | 322.16 | 270.1586 |
| At MPH of |  | 38.88796 | 34.16777 | 26.375 |
| Median Weight/Power |  | 181.1075 | 213.5938 | 185.0762 |
| At MPH of |  | 51.85735 | 48.49781 | 38.5 |
| Trucks with trailers 12 |  | Percentile | Median |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1-m ( 17 ) | 43.81882 | 29.87647 | 65.83072 | 44.88458 |
| Trap $2--\infty$ ( 18) | 39.47213 | 26.91281 | 62.3053 | 42.48988 |
| Trap 3--- (18) | 31.45032 | 21.4434 | 50.25126 | 34.26222 |
| Fnl Clmbg--( 18 ) | 28.23333 | 19.25 | 35.2 | 24 |
| 12.5\% Weight/Power |  | Traps 1-2 Traps 2-3 |  | Fnl Clmbg |
|  |  | 280.3295 | 408.4522 | 370.1524 |
| At MPH of |  | 28.39464 | 24.17811 | 19.25 |
| Median Weight/Power |  | 188.5541 | 379.6923 | 296.893 |
| At MPH of |  | 43.68273 | 38.37155 | 24 |
| Tractor trailers $\quad 12.5$ |  | Percentile Median |  |  |
| Trap 1--- ( 148 ) | Ft/sec | MPH | Ft/sec | MPH |
|  | 61.58675 | 41.99096 | 72.07207 | 49.14005 |
| Trap 2 --- ( 148 | 56.25924 | 38.35857 | 66.88963 | 45.60657 |
| Trap 3--- (148) | 46.82999 | 31.38408 | 58.65183 | 39.98934 |
| Fnl Cimbg--( 148 ) | 35.2 | 24 | 44 | 30 |
| 12.5\% Weight/Power |  | Traps 1-2 Traps 2-3 |  | Fnl Clmbg |
|  |  | 220.835 | 351.9827 | 296.893 |
| At MPH of |  | 48.17477 | 34.87132 | 24 |
| Median Weight/Power |  | 194.0528 | 284.98 | 237.5145 |
| At MPH of |  | 47.37331 | 42.79796 | 30 |


| WELLE |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| 880.000001000 .00000 |  | 0.05350 | 0.04681 | 0.05263 |
| 65 foot Doutles | 12.5 | Fercentile | - Median |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- (5) | 37.03704 | 25.25253 | 60.44282 | 41.21112 |
| Trap 2 --- (5) | 31.84713 | 21.71396 | 56.46206 | 38.49686 |
| Trap 3--- ( 5 ) | 26.65245 | 18.17213 | 45.70552 | 31.16286 |
| Fnl Cimbg-- 5 ) | 20.16667 | 13.75 | 33.73333 | 23 |
|  |  | Traps 1-2 | Trape 2-3 | Fnl Cimbg |
| 12.5\% Weight/Fower |  | 338.3839 | 446.7688 | 518.2134 |
| At MPH of |  | 23.48324 | 19.94304 | 13.75 |
| Mediarı Weight/Power |  | 207.7667 | 362.0021 | 309.8015 |
| At MPH of |  | 39.85394 | 34.82986 | 23 |
| Rocky Mountain Doubles |  | Percentile | - Median |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- (5) | 26.09604 | 17.79275 | 46.48726 | 31.69586 |
| Trap 2 --- (5) | 20.81599 | 14.19272 | 40.59251 | 27.676 .71 |
| Trap 3 --- (5) | 17.61884 | 12.0123 | 30.56118 | 20.83716 |
| Fnl Clmbg--( 5) | 15.58333 | 10.625 | 25.66667 | 17.5 |
|  |  | Traps 1-2 | Traps 2-3 | Fril Clmbg |
| 12.5\% Weight/Power |  | 477.2769 | 637.4541 | 670.629 |
| - At MPH of |  | 15.99273 | 13.10251 | 10.625 |
| Median Weight/Power |  | 284.237 | 432.7567 | $44^{47.1676}$ |
| At MPH of |  | 29.68628 | 24.25694 | 17.5 |
| Turnpike Doutiles | 12.5 | Percentile | Media |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- ( ${ }^{\text {P }}$ ) | 0 | 0 | 0 | $\square$ |
| Trap 2 --- ( $\theta$ ) | 0 | 0 | 0 | 0 |
| Trap 3--- (0) | 0 | 0 | 0 | 0 |
| Fnl Clmbg-- 0 ) | 0 | 0 | 0 | 0 |
| Triples | 12.5 | Percentile | Media |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- (10) | 38.55382 | 26.28669 | 50.63291 | 34.52244 |
| Trap $2---(10)$ | 30.7995 | 20.99966 | 41.58004 | 28.35003 |
| Trap 3--- (10) | 26.73069 | 18.22547 | 32.38867 | 22.08318 |
| Fnl Clmbg--(10) | 23.46667 | 16 | 27.86667 | 19 |
|  |  | Traps 1-2 Tr | Traps 2-3 | Fnl Cimbg |
| 12.5\% Weight/Power |  | 360.3889 | 442.8904 | 445.3396 |
| At MPH of |  | 23.64318 | 19.61256 | 16 |
| Median Weight/Power |  | 307.6879 | 410.2435 | 375.0228 |
| At MPH of |  | 31.43624 | 25.2166 | 19 |


| COYOTE |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Trucks | 12.5 | Percentile | - Medi |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- (55) | 56.49471 | 38.51912 | 70.05255 | 47.7631 |
| Trap 2 --- (75) | 47.29577 | 32.24712 | 64.62053 | 44.05945 |
| Trap 3--- (75) | 46.01877 | 31.37998 | 61.20891 | 41.73335 |
| Fnl Clmbg-- 73 ) | 45.65 | 31.125 | 60.13334 | 41 |
|  |  | Traps 1-2 | Traps 2-3 | Fnl Clmbg |
| 12.5\% Weight/Power |  | 295.5514 | 230.5493 | 203.1667 |
| At MPH of |  | 35.38312 | 31.80905 | 31.125 |
| Median Weight/Power |  | 205.6867 | 190.8859 | 154.2333 |
| At MPH of |  | 45.91127 | 42.8964 | 41 |
| Trucks with trailers |  | Percentile | Median |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- (16) | 45.45455 | 30.99174 | 58.73715 | 46.04806 |
| Trap $2---(22)$ | 36.98432 | 25.21658 | 48.48485 | 33.05785 |
| Trap 3--- ( 22 ) | 32.95428 | 22.46883 | 48.48485 | 33.05785 |
| Fnl Cimbg--( 22 ) | 32.26667 | 22 | 39.6 | 27 |
| 12.5\% Weight/Power |  | Traps 1-2 Traps 2-3 |  |  |
|  |  | $331.2242$ | $\begin{array}{cc}\text { Traps 2-3 Fni Clmbg } \\ 325.36 & 287.4347\end{array}$ |  |
| At MPH of |  | 28.10416 | 23.84271 | 22 |
| Median Weight/Power |  | 307.4709 | 213.2144 | 234.2061 |
| At MPH of |  | 36.55295 | 33.85785 | 27 |
| Tractor trailersNo:Trapctsec |  | Percentile <br> Medi |  |  |
| Trap 1--- (69) | 46.78459 | 31.89858 | 63.59445 | 43.35985 |
| Trap $2 \cdots-$ ( 85) | 37.55592 | 25.60631 | 56.89947 | 38.79509 |
| Trap 3--- ( 83) | 35.0685 | 23.91834 | 52.39051 | 35.7288 |
| Fnl Clmbg--( 85 ) | 35.2 | 24 | 52.8 | 36 |
|  |  | Traps 1-2 | Traps 2-3 | Fnl Clmbg |
| 12.5\% Weight/Power |  | 335.2536 | 302.4039 | 263.4818 |
| At MPH of |  | 28.75244 | 24.75832 | 24 |
| Median Weight/Power |  | 237.6514 | 225.1602 | 175.6545 |
| At MPH of |  | 41.07747 | 37.25795 | 36 |



| DENUER |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| 600.00098600 .0 | 0808 | 0.04623 | 0.85930 | 0.06157 |
| Trucks No. | 12.5 | Percentile | Median |  |
|  | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- (71) | 61.04326 | 41.6204 | 76.42343 | 52.18689 |
| Trap $2---(74)$ | 56.08142 | 38.23733 | 72.20216 | 49.22875 |
| Trap 3--- (73) | 51.08243 | 34.82892 | 65.28607 | 44.51323 |
| Fnl Clmbg--( 71 ) | 39.41667 | 26.875 | 52.8 | 36 |
|  |  | Traps 1-2 | Traps 2-3 | Fnl Clmbg |
| 12.5\% Weight/Power |  | 301.1017 | 225.9054 | 226.6405 |
| At MPH of |  | 39.92887 | 36.53313 | 26.875 |
| Median Weight/Power |  | 246.7495 | 230.6119 | 169.1934 |
| At MPH of |  | 50.66781 | 46.87899 | 36 |
| Trucks with trailers | 12.5 | Percentile | Median |  |
|  | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- (2) | 8 | 0 | 52.88333 | 35.51137 |
| Trap 2 --- (2) | 0 | 0 | 45.83477 | 31.25098 |
| Trap 3--- ( 2 ) | 0 | 0 | 35.46099 | 24.17795 |
| Fril Clmbg--( 1 ) | 0 | 0 | 18.33333 | 12.5 |
| Tractor trailersNo. | 12.5 | Percentile | Median |  |
|  | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- ( 121) | 51.82939 | 34.79276 | 64.31931 | 43.85407 |
| Trap 2 --- ( 125) | 44.11364 | 30.07748 | 58.17495 | 39.66474 |
| Trap 3--- ( 126) | 34.49547 | 23.51964 | 51.9548 | 35.42373 |
| Fril Clmbg--( 125 ) | 29.33334 | 28 | 39.6 | 27 |
|  |  | Traps 1-2 | Traps 2-3 | Fnl Clmbg |
| 12.5\% Weight/Power |  | 395.9861 | 352.1668 | 304.5481 |
| At MPH of |  | 32.43512 | 26.79856 | 20 |
| Median Weight/Power |  | 335.6655 | 240.2537 | 225.5912 |
| At MPH of |  | 41.7594 | 37.54423 | 27 |


| DENUER |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| 600.06090600 .00 | 600.60000 | 0.04623 | 0.05930 | 0.06157 |
| 65 foot Doubles | 12.5 | Percentile | Median |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- (1) | 7.654624 | 5.219062 | 30.6185 | 20.87625 |
| Trap $2--\infty$ ( 1 ) | 7.144899 | 4.871522 | 28.57959 | 19.48609 |
| Traf 3--- ( 1) | 6.624271 | 4.516548 | 26.49708 | 18.06619 |
| Fnl Cimbg-- 1 ) | 5.683334 | 3.875 | 22.73333 | 15.5 |
| 12.5\% Weioht/Power |  | Traps $1-2$ 1614.531 | Traps $2-3$ 1351.377 | Fnil Cimbg |
| At MPH of |  | 5.045292 | 4.694035 | 3.875 |
| Median Weight/Power |  | 431.0529 | 354.5334 | 392.9653 |
| At MPH of |  | 21.18117 | 18.77614 | 15.5 |
| Rocky Mountain Doubles 12.5 Percentile No. $\mathrm{Ft} / \mathrm{sec} \mathrm{MPH}$ |  |  | Median |  |
|  |  |  | Ft/sec | MPH |
| Trap 1--- (0) | 0 | 0 | 0 | 0 |
| Trap $2-\cdots$ ( ${ }^{\text {P }}$ ) | 0 | 0 | 0 | 0 |
| Trap 3--- (0) | 0 | 0 | 0 | (1) |
| Fnl Clmbg-s 0) | 0 | 0 | 0 | 0 |
| Turnpike Doubles No. | $\begin{gathered} 12.5 \\ \mathrm{Ft} / \mathrm{sec}^{2} \end{gathered}$ | $\begin{aligned} & \text { Fercentile } \\ & \text { MPH } \end{aligned}$ | Medi <br> Ft/sec | MPH |
| Trap 1---(0) | 0 | 0 | $\square$ | 0 |
| Trap 2 --- ( ${ }^{\text {( }}$ ) | 0 | 0 | 0 | 0 |
| Trap 3--- (0) | 0 | 0 | 0 | 0 |
| Fnl Clmbg-- ( ) | 0 | 0 | 0 | 0 |
| Triples No. | $\begin{array}{r} 12.5 \\ \mathrm{Ft} / \mathrm{sec} \end{array}$ | Percentile MPH | Medi <br> Ft/sec | ${ }^{\text {MPH }}$ |
| Trap 1--- ( ${ }^{\text {( }}$ ) | 0 | 0 | 0 | 0 |
| Trap 2 --- (0) | 0 | 0 | 0 | 0 |
| Trap 3--- (0) | 0 | 0 | 0 | 0 |
| Fnl Clmbg-- 0 ) | 0 | 0 | 0 | 0 |


| TRINIDAD |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| 1108.00080908 .0 | 908.00808 | 0.84506 | 0.05176 | 0.06395 |
| Trucks | 12.5 | Percentile | Median |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- ( 25 ) | 54.17096 | 36.93475 | 66.32446 | 45.22123 |
| Trap 2 --- ( 27 ) | 47.08813 | 32.10554 | 62.01496 | 42.28293 |
| Trap 3--- ( 26 ) | 40.45801 | 27.57955 | 54.10523 | 36.88993 |
| Fnl Clmbg--( 26 ) | 33.73333 | 23 | 42.53333 | 29 |
| 12.5\% Weight/Power |  | Traps 1-2 Traps 2-3 |  | Fnl Clmbg |
|  |  | $318.9194$ | 301.1158 | $254.9434$ |
| At MPH of |  | 34.52014 | 29.84255 | 23 |
| Median Weight/Power |  | 230.0586 | 263.7954 | 202.1964 |
| At MPH of |  | 43.75208 | 39.58643 | 29 |
| Trucks with trailers |  | Percentile | Median |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- (19) | 33.59439 | 22.90527 | 60.41962 | 41.1952 |
| Trap $2--\infty$ ( 20 ) | 31.30498 | 21.3443 | 52.9661 | 36.11325 |
| Trap 3--- (19) | 26.11132 | 17.88317 | 49.26116 | 33.58715 |
| Fnl Clmbg-- 18 ) | 27.5 | 18.75 | 39.6 | 27 |
|  |  | Traps 1-2 | Traps 2-3 | Fnl Clmbg 312.7305 |
| 12.5\% Weight/Power |  | 394.4835 | 411.0168 |  |
| At MPH of |  | 22.12479 | 19.57374 | 18.75 |
| Median Weight/Power |  | 292.7995 | 237.9429 | 217.174 |
| At MPH of |  | 38.65422 | 34.8502 | 27 |
| Tractor trailersNo.Trape |  | Percentile | Median |  |
|  |  | MPH | Ft/sec | MPH |
| Trap 1--- ( 105) | 51.72386 | 35.26627 | 67.82811 | 45.76099 |
| Trap $2--\infty(138)$ | 42.52519 | 28.99445 | 58.38984 | 39.75617 |
| Trap 3--- ( 136) | 36.29764 | 24.74839 | 50.71637 | 34.57934 |
| Fnl Clmbg--( 137 ) | 29.33334 | 20 | 38.13334 | 26 |
|  |  | Traps 1-2 | Traps 2-3 | Fil Clmbg |
| 12.5\% Weight/Power |  | 355.5594 | 322.3852 | 293.1848 |
| At MPH of |  | 32.13036 | 26.87142 | 20 |
| Median Weight/Power |  | 296.1312 | 269.2313 | 225.5268 |
| At MPH of |  | 42.72858 | 37.16775 | 26 |

TRINIDAD
$1100.00000 \quad 900.00000 \quad 0.04506 \quad 0.05176 \quad 0.0395$


| 12.5\% Weight/Power | 313.365 | 360.7061 | 366.4811 |
| ---: | :--- | :--- | :--- |
| At MPH of | 24.34549 | 23.5513 | 16 |
| Median Weight/Power | 345.5188 | 325.2556 | 325.7609 |
| At MPH of | 32.53161 | 27.23847 | 18 |



| e Doubles No. | $\begin{gathered} 12.5 \\ \mathrm{Ft} / \mathrm{sec} \end{gathered}$ | Percenti MPH | $\begin{gathered} \text { Medi } \\ \mathrm{Ft} / \mathrm{sec} \end{gathered}$ | MPH |
| :---: | :---: | :---: | :---: | :---: |
| Trap 1 --- (0) | 0 | 0 | 0 | 0 |
| Trap 2 --- (1) | 7.434944 | 5.06928 | 29.73978 | 20.27712 |
| Trap 3 --- ( 1) | 6.989097 | 4.765294 | 27.95639 | 19.06118 |
| Fril Clmber-( 1 ) | 5.683334 | 3.875 | 22.73333 | 15.5 |



| $\begin{array}{ll} \text { BEAN STATION } \\ 908.00000 & 900.00000 \end{array}$ |  | 0.05889 | 0.84897 | 0.04362 |
| :---: | :---: | :---: | :---: | :---: |
| Trueks | 12.5 | Percentile | Median |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- ( 47 ) | 63.01333 | 42.96364 | 73.39672 | 50.04322 |
| Trap 2 --- (47) | 60.88595 | 41.51315 | 71.30148 | 48.61464 |
| Trap 3--- ( 48 ) | 53.76344 | 36.65689 | 67.34808 | 45.91369 |
| Fnl Clmbg--( 49 ) | 47.3 | 32.25 | 61.6 | 42 |
| 12.5\% Weight/Power |  | Traps 1-2 Traps 2-3 |  | Fnl Clmbe 266.5962 |
|  |  | 191.5966 | 275.6762 |  |
|  |  | 42.23839 | 39.08502 | 32.25 |
| Median Weight/Power |  | 166.5114 | 286.9025 | 284.7078 |
| At MPH of |  | 49.32893 | 47.26416 | 42 |
| Trucks with trailers 12 |  | $\begin{aligned} & \text { Percentile } \\ & \text { MPH } \end{aligned}$ | Median |  |
| No. | Ft/sec |  | Ft/sec | MPH |
| Trap 1--- (2) | 0 | 0 | 56.65723 | 38.62993 |
| Trap 2--- ( 2 ) | 0 | 0 | 58.30964 | 39.75617 |
| Trap 3--- (2) | 0 | 0 | 56.25879 | 38.35827 |
| Fnl Cimbg--( 2 ) | 0 | 0 | 45.46667 | 31 |
| Tractor trailers No. | $\begin{aligned} & 12.5 \\ & \mathrm{Ft} / \mathrm{sec} \end{aligned}$ | $\begin{gathered} \text { Percentile } \\ \text { MPH } \end{gathered}$ | Median |  |
| Trap 1 --- ( 154 ) | 61.09269 | 41.65411 | 72.85974 | 49.6771 |
| Trap 2 --- ( 158 ) | 55.02094 | 37.51428 | 68.96551 | 47.02194 |
| Trap 3-- ( 150 ) | 44.15028 | 30.18246 | 61.8687 | 41.63775 |
| Fnl Clmbg- ( 156) | 35.2 | 24 | 49.86667 | 34 |
|  |  | Traps 1-2 | Traps 2-3 | Fnl Clmbg |
| 12.5\% Weight/Power |  | 244.6553 | 365.2485 | 358.2386 |
| At MPH of |  | 39.58419 | 33.80837 | 24 |
| Median Weight/Power |  | 187.5404 | 270.6845 | 252.8743 |
| At MPH of |  | 48,34952 | 44.32985 | 34 |

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Focky Mountain Doutles 12.5 Fercentile No. $\mathrm{Ft} / \mathrm{sec} \mathrm{MPH}$
Trap 1 --- ( 1 ) $10.204486 .95732840 .81633 \quad 27.82931$
Trap 2--- (1) 9.920635 6.764069 $39.68254 \quad 27.05628$
Trap 3 --- ( 0 ) 0
Fnl Clmbg--( 1 ) 7.7
5.25
30.8

Median



| DUNCANSUILLE |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| 750.00090750 .0 | 08008 | 0.84653 | 0.05813 | 0.04942 |
| Trucks No | $\begin{aligned} & 12.5 \\ & \mathrm{Ft} / \mathrm{sec} \end{aligned}$ | Percentile | Median |  |
|  |  | MPH | Ft/sec | MPH |
| Trap 1 --- (68) | 61.81361 | 42.14564 | 75.95899 | 51.79822 |
| Trap 2 --- (71) | 50.65355 | 34.53651 | 69.77153 | 47.5715 |
| Trap 3 --- ( 72 ) | 42.58491 | 29.03516 | 66.313 | 45.21341 |
| Fnl Clmbg--(68) | 37.4 | 25.5 | 63.06667 | 43 |
|  |  | Traps 1-2 | Traps 2-3 | Fnl Cimbg |
| 12.5\% Weight/Power |  | 476.1432 | 277.2082 | 297.561 |
| At MPH of |  | 38.34108 | 31.78584 | 25.5 |
| Median Weight/Power |  | $270.9427$ | $167.8417$ | 176.4606. |
| At MPH of |  | 49.68086 | $46.39246$ | 43 |
| Trucks with trailers | 12.5 | Percentile | Median |  |
|  | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- ( 4 ) | 52.55842 | 35.82984 | 61.51953 | 41.94514 |
| Trap $2---(4)$ | 41.34476 | 28.18961 | 49.42543 | 33.69916 |
| Trap 3--- ( 4) | 38.04871 | 25.93685 | 42.14519 | 28.73535 |
| Fnl Clmbg--( 4 ) | 30.06667 | 20.5 | 36.66667 | 25 |
| 12.5\% Weight/Power |  | Traps 1-2 Traps 2-3 |  | Fnl Clmbg |
|  |  | $473.4579$ | 262.9094 | 370.1369 |
| At MPH of |  | 32.88972 | 27.06323 | 20.5 |
| Median Weight/Power |  | 528.8588 | 270.9667 | 303.5123 |
| At MPH of |  | 37.82215 | 31.21726 | 25 |
| Tractor trailers | 12.5 | Percentile | Medi |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- (125) | 53.28272 | 36.32913 | 70.67769 | 48.18933 |
| Trap 2 --- ( 130 ) | 40.65626 | 27.72017 | 63.22112 | 43.10531 |
| Trap 3--- (133) | 32.43278 | 22.11326 | 59.16914 | 40.3426 |
| Fnl Clmbg--( 130 ) | 32.26667 | 22 | 49.86667 | 34 |
|  |  | Traps 1-2 | Traps 2-3 | Fnl Clmbg |
| 12.5\% Weight/Power |  | 532.9773 | 329.3915 | 344.9003 |
| At MPH of |  | 32.82465 | 24.91672 | 22 |
| Median Weight/Power |  | 317.723 | 187.762 | 223.1708 |
| At MPH of |  | 45.64732 | 41.72395 | 34 |



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980.89800
900.08090
0.84733
0.04933
0.04993

| Trucks | No. | 12.5 | Percentile | Median |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- | ( 124) | 63.95085 | 43.60285 | 78.58546 | 53.581 |
| Trap 2 --- | ( 135 ) | 55.43074 | 37.79369 | 73.59711 | 50.17985 |
| Trap 3 --- | (132) | 47.31063 | 32.25725 | 68.96551 | 47.02194 |
| Fnl Clmbg- | 127) | 39.6 | 27 | 58.66667 | 40 |

12. 5\% Weight/Power
At MPH of
Median Weight/Power
At MPH of

Traps 1-2 Traps 2-3 Fnl Clmbe $309.4017306 .4224 \quad 278.1846$ $40.69827 \quad 35.02547 \quad 27$ $211.1524 \quad 203.3684 \quad 187.7746$ $51.88043 \quad 48.608940$

Trucks with trailers
12.5 Percentile

Median
No. Ft/sec MPH
Ft/sec MPH
$\begin{array}{lllllll}\text { Trap } 1 & ---(7) & 55.86916 & 37.54715 & 65.28346 & 44.51145\end{array}$
Trap 2 --- ( 8 ) $34.18804 \quad 23.31082 \quad 50.89058 \quad 34.69813$
 Fnl Cimbg-( 8) $27.8666719 \quad 44$

Traps 1-2 Traps 2-3 Fnl Clmbe
12.5\% Weight/Power At MPH of
Median Weight/Power At MPH of
$812.2221 \quad 354.8437 \quad 395.315$ $34.42859 \quad 22.5641 \quad 19$ $512.346 \quad 279.0059 \quad 250.3662$ $39.60479 \quad 32.9513130$

12.5\% Weight/Power

At MPH of
Median Weight/Power At MPH of

Traps 1-2 Traps 2-3 Fni Clmbg $369.4164 \quad 385.1472 \quad 343.3593$ $35.22714 \quad 28.15721 \quad 21.875$ $308.5507 \quad 275.1822 \quad 224.2085$ $46.42788 \quad 40.73305 \quad 33.5$

## UTICA

$904.00000 \quad 0.047330 .000008300 .04993$





| BLOSSEURG |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| $980.04000 \quad 900.80$ | 940.88098 | 0.06277 | 0.04695 | 0.85789 |
| Trucks | 12.5 | Percentile | Medi in |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1 --- (30) | 56.71801 | 38.67137 | 75.32957 | 51.36107 |
| Trap 2 --- (30) | 39.60396 | 27.8027 | 61.3497 | 41.82934 |
| Trap 3-- ( 30 ) | 38.21713 | 26.05713 | 60.79028 | 41.44792 |
| Fnl Clmbg--( 30 ) | 29.33334 | 20 | 52.8 | 36 |
|  |  | Traps 1-2 | Traps 2-3 | Fnl Clmbg |
| 12.5\% Weight/Power |  | 332.682 | 313.5254 | 323.8855 |
| At MPH of |  | 32.83704 | 26.52992 | 20 |
| Median Weight/Power |  | 278.8536 | 196.7797 | 179.9364 |
| At MPH of |  | 46.59521 | 41.63863 | 36 |
| Trucks with trailers 12.5 |  | Percentile | Median |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- ( 1) | 0 | 0 | 44.74273 | 30.50641 |
| Trap 2 --- (1) | 0 | 0 | 38.83495 | 26.47838 |
| Trap 3 --- ( 1 ) | 0 | 0 | 37.95067 | 25.87545 |
| Fnl Clmbg- 1 ) | 0 | 0 | 37.4 | 25.5 |
| Tractor trailersNo. | 12.5 | Percentile | Median |  |
|  | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- (215) | 53.23583 | 36.29715 | 67.28386 | 45.87536 |
| Trap 2 --- ( 225 ) | 32.58752 | 22.21876 | 52.05053 | 35.489 |
| Trap 3 --- ( 219) | 32 | 21.81818 | 46.64763 | 31.8052 |
| Fnl Clmbg--( 213 ) | 29.33334 | 20 | 39.6 | 27 |
|  |  | Traps 1-2 | Traps 2-3 | Fni Cimbg |
| 12.5\% Weight/Power |  | 398.1205 | 367.9119 | 323.8855 |
| At MPH of |  | 29.25796 | 22.01847 | 20 |
| Median Weight/Power |  | 293.5189 | 295.2681 | 239.9152 |
| At MPH of |  | 40.68218 | 33.6471 | 27 |



| BERNALILLO |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| 900.00008900 .00 | 900.08800 | 0.83258 | 0.03373 | 0.03838 |
| Trucks | 12.5 | Percentile | Median |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- ( 49) | 60.22127 | 41.85995 | 76.99776 | 52.49847 |
| Trap 2 --- (49) | 57.28863 | 39.06043 | 73.86945 | 50.36553 |
| Trap 3--- (49) | 55.74932 | 38.8109 | 72.73112 | 49.5894 |
| Fnl Clmbg--(49) | 50.6 | 34.5 | 71.13333 | 48.5 |
|  |  | Traps 1-2 | Traps 2-3 | Fnl Clmbe |
| 12.5\% Weight/Power |  | 351.4989 | 316.7078 | 283.1723 |
| At MPH of |  | 40.06019 | 38.53566 | 34.5 |
| Median Weight/PowerAt MPH of |  | 298.3996 | 243.2285 | 281.4319 |
|  |  | 51.432 | 49.97746 | 48.5 |
| Trucks with trailers 12.5 |  | Percentile | Median |  |
|  | $\begin{aligned} & \text { trailers } \quad 1 \\ & \text { No. } \end{aligned}$ | MPH | Ft/sec | MPH |
| Trap 1 --- ( 16 ) | 43.38395 | 29.57996 | 65.25285 | 44.49058 |
| Trap $2-\cdots-(16)$ | 35.97122 | 24.52584 | 68.33183 | 41.13534 |
| Trap 3-- ( 16 ) | 33.75528 | 23.61496 | 56.98006 | 38.85094 |
| Fnl Clmbg--( 16 ) | 29.33334 | 20 | 51.33333 | 35 |
|  |  | Traps 1-2 | Traps 2-3 | Fril Clmbg |
| 12.5\% Weight/Power |  | 618.8521 | 507.8765 | 488:4722 |
| At MPH of |  | 27.0529 | 23.7784 | 28 |
| Median Weight/Power |  | 399.6909 | 348.0836 | 279.127 |
| At MPH of |  | 42.81296 | 39.99269 | 35 |
| Tractor trajlers | 12.5 | Percentile | M Media |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- (92) | 61.20977 | 41.73393 | 75.32957 | 51.36107 |
| Trap 2-m (92) | 57.1021 | 38.93325 | 72.07207 | 49.14005 |
| Trap 3--- (92) | 53.48368 | 36.46614 | 69.44445 | 47.34849 |
| Fnl Cimbg--(92) | 46.93334 | 32 | 64.53333 | 44 |
|  |  | Traps 1-2 | Traps 2-3 | Fnl Clmbg |
| 12.5\% Weight/Power |  | 384.3152 | 370.8189 | 305.2952 |
| At MPH of |  | 40.33359 | 37.6997 | 32 |
| Median Weight/Power |  | 307.1942 | 284.5913 | 222.0328 |
| At MPH of |  | 50.25956 | 48.24426 | 44 |


| BEFNALILLO |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| 700.00000900 .0 | 00008 | 0.03258 | 0.03373 | 0.63838 |
| 65 foot Doubles | 12.5 | Percentile | Mediar |  |
|  | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- (17) | 63.88561 | 43.55837 | 84.03509 | 57.29666 |
| Trap 2--- (17) | 59.53638 | 40.59299 | 81.46775 | 55.54619 |
| Traf 3--- (17) | 56.38068 | 38.44137 | 81.05445 | 55.26439 |
| Fil Clmbg-- 17 ) | 49.68334 | 33.875 | 80.66666 | 55 |
|  |  | Trape 1-2 | Traps 2-3 | Fril Clmbe |
| 12.51/ Weight/Power |  | 382.2529 | 346.118 | 288.3969 |
| At MFH of |  | 42.07568 | 39.51718 | 33.875 |
| Median Weight/Power |  | 263.2644 | 207.8117 | 177.6263 |
| At MPH of |  | 56.42142 | 55.40529 | 55 |
| Rocky Mountain Doubles 12.5 |  | Percentile | Median |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- (0) | 0 | 0 | 0 | $\emptyset$ |
| Trap 2 --- ( ${ }^{\text {P }}$ ) | 0 | 0 | 6 | 0 |
| Trap $3--\infty$ ( 0 ) | 0 | 0 | 0 | 0 |
| Fnl Cimbg-- ( ${ }^{\text {a }}$ ) | 0 | 6 | 0 | 0 |
| Turnfike DoublesNo. | 12.5 | Percentile | Median |  |
|  | Ft/sec | MPH | Ft/sec | MPH |
|  | $\theta$ | 0 | 0 | 0 |
| Trap $2--\infty$ (0) | 0 | 0 | 0 | 0 |
| Trap 3--- (b) | 0 | 0 | 0 | 0 |
| Fril Cimbg-- 0 ) | 0 | 0 | 0 | 0 |
| Triples No. | 12.5 | Fercentile | Median |  |
|  | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1 --- ( 0 ) | 0 | 0 | 0 | $\square$ |
| Trap $2-\cdots$ ( ${ }^{\text {P }}$ ) | 0 | 0 | 0 | $\square$ |
| Trap $3-\cdots$ ( 0 ) | 0 | 0 | 0 | 0 |
| Fnl Clmbg-- ( ) | 0 | 0 | 0 | 0 |


| CARSON CITY 750.80000 | 750.08600 |  |  | 0.05582 | 0.05669 | 0.8575 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Trucks |  |  | 12.5 | Percentile | Median |  |
|  | No. |  | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- | 95 | ) | 57.37853 | 39.12172 | 71.42858 | 48.7013 |
| Trap 2 --- | 96 | ) | 52.01561 | 35.46518 | 64 | 43.63637 |
| Trap 3--- | 96 | ) | 47.7327 | 32.54582 | 59.88024 | 40.82744 |
| Fnl Clmbg-- | 94 |  | 41.06667 | 28 | 52.8 | 36 |

12. 5\% Weight/Power
At MPH of
Medi an Weight/Power
At MPH of

Traps 1-2 Traps 2-3 Fnl Clmbe $230.2533 \quad 230.4958 \quad 232.6773$ $37.29345 \quad 34.00511 \quad 28$ $232.1445 \quad 192.522 \quad 180.9712$ $46.16883 \quad 42.2319 \quad 36$

| Trucks with trailers | 12.5 |  | Percentile | Median |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| No. | Ft/sec | MPH | Ft/sec | MPH |  |
| Trap $1-m(4 i)$ | 43.04927 | 29.35178 | 53.79962 | 36.68156 |  |
| Trap $2---(41)$ | 31.51988 | 21.49082 | 42.37293 | 28.89063 |  |
| Trap $3--(40)$ | 25.46149 | 17.36011 | 35.74621 | 24.37241 |  |
| Fnl Clmbg--( 41$)$ | 23.46667 | 16 | 33 | 22.5 |  |

12.5\% Weight/Power
At MPH of
Median Weight/Power
At MPH of

Traps 1-2 Traps 2-3 Fni Clmbg
$388.0168 \quad 389.6658 \quad 407.1853$
$25.4213 \quad 19.42547 \quad 16$
$345.9165 \quad 306.3048 \quad 289.554$
$32.7861 \quad 26.63152 \quad 22.5$

| Tractor trailers | 12.5 Percentile |  |  | Median |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap $1-2-(57)$ | 56.9518 | 38.83077 | 69.80803 | 47.59639 |  |
| Trap $2--(58)$ | 43.62618 | 29.74512 | 64.10256 | 43.70629 |  |
| Trap $3--(58)$ | 35.85838 | 24.4489 | 56.81819 | 38.73967 |  |
| Fnl $(5$ mbg-- $(57)$ | 29.51667 | 20.125 | 49.13333 | 33.5 |  |


| $12.5 \%$ Weight/Power | 389.6442 | 315.2028 | 323.7249 |
| :---: | ---: | :--- | :--- |
| At MPH of | 34.28795 | 27.89701 | 20.125 |
| Median Weight/Power | 205.3654 | 236.5777 | 194.4766 |
| At MPH of | 45.65133 | 41.22298 | 33.5 |.


| CAFSON CITY |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| 750.00060750 .0 | 750.00000 | 0.05582 | 0.05669 | 0.05756 |
| 65 foot Doubles | 12.5 | Percentile | Median |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- (11) | 48.61112 | 33.14395 | 71.62093 | 48.83245 |
| Trap $2--(11)$ | 33.6547 | 22.94639 | 62.46221 | 42.58787 |
| Trap 3--- (11) | 29.49738 | 20.11185 | 52.63194 | 35.88542 |
| Fnl Elmbe-- 10 ) | 25.66667 | 17.5 | 41.06667 | 28 |
|  |  | Traps 1-2 | Trapis 2-3 | Fnl Cilmbg |
| 12.5\% Weight/Fower |  | 440.6679 | 339.8493 | 372.2837 |
| At MPH of |  | 28.04516 | 21.52912 | 17.5 |
| Median Weight/Power |  | 269.9318 | 287.3189 | 232.6773 |
| At MPH of |  | 45.71016 | 39.23665 | 28 |
| Rocky Mountain Doubles 12.5 |  | Fercentile | - Median |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- (1) | 6.830601 | 4.657228 | 27.32241 | 18.62891 |
| Trap $2--\infty$ ( 1 ) | 4.99082 | 3.402286 | 19.96008 | 13.60914 |
| Trap 3--- (1) | 3.607504 | 2.459662 | 14.43041 | 9.838646 |
| Fnl Clmbg-- 1 ) | 3.483333 | 2.375 | 13.93333 | 9.5 |
| 12.5\% Weight/Pouler |  | Trape $1-2$ 1680.745 | Traps 2-3 2266.798 | Fni Cimbg 2743.143 |
| 12. At MPH af |  | 4.029758 | 2.950974 | 2.375 |
| Median Weight/Fower |  | 478.5918 | 606.3555 | 685.7856 |
| At MPH of |  | 16.11993 | 11.7239 | 9.5 |
| Turnpike Doubles | 12.5 | Percentile | Medi |  |
| No. | $\mathrm{Ft} / \mathrm{sec}$ | MPH | Ft/sec | MPH |
| Trap 1--- (0) | 0 | 0 | 0 | 8 |
| Trap $2--\infty$ ( 0 ) | 0 | 0 | 0 | 0 |
| Traf 3 --- ( ${ }^{\text {a }}$ ) | 0 | 0 | 0 | 0 |
| Fnl Clmbg-- ( 0 ) | 0 | 0 | 0 | 0 |
| Trifiles | 12.5 | Fercentile | Medi |  |
| No. | Ft/sec | MPH | Ft/sec | MFH |
| Trap 1 --- (0) | 0 | 0 | 0 | 0 |
| Trap 2 --- (0) | 0 | 0 | 0 | a |
| Trap 3 --- ( ) | 0 | 0 | 0 | a |
| Fnl Clmbg-- (0) | 0 | 0 | 0 | 0 |


| SAN LUIS |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| 1088.800081080 .00000 |  | 0.04942 | 0.84885 | 0.05901 |
| Trucks N | $\begin{array}{r} 12.5 \\ F t / \sec \end{array}$ | Percentile | - Median |  |
|  |  | MPH | Ft/sec | MPH |
| Trap 1--- ( 15 ) | 53.10183 | 36.20579 | 72.67134 | 49.54864 |
| Trap 2 --- ( 14 ) | 42.19504 | 28.76934 | 65.25285 | 44.49058 |
| Trap 3--- (15) | 37.02528 | 25.24451 | 60.6646 | 41.36223 |
| Fnl Clmbg-- 15 ) | 35.93334 | 24.5 | 55 | 37.5 |
|  |  | Traps 1-2 | Traps 2-3 | Fnl Clmbg |
| 12.5\% Weight/Power |  | 346.8214 | 326.8158 | 259.4001 |
| At MPH of |  | 32.48757 | 27.08693 | 24.5 |
| Median Weight/Power |  | 237.8347 | 219.0815 | 169.4747 |
| At MPH af |  | 47.01961 | 42.9264 | 37.5 |
| Trucks with trailers |  | Percentile | Median |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1 --- ( 23 ) | 57.48021 | 39.13651 | 67.11882 | 45.76283 |
| Trap 2 --- ( 22 ) | 44.89406 | 30.60958 | 55.71031 | 37.9843 |
| Trap 3--- ( 23 ) | 36.51349 | 24.89556 | 49.24869 | 33.57865 |
| Fnl Clmbg--( 23 ) | 31.9 | 21.75 | 43.26667 | 29.5 |
| 12.5\% Weight/Power |  | Traps 1-2 | Trape 2-3 Fnl Clmbg |  |
|  |  | 363.8232 | 353.2399 | 292.1978 |
| At MPH of |  | 34.87304 | 27.75257 | 21.75 |
| Median Weight/Power |  | 323.7471 | 273.5291 | 215.434 |
| At MPH of |  | 41.87357 | 35.78148 | 29.5 |
| Tractor trailers 12.5 |  | $\begin{gathered} \text { Percentile } \\ \text { MPH } \end{gathered}$ | Median |  |
| Trap 1--- (122) | 59.7238 | 40.72077 | 72.33273 | $49.31777$ |
| Trap 2 --- ( 122 ) | 48.82582 | 33.29033 | 64 | 43.63637 |
| Trap 3--- ( 122) | 42.623 | 29.06114 | 56.17978 | 38.3044 |
| Fnl Clmbg--( 117) | 33.73333 | 23 | 46.93334 | 32 |
| 12.5\% Weight/Power |  | Traps 1-2 | Traps 2-3 Fnl Cimbe |  |
|  |  | 326.3406 | 300.4286 | 276.3175 |
| At MPH of |  | 37.00556 | 31.17573 | 23 |
| Median Weight/Power |  | 253.8759 | 267.22 | 198.6032 |
| At MPH of |  | 46.47707 | 40.97038 | 32 |



| PAYSON |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| 908.00800900 .8 | 900.00000 | 0.05813 | 0.86104 | 0.05901 |
| Trucks | 12.5 | Percentile | Median |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- (60) | 72.57295 | 49.48156 | 80.64516 | 54.98534 |
| Trap 2 --- (60) | 64.06607 | 43.68141 | 74.07408 | 50.50505 |
| Trap 3--- (61) | 55.68385 | 37.91172 | 70.54674 | 48.10005 |
| Fnl Clmbg--( 60 ) | 47.66667 | 32.5 | 61.6 | 42 |
|  |  | Traps 1-2 | Traps 2-3 | Fnl Clmbg |
| 12.5\% Weight/Power |  | 211.4077 | 210.9595 | 195.5478 |
| At MPH of |  | 46.58149 | 40.79656 | 32.5 |
| Median Weight/Power |  | 175.1414 | 145.5916 | 151.3167 |
| At MPH of |  | 52.7452 | 49.30255 | 42 |
| Trucks with trailers 12 |  | Percentile | Median |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1-m (19) | 65.21496 | 44.46475 | 72.59744 | 49.49825 |
| Trap 2 --- ( 19 ) | 44.28311 | 30.19303 | 54.54647 | 37.19078 |
| Trap 3--- ( 19 ) | 27.48168 | 18.73751 | 40.40817 | 27.55182 |
| Fnl Clmbg--( 19) | 27.86667 | 19 | 34.46667 | 23.5 |
|  |  | Traps 1-2 | Traps 2-3 | Fnl Cimbg |
| 12.5\% Weight/Power |  | 540.3811 | 380.9024 | 334.4896 |
| At MPH of |  | 37.32889 | 24.46527 | 19 |
| Median Weignt/Power |  | 466.7146 | 305.8079 | 270.4384 |
| At MPH of |  | 43.34451 | 32.3709 | 23.5 |
| Tractor trailers | 12.5 | Fercentile | Media |  |
| No. | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1 --- (13) | 57.14618 | 38.9633 | 76.92564 | 52.4493 |
| Trap $2---(113)$ | 43.68398 | 29.78453 | 63.19179 | 43.08531 |
| Trap 3--- (113) | 32.67315 | 22.27715 | 48.96027 | 33.382 |
| Fil Clmbg-e( 113) | 32.26667 | 22 | 41.86667 | 28 |
|  |  | Traps 1-2 | Traps 2-3 | Fnl Clmbg |
| 12.5\% Weight/Power |  | 314.2551 | 309.5515 | 288.8774 |
| At MPH of |  | 34.37392 | 26.03084 | 22 |
| Median Weight/Power |  | 314.8614 | 292.7254 | 226.9751 |
| At MPH of |  | 47.76731 | 38.23366 | 28 |


| FAYSON |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| 900.00000 908.0 | 98908 | 0.05813 | 0.06184 | 0.015901 |
| 65 foot Doubles | 12.5 | Percentile | Median |  |
|  | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- (7) | 41.07981 | 28.80896 | 82.09697 | 55.97521 |
| Trap 2 --- (7) | 37.31343 | 25.44098 | 75.76043 | 51.65457 |
| Trap 3--- ( 7 ) | 24.3563 | 16.60657 | 71.61502 | 48.82842 |
| Fnl Clmbg-- 6) | 23.1 | 15.75 | 57.2 | 39 |
|  |  | Traps 1-2 | Traps 2-3 | Fril Cimbe |
| 12.5\% Weight/Power |  | 264.5489 | 377.441 | 403.5113 |
| At MPH of |  | 26.72497 | 21.02377 | 15.75 |
| Median Weight/Fower |  | 170.4759 | 147.7877 | 162.9565 |
| At MPH of |  | 53.81489 | 50.2415 | 39 |
| Focky Mountain Doubles 12.5 No. Ft/sec |  | Percentile | Median |  |
|  |  | MPH | Ft/sec | MPH |
| Traf 1--- ( ) | 0 | 0 | 0 | 0 |
| Trap 2 - - ( ) | 0 | 0 | 0 | 0 |
| Trap 3 --- (0) | 0 | 0 | 0 | 0 |
| Fril Clmbg- ( 0 ) | 0 | 0 | 0 | 0 |
| Turnfike DoublesNo. | 12.5 | Percentile | Median |  |
|  | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- (0) | 0 | 0 | 0 | 0 |
| Trap 2 --- (0) | 0 | 0 | 0 | 0 |
| Trap 3--- (0) | 0 | 0 | 0 | 0 |
| Fnl Cimbg- ( 0 ) | 0 | 0 | 0 | 0 |
| Triples No. | 12.5 | Fercentile | Median |  |
|  | Ft/sec | MPH | Ft/sec | MPH |
| Trap 1--- (0) | 0 | 0 | 1 | 0 |
| Trap 2 --- (0) | 0 | 0 | 0 | 0 |
| Trap 3-- ( ${ }^{\text {a }}$ ( | 0 | 0 | 0 | 0 |
| Fnl Clmbg- ( 0 ) | 0 | 0 | 0 | 0 |


[^0]:    Comparison of "Effective" and "Rated" Engine Power
    The performance characterization by the "effective" power ( $P_{3} / W$ ) available for acceleration or overcoming grade has provided a direct measure by which to predict decelerations of the truck population on grades. However, it can only be evaluated by field measurements. Past prediction methods have been based on estimates of actual vehicle parameters. Those necessary are engine power ( $P_{1}$ ), weights, rolling

