

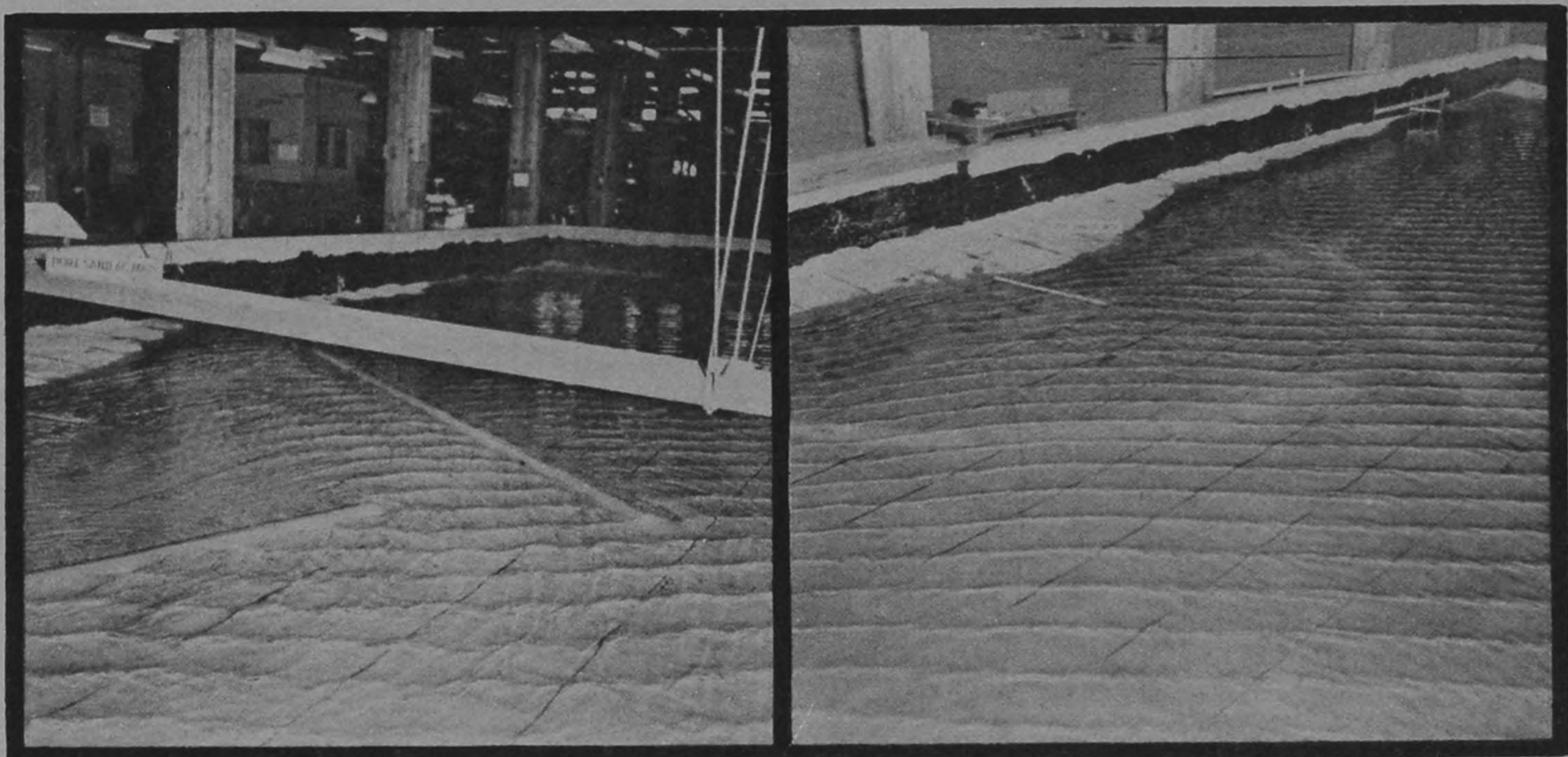
ENGINEERING RESEARCH INSTITUTE  
UNIVERSITY OF MICHIGAN  
ANN ARBOR

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MODEL STUDY FOR  
HARBOR OF REFUGE FOR  
LIGHT DRAFT VESSELS  
AT PORT SANILAC , MICHIGAN

Technical Report No. 1  
Lake Hydraulics Laboratory  
Department of Civil Engineering

JULY 1, 1949



BY: E.F. BRATER

APPROVED BY: C.O. WISLER

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Project M 804 - Contract W-20-064-eng-1774  
Detroit District, Corps of Engineers,  
United States Army



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FOR  
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Project M804

CORPS OF ENGINEERS  
DETROIT, MICHIGAN

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MODEL STUDIES FOR HARBOR OF REFUGE  
FOR  
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INTRODUCTION

Port Sanilac, Michigan is located approximately 30 miles from the southern end of Lake Huron as shown by the map in Drawing 1. It is proposed to construct breakwaters and to dredge areas within the breakwaters in order to create a harbor of refuge for light draft vessels at this location. The primary purpose of the model study was to determine the breakwater arrangement which would afford the maximum protection from wave action in the harbor area. As an additional basis for comparison, currents were measured within the harbor and near the harbor entrance for each of the plans. The effect of the size of the harbor opening was investigated by constructing one of the plans with a larger opening than those of the others. The effect of the shape of the breakwaters was studied by reconstructing one of the plans with vertical-walled breakwaters. Rubble mound breakwaters were simulated in all other plans.

The study was made as a result of a contract dated November 8, 1948, between the University of Michigan Engineering Research Institute and the Detroit District, Corps of Engineers, U. S. Army. The initiation of the work was expedited by the interest and financial support of the Michigan State

### Waterways Commission.

Throughout the model study frequent consultations were held with the following personnel of the Detroit District: Colonel Louis J. Rumaggi, District Engineer, Tom C. Trelfa, Harley F. Lawhead and Charles E. Lee. Colonel E. W. Nelson and W. H. Booth, Jr. of the Great Lakes Division Office, Corps of Engineers, visited the laboratory in connection with the work. Mr. R. Y. Hudson of the Waterways Experiment Station, Vicksburg, Mississippi, visited the laboratory at various stages of the work, as did Colonel H. D. Vogel, District Engineer of the Buffalo District.

Members of the Michigan State Waterways Commission visited the laboratory at various times. E. W. Kiefer, Chairman, and Leonard H. Thomson, Secretary, were in close touch with the work throughout the tests.

The University of Michigan Lake Hydraulics Laboratory where the tests were made is a facility of the Engineering Research Institute and the Department of Civil Engineering of the College of Engineering. Professor A. E. White is Director and Professor C. W. Good is Assistant Director of the Engineering Research Institute. Professor Earnest Boyce is Chairman of the Department of Civil Engineering and Ivan C. Crawford is Dean of the College of Engineering. The laboratory is under the general direction of C. O. Wisler, Professor of Hydraulic Engineering, and the model tests were conducted under the supervision of E. F. Brater, Associate Professor of Civil Engineering. The following men took an active part in conducting the tests: John H. Boeckerman, Jerome Pepper, Leslie D. Stair and Dah C. Woo.



### THE MODEL

The model was built to an undistorted linear scale of 1 to 75. Templates were cut from 3/8-inch waterproof plywood according to sounding data supplied by the Corps of Engineers. The templates were spaced at 1.5-foot intervals in and near the harbor area and at 3-foot intervals in the more remote regions. A plan of the model and wave tank is shown in Drawing 2. The space between the templates was packed with sand to within an inch of their' upper edges. The model was then surfaced with cement mortar which was screeded to conform with the edges of the templates. The accuracy of the model was checked by means of an engineer's level and again by checking contours of the lake bottom against locations of the shore line for various water surface elevations. Plate 1 shows the templates in place before the concrete was poured.

### METHOD OF CONDUCTING TESTS

The waves were generated by means of a plunger-type wave machine 30 feet long. The wave machine is shown in Plate 1. The amplitude and period of the plunger may be varied to produce waves of any required height, length and period. The wave machine is portable, so that any desired wind direction can be simulated.

Wave heights were measured with electric resistance gages arranged so that variations in water level were recorded by means of oscillographs. The calibrations of the resistance gages were checked systematically during the tests. The wave height at any point was determined by computing the

average of the highest one-third of 180 successive waves. The instruments described above are shown in Plate 2.

Surface currents were measured by timing the movements of small floats with reference to coordinate lines on the model.

The elevation of the water surface in the tank was checked by means of a hook gage mounted on one of the tank walls. It was necessary to add small amounts of water occasionally to compensate for evaporation and leakage.

#### THE TESTING PROGRAM

Five breakwater arrangements designated as Plans 1, 2, 3, 4 and 5 were tested. Each of these was constructed to simulate the shape of a rubble mound breakwater as shown in Drawing 1. In each of these the outer 200 feet of the north breakwater had the shape designated in Drawing 1 as Type A, whereas the remainder was of Type B. Plan 6 consisted of vertical-walled breakwaters placed in the same position as those of Plan 3. Typical sections through these breakwaters are also shown in Drawing 1.

For all plans, except Plan 4, the width of the dredged entrance channel was 150 feet and the distance from toe to toe of rubble mound breakwaters was 185 feet. Plan 4 was constructed with a dredged entrance channel 200 feet wide and a distance from toe to toe of rubble mound breakwaters of 235 feet. The clear distance between the vertical faces of breakwaters of Plan 6 was 210 feet at the harbor entrance.

The harbor arrangement designated as Plan 1 may be seen in Drawings 3-9 and 11-13 and Plate 2. Plan 1a, shown in Drawing 10, consisted of the same breakwater arrangement as Plan 1 with the elevation of the north breakwater raised to prevent overtopping by waves from the NNE.

Plan 2 differed from Plan 1 only in having the outer 100 feet of the south breakwater rotated toward the lake as shown in Drawings 14-22 and Plates 3 and 4.

For Plan 3, the south breakwater was in the same position as for Plan 1. The entire north breakwater was rotated in a clockwise direction to the position shown in Drawings 23-31 and Plates 4, 5 and 6.

Plan 4 was similar to Plan 3 except that the south breakwater was moved southward a distance sufficient to increase the size of the opening by 50 feet. Plan 4 is illustrated in Drawings 32-40 and Plates 7 and 8.

Plan 5 consisted of curved breakwaters placed in the same general location as for Plan 3. Plan 5 is shown in Drawings 41-49 and Plates 9 and 10.

The breakwater arrangement for Plan 6 was exactly the same as for Plan 3, the only difference being in the nature of the cross section of the breakwaters as previously described. The drawings for Plan 6 are numbered 50-56 and 58-60, and photographs are shown in Plates 10, 11 and 12.

Plan 6a consisted of the breakwaters of Plan 6 with spurs added to the inner face of the north breakwater as shown in Drawing 57.

#### TEST CONDITIONS

The basic wave data were prepared by H. F. Lawhead and C. E. Lee of the Detroit District Office. Deep water wave heights were computed from the records of wind velocity and duration for the following four wind directions: N, NNE, E and SSE. Refraction diagrams were then prepared for each wind direction. It was found from these diagrams that the wave action at Port Sanilac resulting from a north wind was similar to but less severe than that produced by a wind from the NNE. Consequently no tests were run for the north direction. The refraction diagrams for the three wind directions used in the tests

are reproduced in Appendix C at the end of this report.

For Plan 6 one additional wind direction was tested. This direction was approximately SE. It was determined in the wave tank by trial so that the resulting wave would be projected directly into the harbor entrance; i.e., the waves moved in a direction parallel to the north breakwater as they approached the harbor. This same wind direction was used in the test on Plan 6a.

For each wind direction a "large" wave and a "small" wave was projected against the harbor. The large wave was one that would be produced by a severe storm. The small wave would be produced by less severe storms of more frequent occurrence. A summary of the characteristics of the waves used in the tests is given in Table I. The frequencies shown in the table were determined from curves given in Appendix C.

TABLE I  
SUMMARY OF WAVE DATA

Wave Characteristic	Small Wave			Large Wave		
	SSE	E	NNE	SSE	E	NNE
Deep Water Wave Height (Ft.)	4.5	4.5	4.5	8.7	7.0	13.0
Deep Water Wave Length (Ft.)	81.9	81.9	81.9	90.3	81.9	190.5
Wave Period (Sec.)	4.0	4.0	4.0	4.2	4.0	6.1
Frequency *	75	45	272	1	5	4

\* Number of times wave height will be equaled or exceeded in ten years.

It is believed that the larger waves would give an indication of the disturbance inside the harbor when severe Lake Huron storms have reached their full intensity and are producing near maximum waves at the harbor site. The smaller waves would occur more frequently and might be thought to represent the conditions that would commonly exist when small boats are entering

the harbor to seek refuge from a major storm before it has reached its full intensity. Finally, the smaller waves permit a comparison of the various harbor arrangements under conditions of virtually no overtopping of the breakwaters by storm waves, whereas in the case of the larger waves the conditions inside the harbor are affected by overtopping to some extent for the SSE and E winds and to a greater degree for the NNE wind.

Low-water datum for Lake Huron is at elevation 578.5. The crests of the breakwaters were set 8 feet above low-water datum. Throughout the tests the lake elevation was kept 3 feet above low-water. Thus, the crests of the breakwaters were 5 feet above the still water level of the lake. Such a high water condition is primarily the result of a prolonged westerly wind which tends to cause some Lake Michigan water to enter Lake Huron through the Straits of Mackinac. A change in wind direction will then produce the conditions simulated in the tests. The lake stage used in the tests was determined from a consideration of the records of the U. S. Lake Survey's water level recorder at Harbor Beach, Michigan, covering a number of storm periods.

#### PRESENTATION OF RESULTS

All of the data obtained from the tests are presented in Drawings 3-60. The successive plans are presented in numerical order. For each plan the drawings showing measured values of harbor wave heights are presented first. These are followed by drawings giving the results of current measurements. Photographs showing harbor conditions during large wave tests are presented in Plates 1-12.

### Wave Heights

Wave heights were measured at from 25 to 30 locations for each test. The measured values are recorded on the drawings at the gage locations. With these values as a basis, lines of equal wave height were drawn. The regions where the wave heights were less than 1.5 feet and where they were greater than 5.0 feet were hatched. The direction in which the predominant wave was moving is indicated at each gage location by means of an arrow. The wave height drawings for Plans 1, 3, 4 and 6 were also reproduced on single sheets for each wind direction. These are shown on Pages 81 through 91 following Drawing 61.

Some numerical averages were found to be useful in analyzing the test data. In Table II are shown three groups of averages for each wind direction and for both the large and small waves. The first group consists of the measurements made near the harbor entrance. The second group comprises those inside the harbor. The dividing line between the two groups is shown as a dotted line in Drawing 61. A third group designated in Table II as the "mooring area" consisted of eleven measuring stations located in the quietest portion of the harbor. This region is outlined by the dashed line in Drawing 61.

### Currents

The magnitude and direction of velocities in and around the harbor are shown on the drawings by means of arrows. The lengths of the arrows were made proportional to the velocities according to a scale shown on the drawings. Paths followed by the floats are shown by means of dotted lines. In some locations the velocities varied with time, so that occasionally different paths may be seen to emanate from the same point. A summary of the maximum

TABLE II  
AVERAGE WAVE HEIGHTS

Plan	Entrance			Harbor			Mooring Area			
	SSE	E	NNE	SSE	E	NNE	SSE	E	NNE	
Small Wave	1	4.6	2.0	0.9	0.9	0.5	0.1	0.6	0.6	0.1
	2	4.5	1.2	1.4	1.8	0.4	0.4	1.4	0.5	0.5
	3	4.0	2.1	1.6	1.2	0.2	0.3	0.6	0.3	0.2
	4	3.9	2.6	0.9	1.4	0.6	0.2	0.6	0.7	0.2
	5	4.8	2.2	1.3	1.2	0.6	0.2	0.6	0.8	0.2
	6	4.9	4.3	0.9	1.2	1.1	0.1	0.6	0.7	0.1
Large Wave	1	5.6	5.2	5.1	1.4	1.6	1.7	1.0	1.3	1.8
	2	7.0	3.8	4.2	2.1	1.4	1.6	1.6	1.1	1.4
	3	5.5	4.6	3.6	1.4	0.6	1.2	0.9	0.6	1.1
	4	6.1	4.2	2.8	1.6	0.8	1.6	0.8	0.8	1.5
	5	6.0	4.1	3.9	1.5	0.9	1.6	0.8	0.7	1.6
	6	6.7	5.2	2.2	2.0	1.6	0.6	1.2	1.0	0.6

velocities found in various locations is given in Table III.

TABLE III

## MAXIMUM CURRENTS FOUND IN VARIOUS HARBOR LOCATIONS

Values are in mi. per hr.

Plan	Entrance			Harbor			Mooring Area			Between Shore and Inner End of Breakwater		
	SSE	E	NNE	SSE	E	NNE	SSE	E	NNE	SSE	E	NNE
1	3.1	2.8	2.5	3.3	1.8	2.4	0.9	0.9	2.4	3.3	---	2.0
2	1.2	2.0	3.2	1.0	3.5	2.2	1.1	3.5	2.2	1.0	1.0	2.5
3	0.8	2.3	3.0	0.6	1.0	2.6	0.4	0.7	2.6	0.8	---	---
4	1.7	2.6	2.4	3.1	1.8	1.5	0.8	1.8	1.5	1.7	---	0.5
5	2.4	1.1	3.2	3.1	1.6	1.6	1.3	1.4	1.6	1.9	1.2	1.1
6	2.1	1.5	3.6	2.5	0.9	1.6	1.1	0.9	1.5	2.0	1.6	2.0

CONCLUSIONS

The model tests provided data which permit the evaluation of the effectiveness of the various plans in regard to wave heights inside the harbor, wave heights in the vicinity of the entrance and currents. The final selection of a plan will require the consideration of other factors, such as the costs of the various plans, the availability of construction materials, desirable entrance size and suitability for mooring and docking purposes.

On the basis of an inspection and comparison of the wave height drawings, Table IV was prepared, showing the best plans for various conditions. Where two or more plans have been tabulated, it was for the reason that several seemed to be so nearly equal that it would have been misleading to select only one. It will be noted that Plan 3 appears 10 times in Table IV, whereas the



the others appear only 4 or 5 times.

TABLE IV

## PLANS GIVING THE MOST FAVORABLE RESULTS FOR VARIOUS CONDITIONS

Wind Direction	Small Wave		Large Wave	
	Entrance	Harbor	Entrance	Harbor
SSE	3,4	1,3,4,5,6	3	3
E	1,2,3	2,3,5	3,4	3
NNE	1,2,3,4,5,6	1,2,3,4,5,6	5,6	6

In Table V is given a set of values obtained by numbering the average wave heights of Table II from 1 to 6 in order of ascending size. Thus, the best plan for any region and wind direction is given the value 1, the next best 2, and so on. The sum of the values for each wave size is given for all of the plans. Plan 3 is again indicated as being better than the others. A more detailed method of summarizing the values in Table V is shown in Table VI. Here the values for a particular plan are added for each wind direction and wave size. This tabulation indicates that Plan 3 is as good or better than the others for waves from the SSE and E, but for the NNE direction Plan 6 is very good for both the large and small waves. For the large wave this is due in part to the fact that harbor waves resulting from the NNE wind were affected to a considerable degree by the overtopping of the north breakwater. The rectangular shape of the breakwaters of Plan 6 broke up the overtopping waves much more than the sloping shape of the other breakwaters. In this connection it should be noted that harbor waves resulting from overtopping cannot be determined as confidently from model tests as those entering through openings in the breakwaters. It can be assumed that harbor waves will be worse

in the model than for comparable conditions in the prototype because the surface of the model breakwater is relatively smoother than the prototype surfaces. It is also probable that the wind action may tend to break up some of the overtopping waves in the prototype.

TABLE V  
NUMERICAL EVALUATION OF THE VARIOUS PLANS

Plan	Entrance			Harbor			Mooring Area			Summation	
	SSE	E	NNE	SSE	E	NNE	SSE	E	NNE		
Small Wave	1	4	2	2	1	3	1.5	3	3	1.5	21
	2	3	1	5	6	2	6	6	2	6	37
	3	2	4	6	3	1	5	3	1	4	29
	4	1	3	2	5	4.5	3.5	3	4.5	4	30.5
	5	5	5	4	3	4.5	3.5	3	6	4	38
	6	6	6	2	3	6	1.5	3	4.5	1.5	33.5
Large Wave	1	2	5.5	6	1.5	5.5	6	4	6	6	42.5
	2	6	1	5	6	4	4	6	5	3	40
	3	1	4	3	1.5	1	2	3	1	2	18.5
	4	4	3	2	4	2	4	1.5	3	4	27.5
	5	3	2	4	3	3	4	1.5	2	5	27.5
	6	5	5.5	1	5	5.5	1	5	4	1	33

The results shown in Tables V and VI fail to take into account the magnitude of the differences between various plans. This is also true to some extent of the evaluations shown in Table IV. Consequently, these summaries can only be considered as a guide and constant reference must be made to the

original wave height drawings and the wave height averages of Table II in evaluating the various plans.

TABLE VI

## SUMMARY OF THE NUMERICAL EVALUATION OF THE VARIOUS PLANS

Plan	Small Wave			Large Wave		
	SSE	E	NNE	SSE	E	NNE
1	8	8	5	7.5	17	18
2	15	5	17	18	10	12
3	8	6	15	5.5	6	7
4	9	12	9.5	9.5	8	10
5	11	15.5	11.5	7.5	7	13
6	12	16.5	5	15	15	3

The following evaluations of the various plans are based entirely on wave heights. However, the results of the current measurements shown in the drawings and summarized in Table III are generally in accord with the conclusions reached.

Plan 1

This breakwater arrangement gave good results for the small waves. However, the large wave tests indicated that Plan 3 has definite advantages over Plan 1. Therefore, barring other considerations, Plan 3 should be chosen in preference to Plan 1.

Plan 1a

Overtopping of the north breakwater was prevented in the model by placing bricks on the breakwater. Tests were run only for the large NNE wave.

The results of the tests, given in Drawing 10, may be compared with those obtained under the same conditions with the normal breakwater elevation, as shown in Drawing 7, to obtain an indication of the effect of overtopping on the harbor conditions. With overtopping prevented, the waves in the harbor were less than one-third of the height of those during overtopping. However, the results are not entirely comparable with those that would occur if the sloping faces of the breakwater were carried high enough to prevent overtopping. This is because the sloping face would absorb some of the energy which was reflected by the vertical face of the bricks.

#### Plan 2

This plan gave the least satisfactory results of any of the plans tested. It can be eliminated from further discussion.

#### Plan 3

This plan gave very good results. Its relative advantages and disadvantages will be discussed in more detail as a basis for comparison with Plans 4 and 6.

#### Plan 4

In general, the average wave heights inside the harbor were found to be from 0.2 feet to 0.4 feet higher for Plan 4 than for Plan 3. The choice between Plans 3 and 4 depends upon whether the advantages of the larger entrance of Plan 4 outweigh the more comfortable mooring conditions provided by Plan 3.

#### Plan 5

The tests showed that the curved breakwaters of this plan provided no advantages over several of the other plans.

Plan 6

Plan 6 is better than Plan 3 during NNE wind storms, but Plan 3 provides much better protection during storms from the SSE and E. The frequencies given in Table I indicate that for every 4 occurrences of the large wave from the NNE, the combined number of occurrences from the SSE and E will be 6. However, in the case of the small waves, there will be more than twice as many occurrences from the NNE as the combined number from the SSE and E. Consequently, from a statistical point of view the large wave tests would lead to the conclusion that Plan 3 is the better one, whereas the reverse would be indicated by the small wave tests. However, the statistical importance of the small wave from the NNE is nearly nullified by the fact that actual wave heights in the harbor and at the entrance are only a small amount greater for Plan 3 than for Plan 6. This may be seen by comparing Drawings 28 and 55. In contrast, the small waves from the east result in much greater differences between the two plans, as shown by Drawings 26 and 53. Moreover, the vertical-walled breakwaters of Plan 6 produce regions of large wave height near the entrance and in the northeastern portion of the harbor during both the large and the small waves from the SSE. Therefore, unless other considerations favor the use of the rectangular-shaped breakwater, the tests indicate that Plan 3 should be selected.

Plan 6: Intermediate Wind Direction

The conditions for this test would be produced by a wind from approximately the SE direction. The deep water wave height used in the test was 8.0 feet, as compared with values of 8.7 feet for the SSE and 7.0 feet for the E. The results of the test shown in Drawing 56 may be compared with those of the SSE (Drawing 50) and E (Drawing 52). It will be seen that the harbor and harbor

entrance wave heights were somewhat higher than those from the E and lower than those from the SSE. The test provided assurance that an intermediate wind direction would not cause unusually bad harbor conditions.

Plan 6a

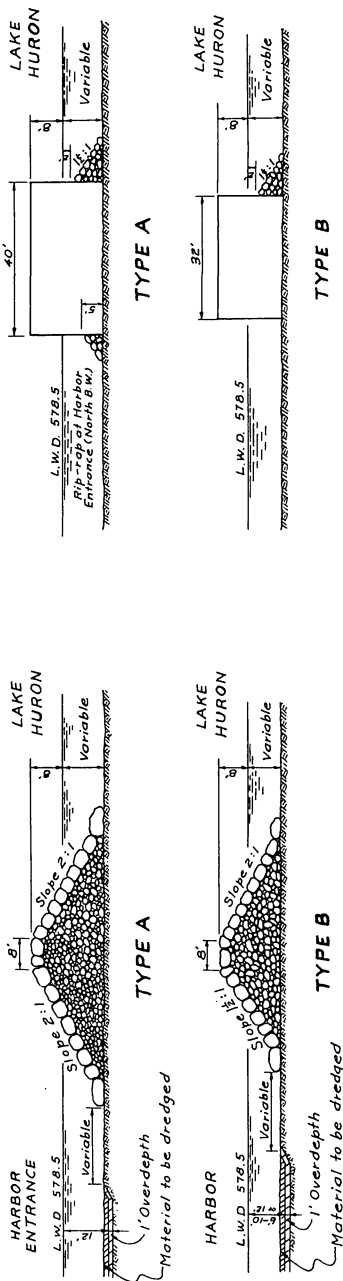
A limited number of measurements were taken with spurs on the inner face of the north breakwater. The wind direction was SE and the deep water wave height was 8.0 as in the test described in the previous paragraph. The results are shown in Drawing 57. The spurs were found to have little effect at the entrance and in the northerly portion of the harbor. However, they caused reflections which more than doubled the wave height in the mooring area.

**APPENDIX A**

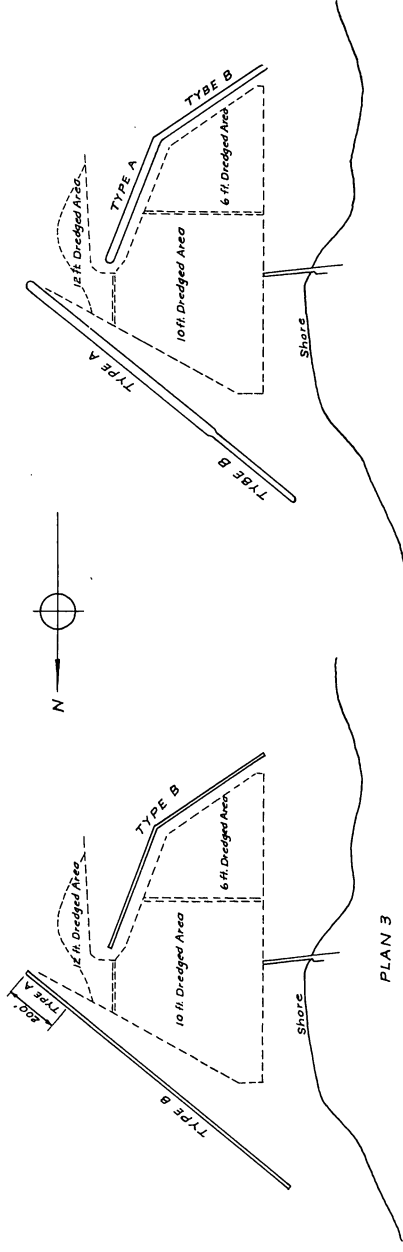
**DRAWINGS**



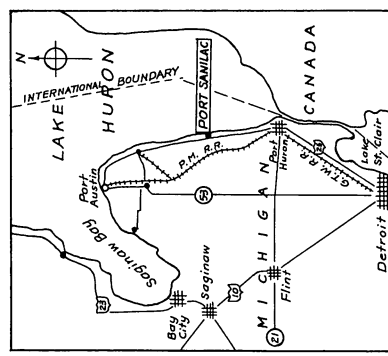




Scale of Feet: 0 10 20 30 40 50



Scale of Feet: 0 100 200 300 400 500



LOCATION MAP

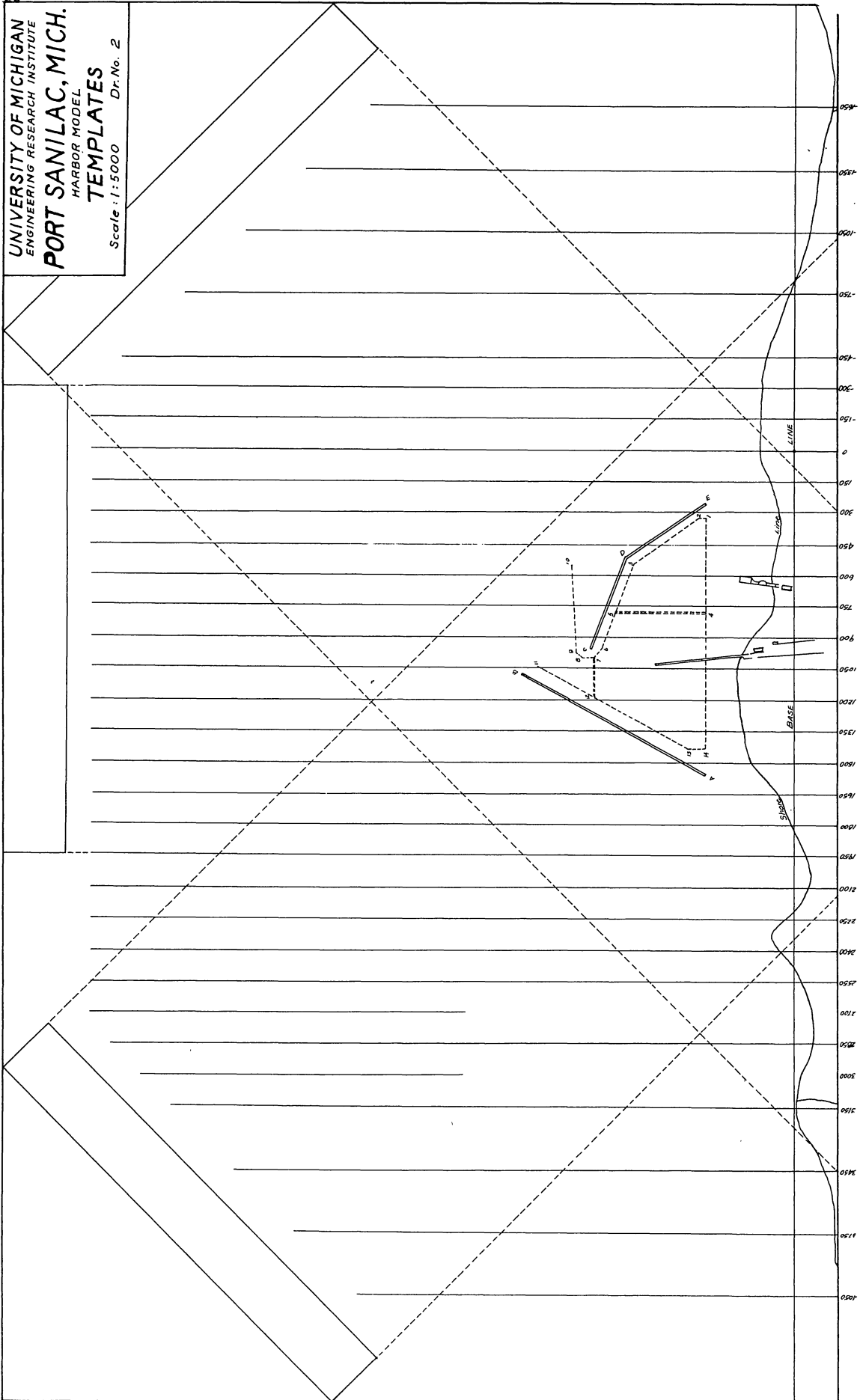
Scale of Miles  
0 10 20 30 40

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HARBOR MODEL  
Dr. No. 1

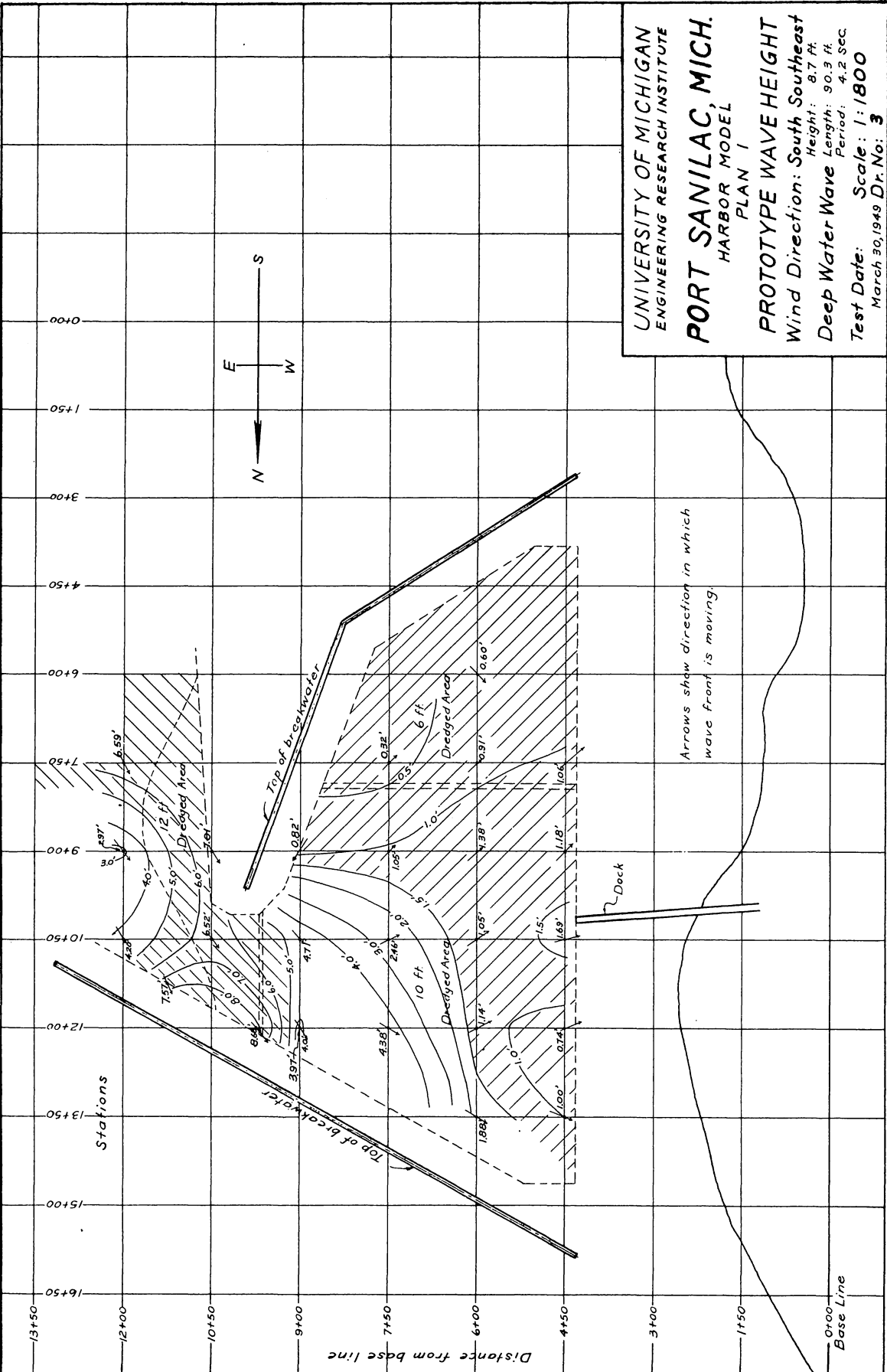
TYPICAL RUBBLE MOUND BREAKWATER      VERTICAL WALLED BREAKWATER

PLANS 1-5      PLAN 6

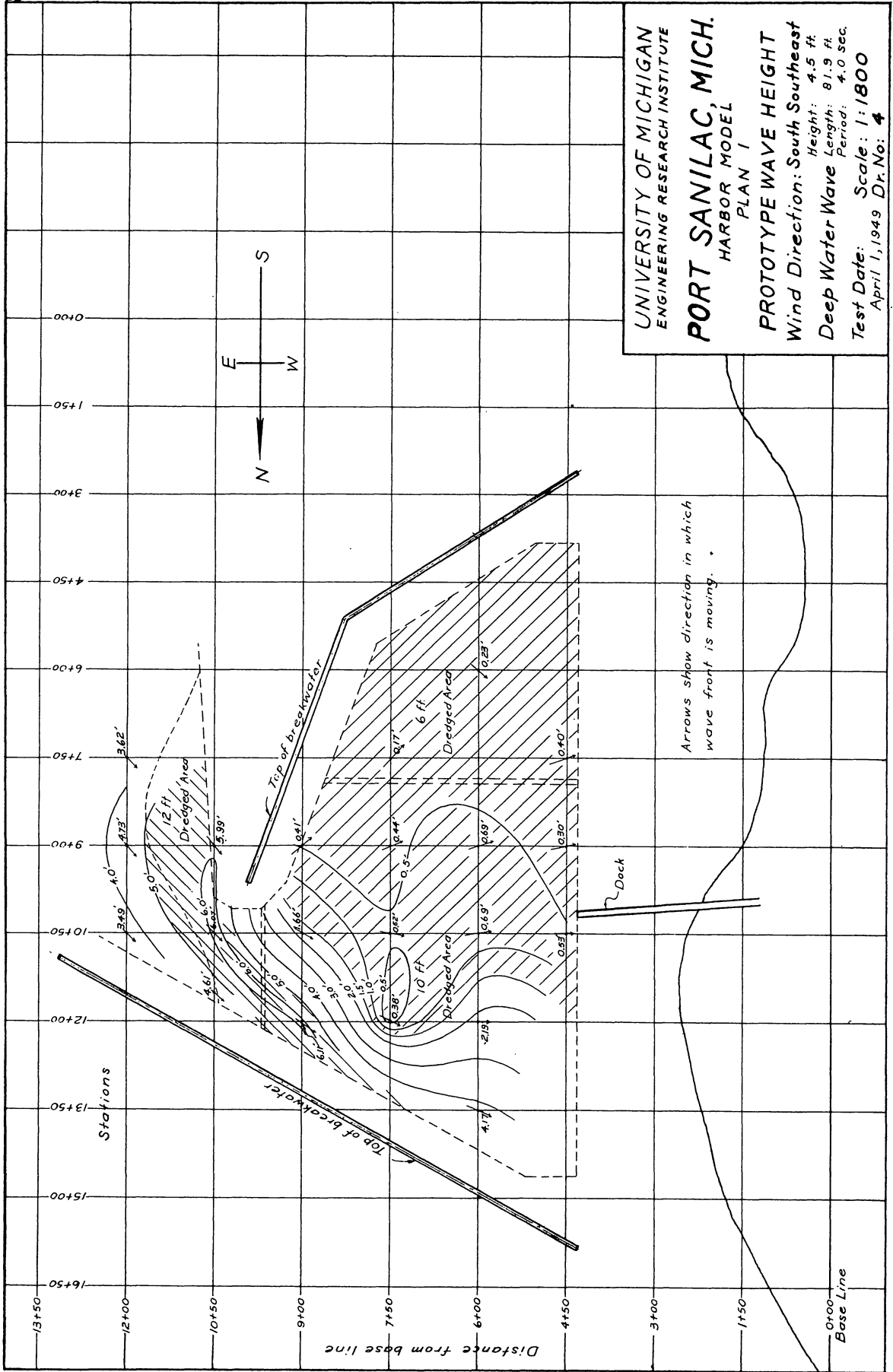
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ENGINEERING RESEARCH INSTITUTE  
**PORT SANILAC, MICH.**  
HARBOR MODEL  
TEMPLATES  
Scale: 1:5000 Dr. No. 2



Drawing 2



UNIVERSITY OF MICHIGAN  
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**PORT SANILAC, MICH.**  
 HARBOR MODEL  
 PLAN I  
**PROTOTYPE WAVE HEIGHT**  
 Wind Direction: South Southeast  
 Height: 8.7 ft.  
 Length: 90.3 ft.  
 Deep Water Wave Period: 4.2 Sec.  
 Test Date: March 30, 1949  
 Scale: 1:1800  
 Dr. No: 3



Drawing 4

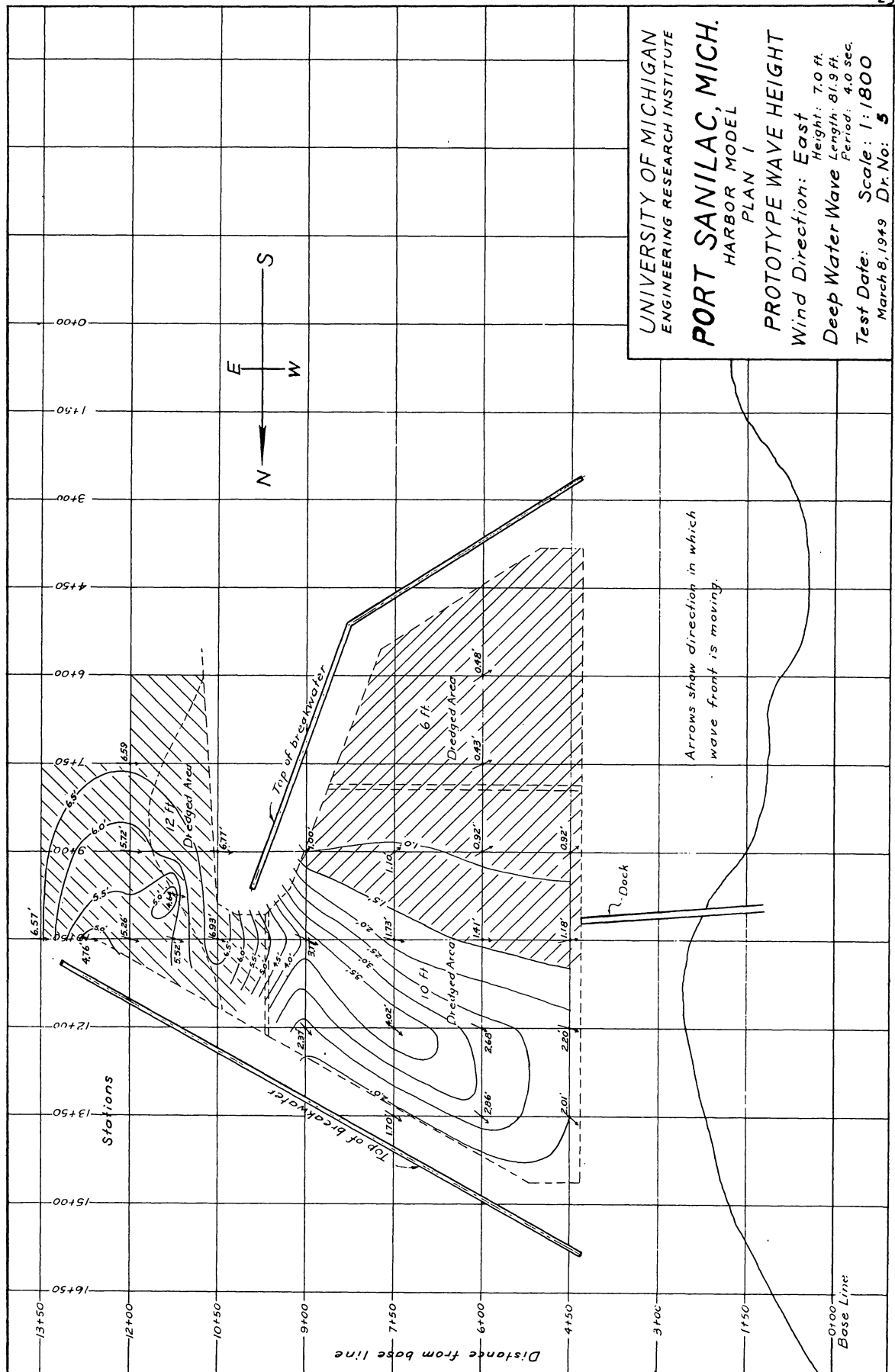
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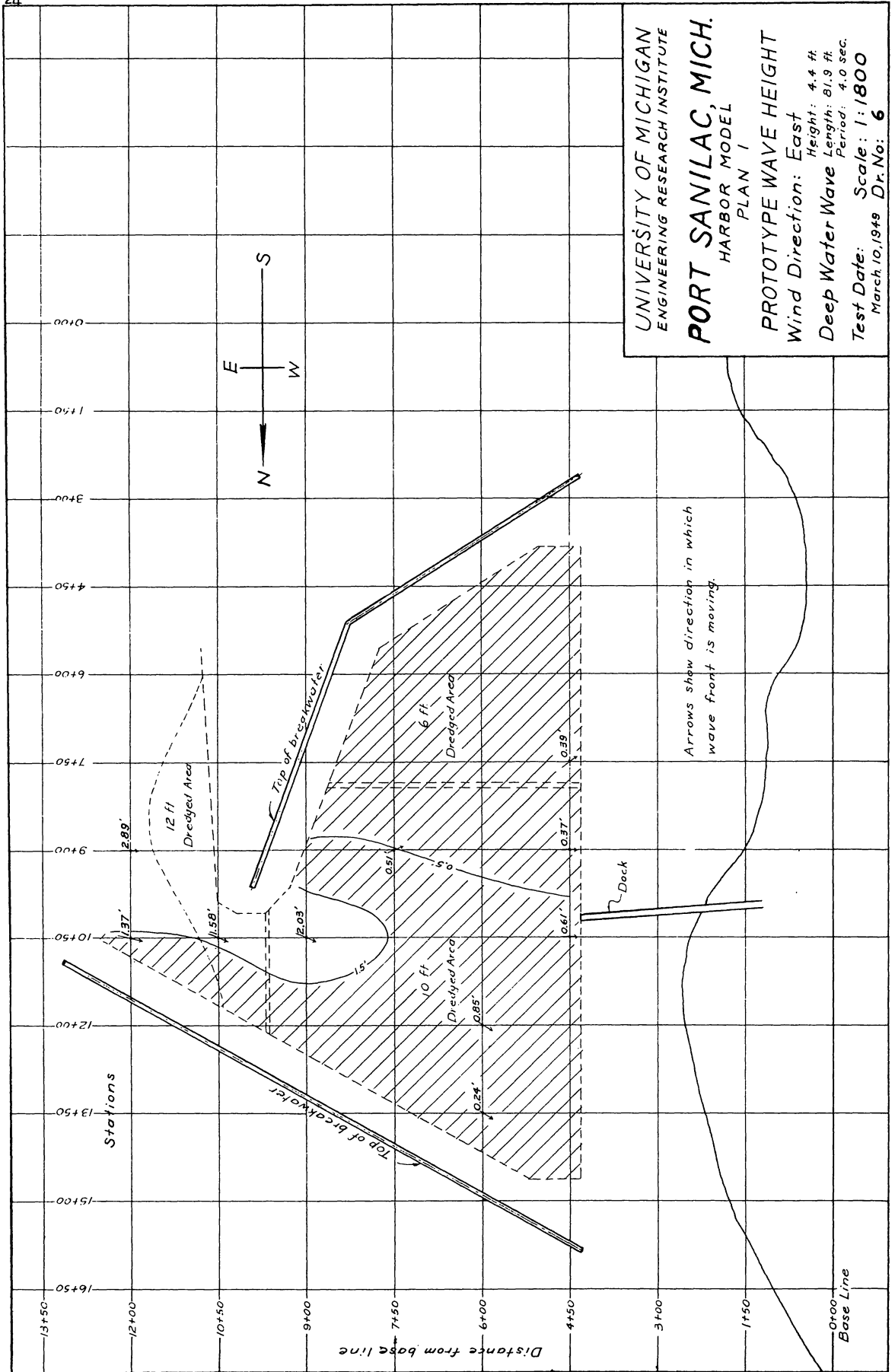
# PORT SANILAC, MICH. HARBOR MODEL PLAN 1

## PROTOTYPE WAVE HEIGHT

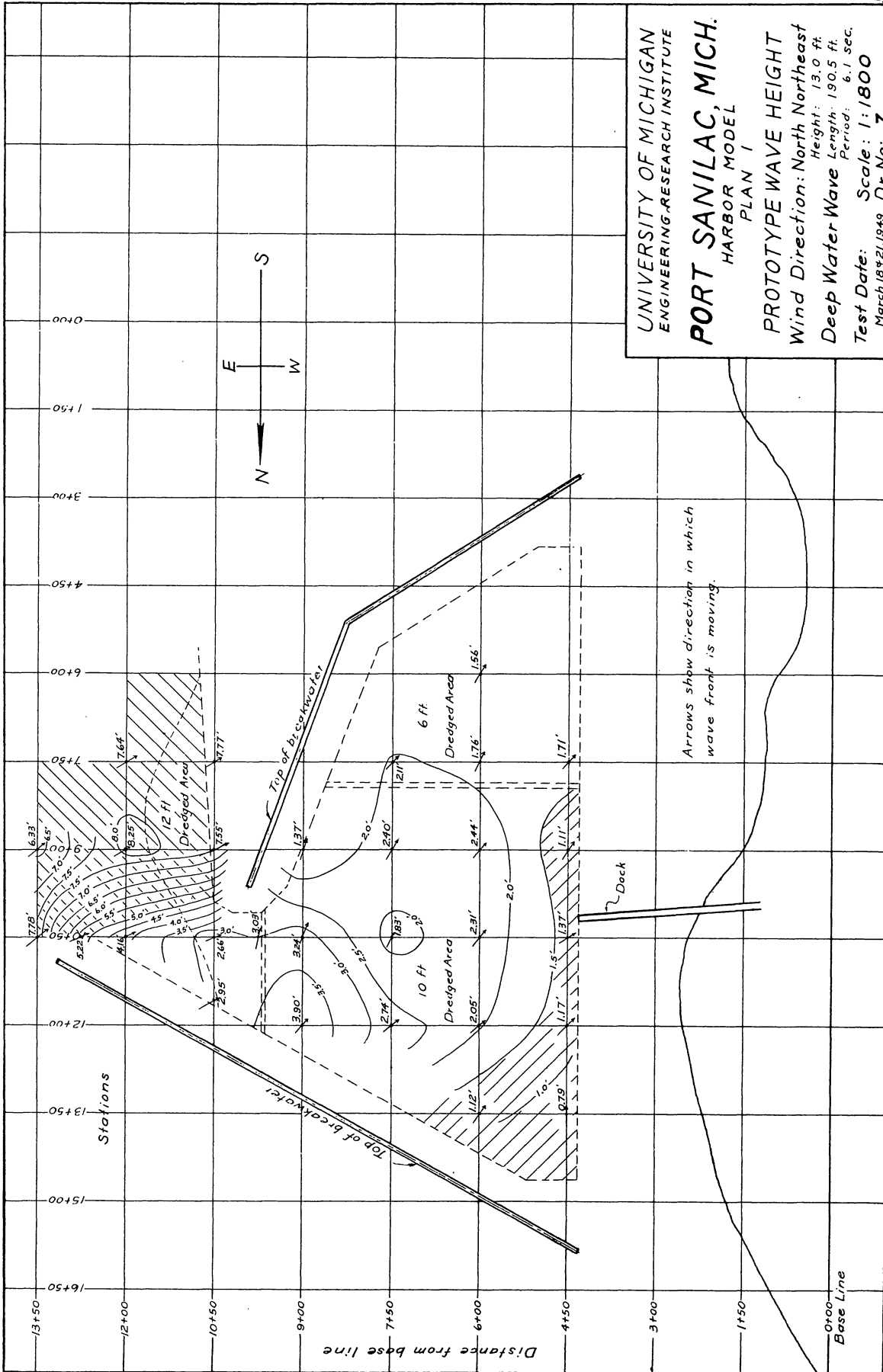
Wind Direction: East  
Height: 7.0 ft.  
Deep Water Wave Length: 81.9 ft.  
Period: 4.0 sec.

Test Date: March 8, 1949 Scale: 1:1800 Dr. No: 5





Drawing 9



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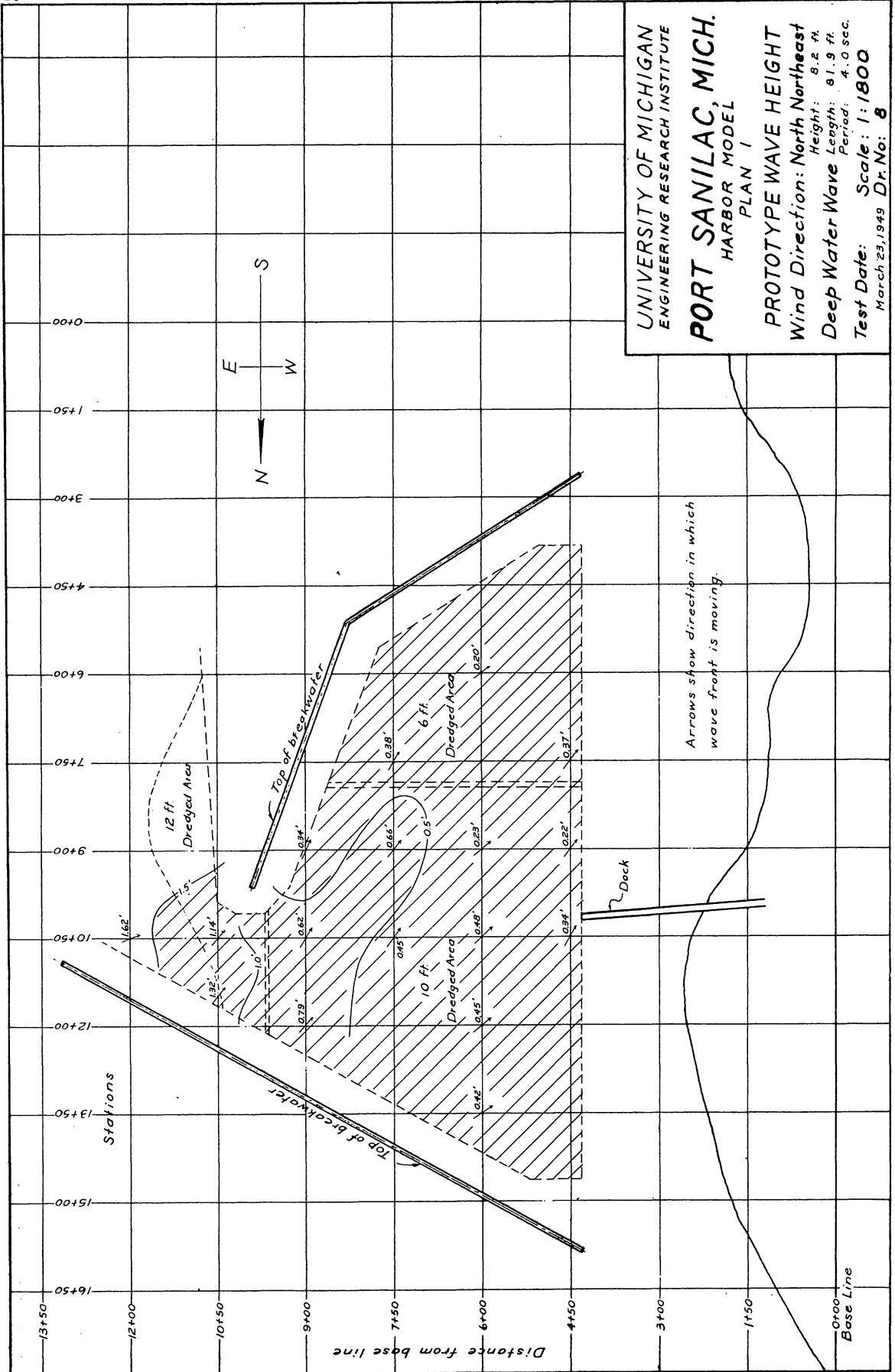
**PORT SANILAC, MICH.**  
HARBOR MODEL  
PLAN 1

PROTOTYPE WAVE HEIGHT  
Wind Direction: North Northeast  
Deep Water Wave Height: 13.0 ft  
Length: 190.5 ft  
Period: 6.1 sec.

Test Date: March 18, 21, 1949  
Scale: 1:1800  
Dr. No: 7

Arrows show direction in which  
wave front is moving.

Drawing 7



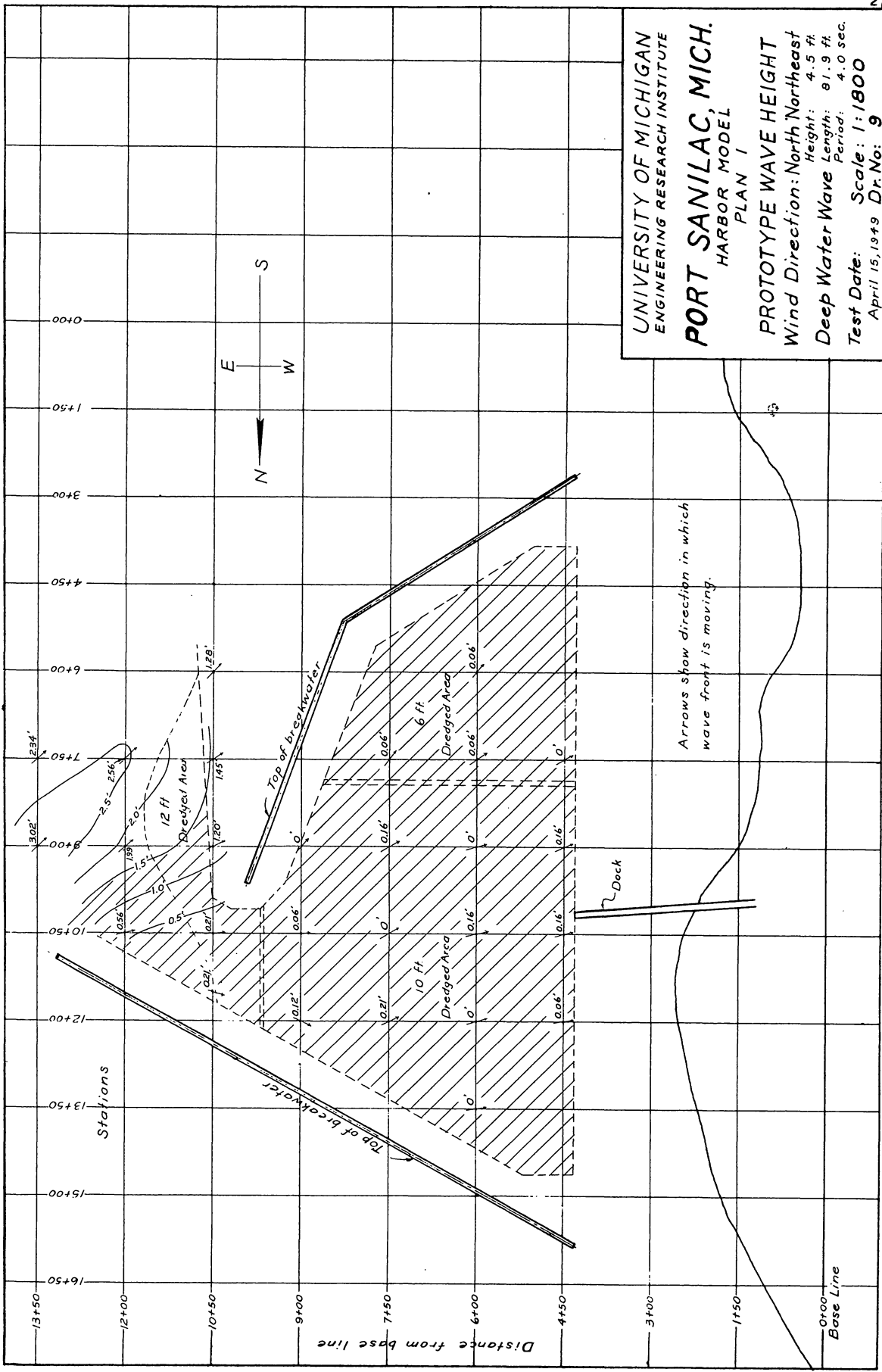
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**PORT SANILAC, MICH.**  
HARBOR MODEL  
PLAN 1

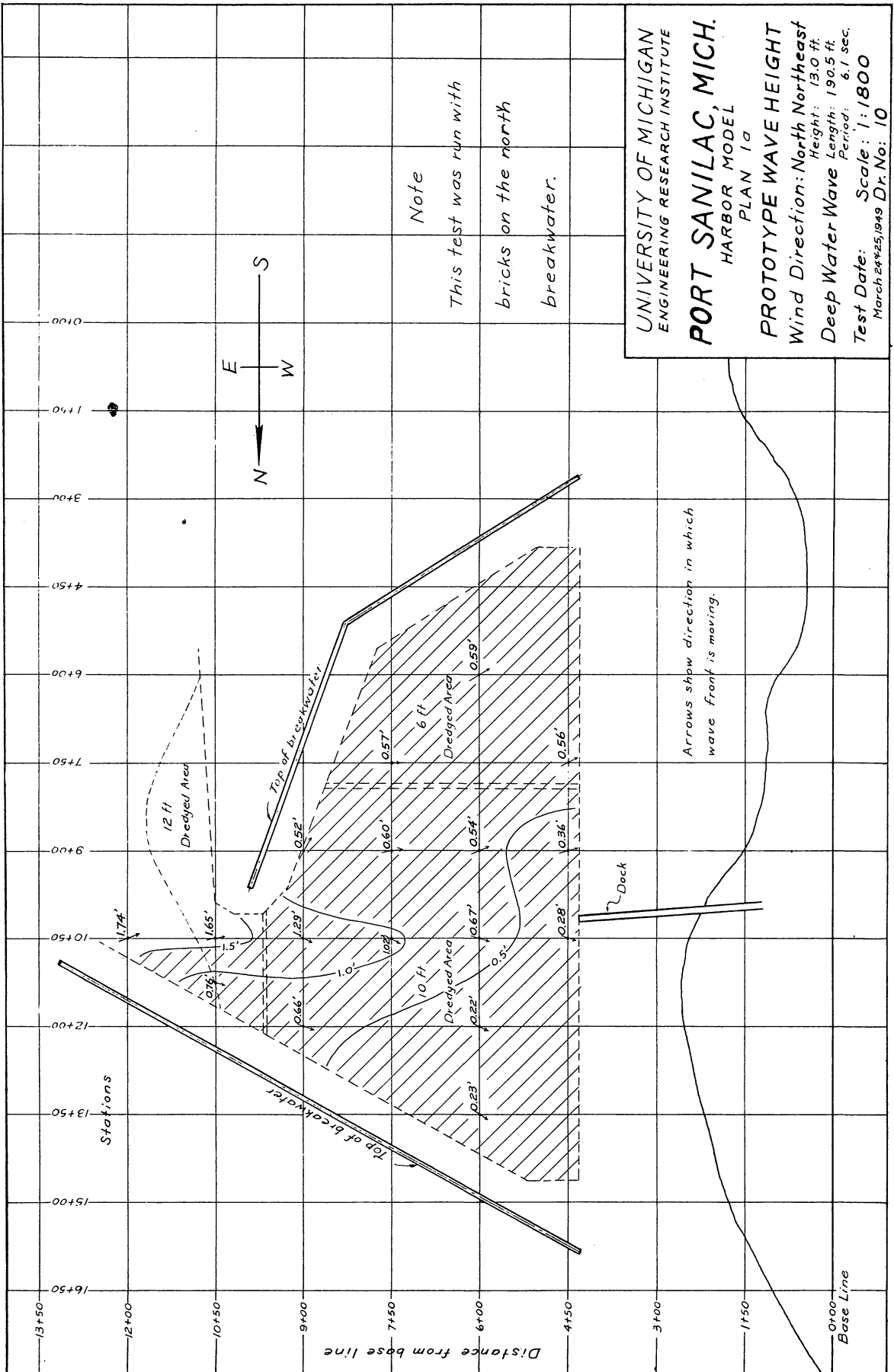
**PROTOTYPE WAVE HEIGHT**  
Wind Direction: North Northeast  
Height: 8.2 ft.  
Deep Water Wave Length: 81.9 ft.  
Period: 4.0 sec.  
Test Date: March 23, 1949  
Scale: 1:1800  
Dr. No: 8

Drawing 8

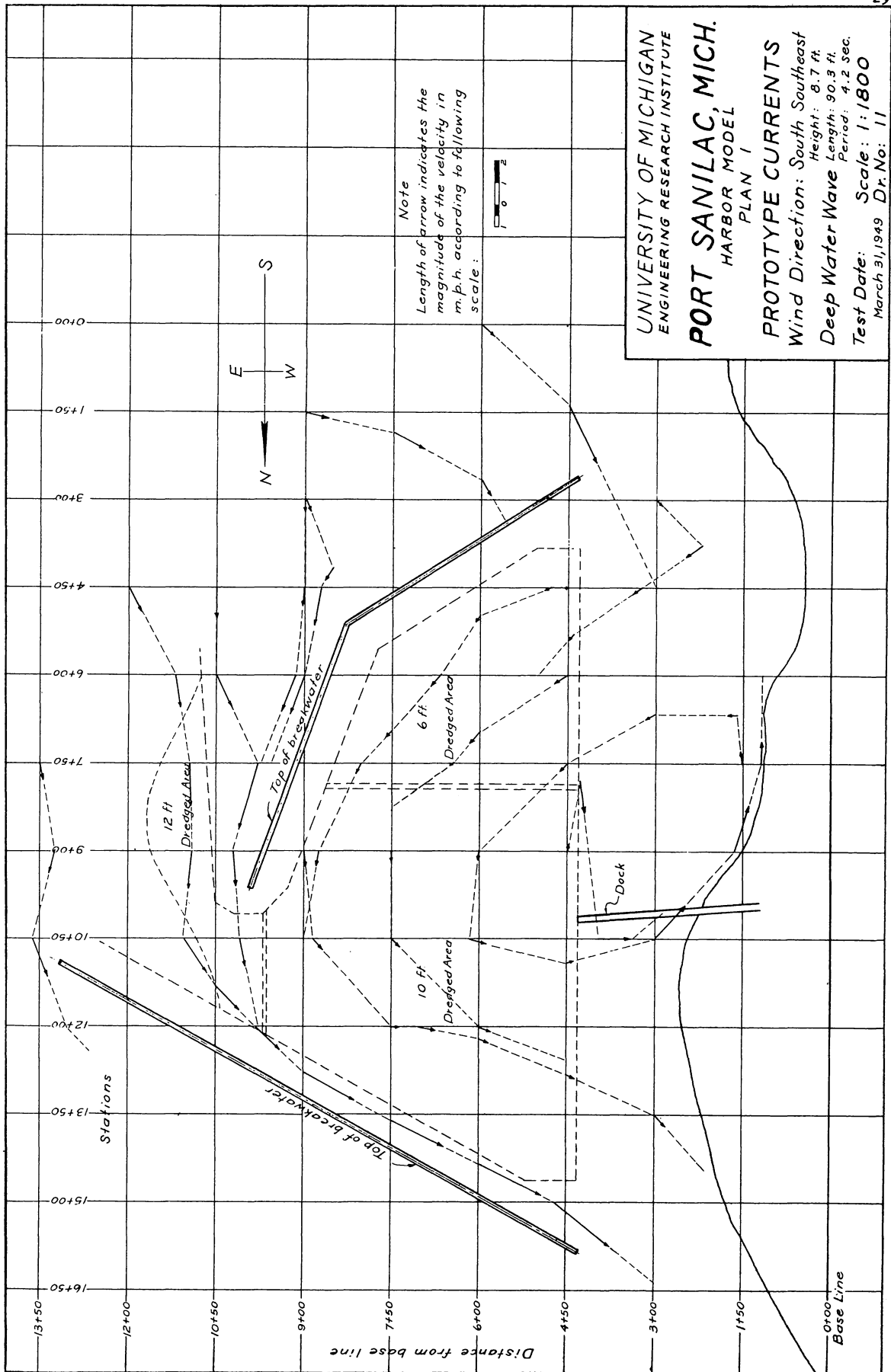




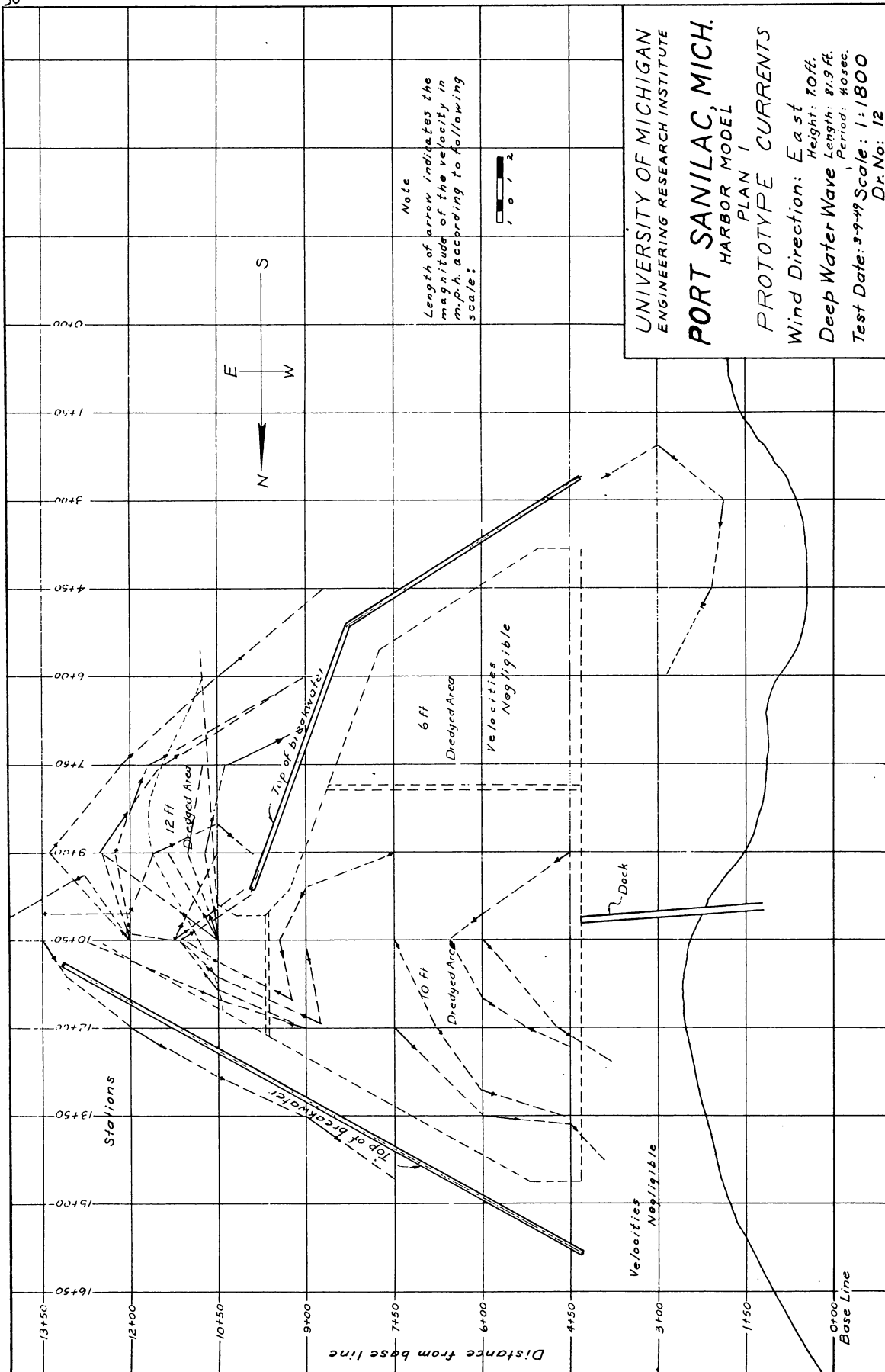
UNIVERSITY OF MICHIGAN  
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**PORT SANILAC, MICH.**  
 HARBOR MODEL  
 PLAN 1  
**PROTOTYPE WAVE HEIGHT**  
 Wind Direction: North Northeast  
 Height: 4.5 ft  
 Deep Water Wave Length: 81.9 ft  
 Period: 4.0 sec.  
 Test Date: April 15, 1949  
 Scale: 1:1800  
 Dr. No: 9



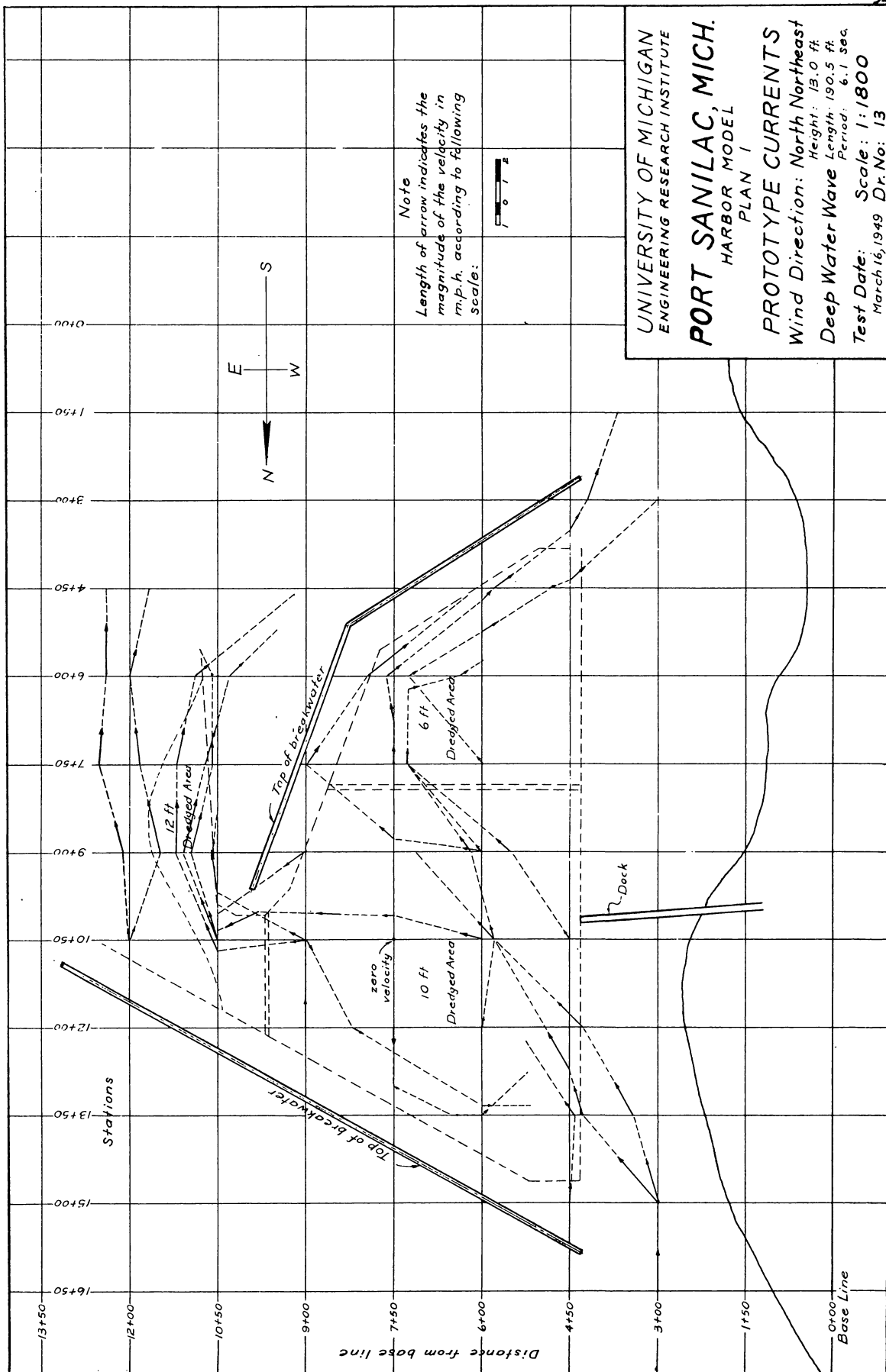
Drawing 10

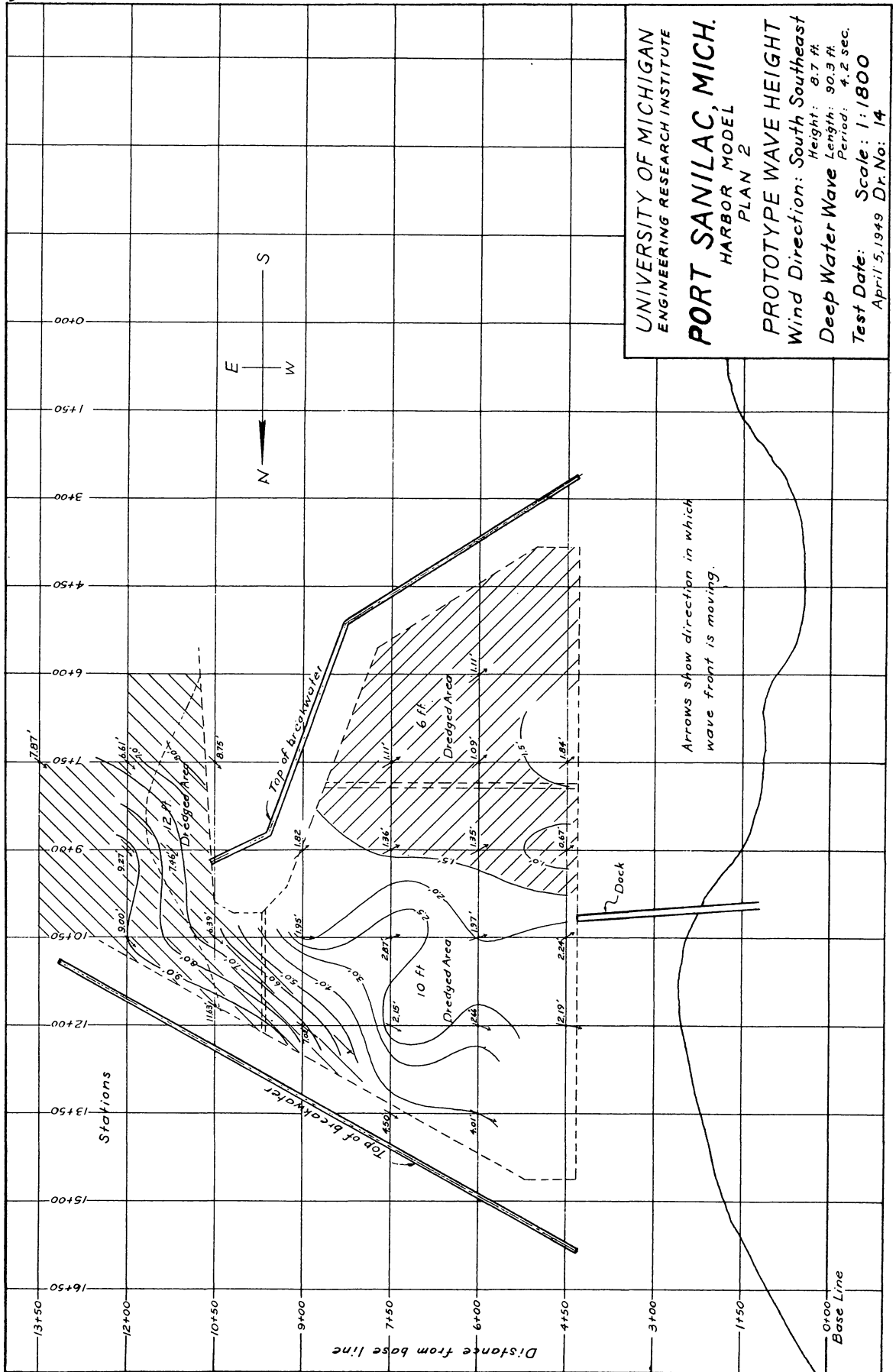


Drawing 11



Drawing 12





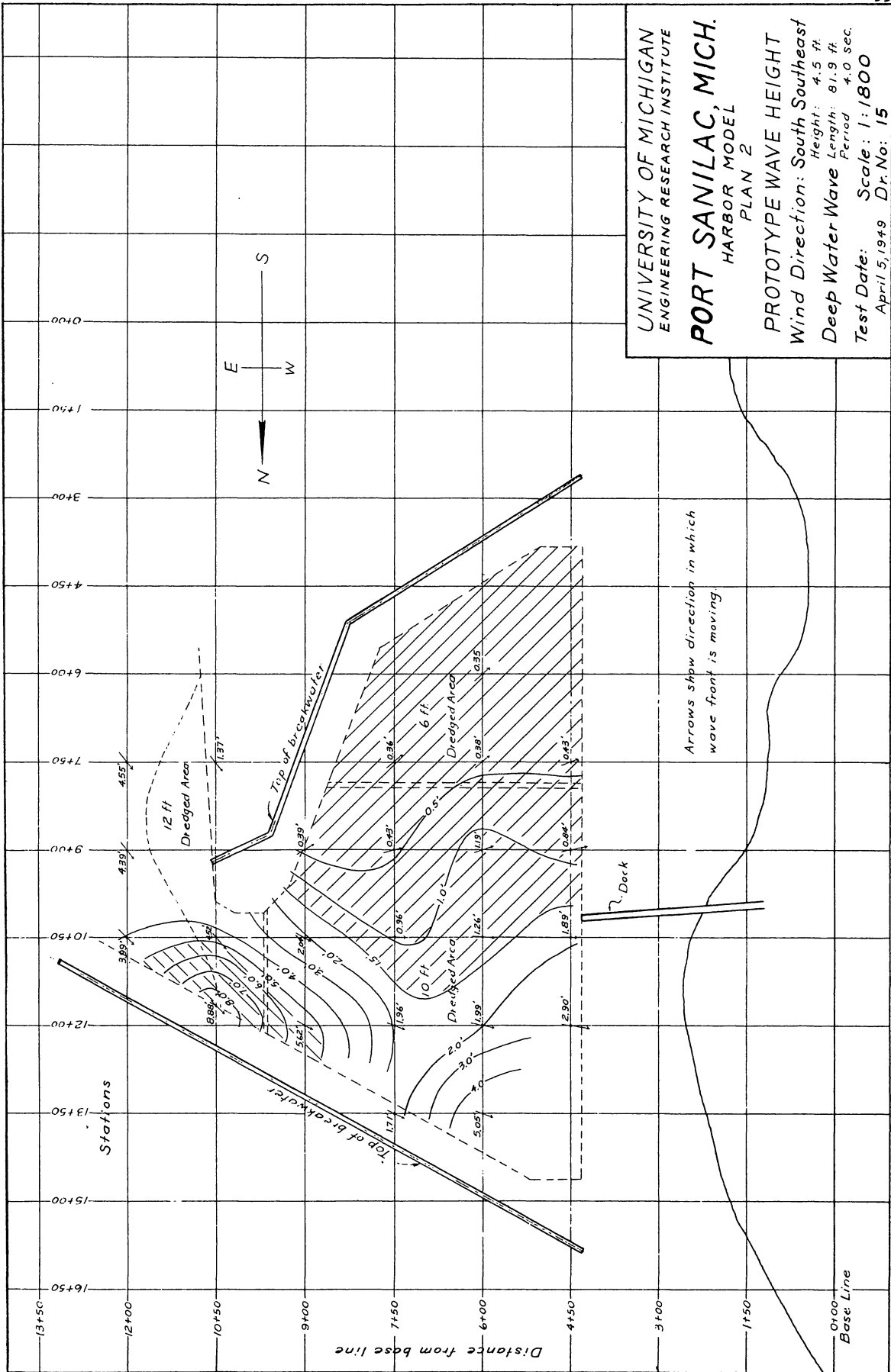
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**PORT SANILAC, MICH.**  
HARBOR MODEL  
PLAN 2

PROTOTYPE WAVE HEIGHT  
Wind Direction: South Southeast Height: 8.7 ft.  
Deep Water Wave Length: 90.3 ft.  
Period: 4.2 sec.

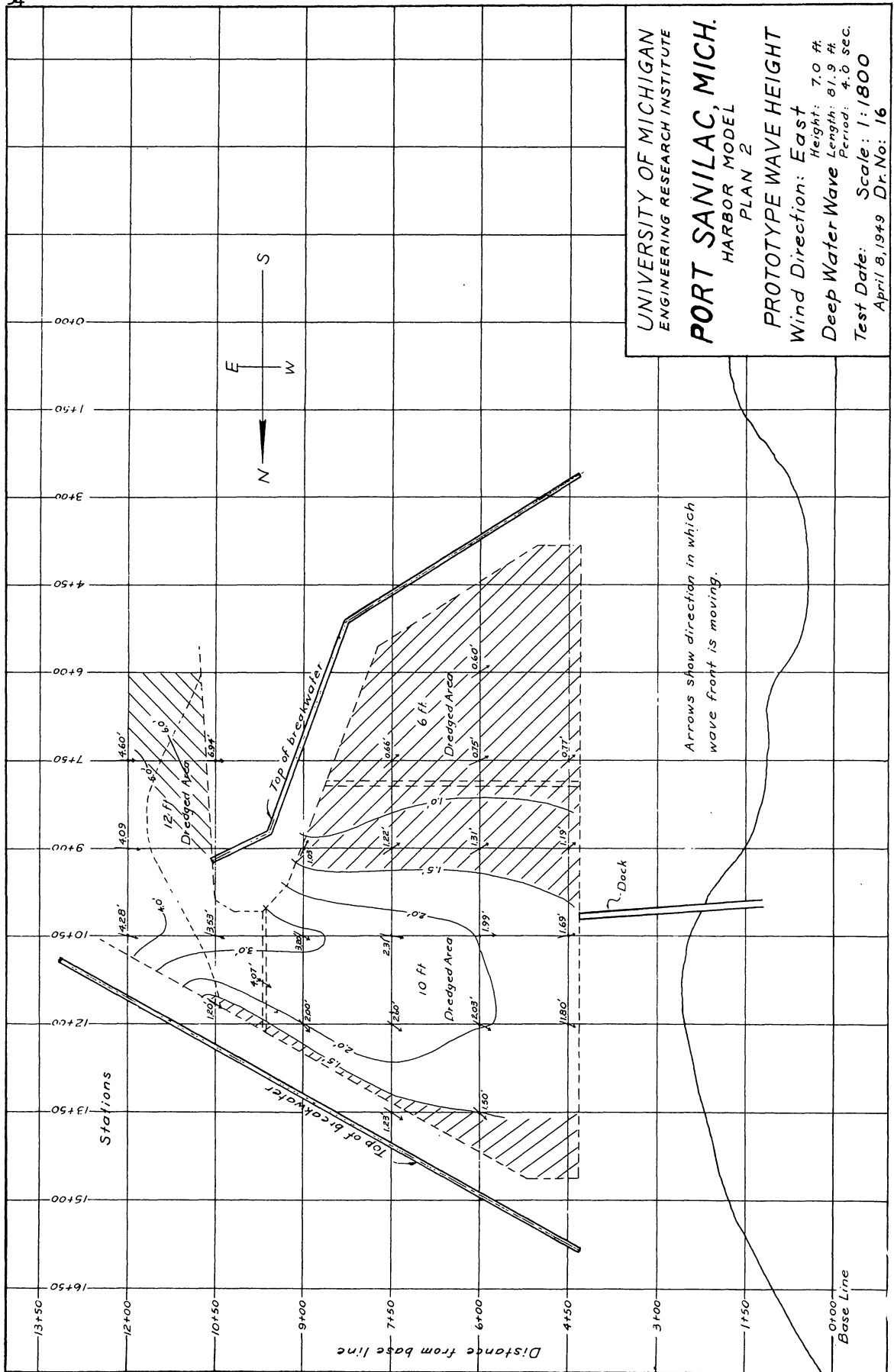
Test Date: April 5, 1949 Scale: 1:1800 Dr. No: 14

Drawing 14



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**PORT SANILAC, MICH.**  
 HARBOR MODEL  
 PLAN 2  
 PROTOTYPE WAVE HEIGHT  
 Wind Direction: South Southeast  
 Height: 4.5 ft  
 Deep Water Wave Length: 81.9 ft  
 Period: 4.0 sec.  
 Test Date: April 5, 1949  
 Scale: 1:1800  
 Dr.No: 15

Drawing 15



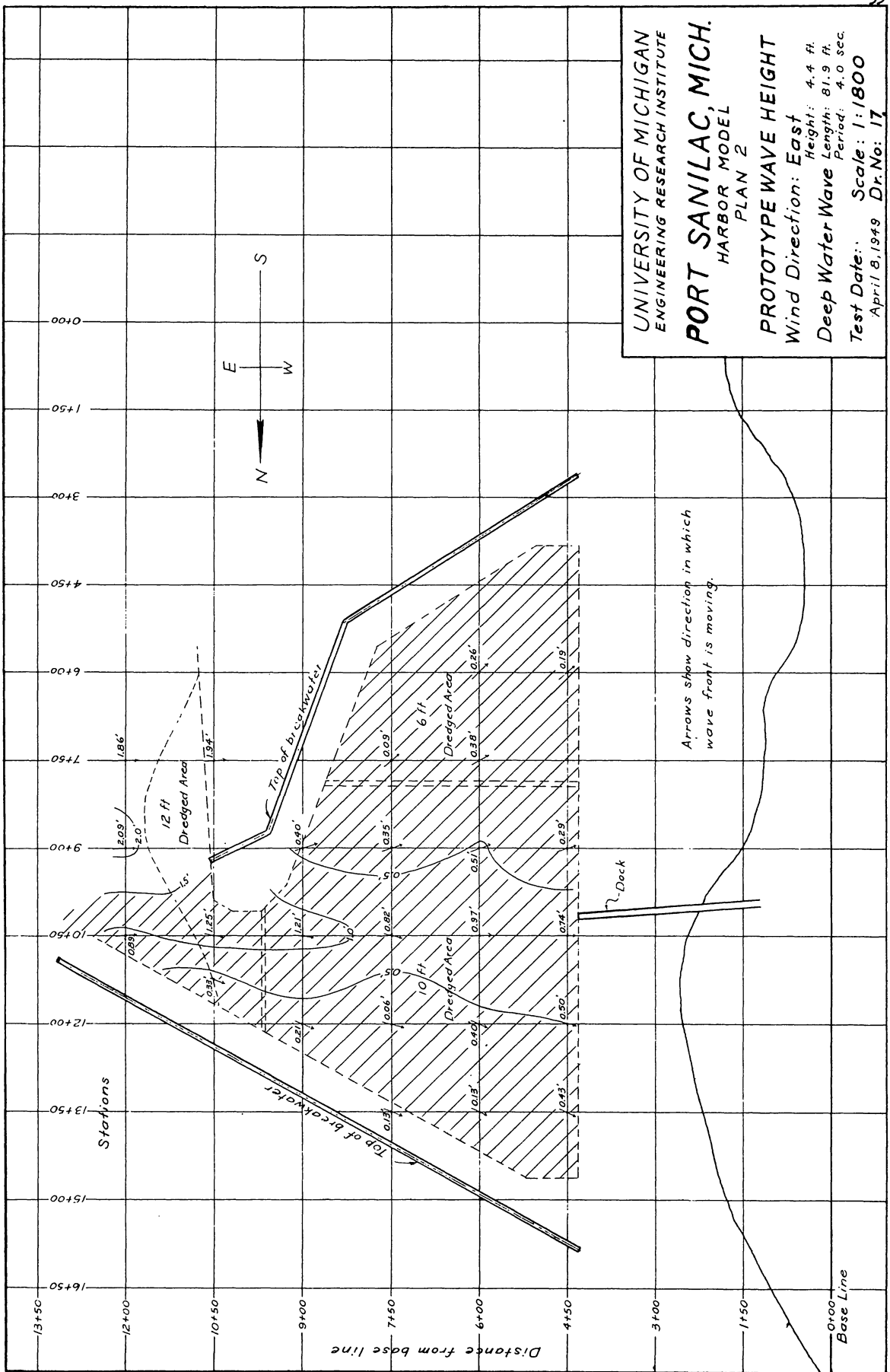
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**PORT SANILAC, MICH.**  
HARBOR MODEL  
PLAN 2

**PROTOTYPE WAVE HEIGHT**  
Wind Direction: East  
Height: 7.0 ft  
Deep Water Wave Length: 81.9 ft  
Period: 4.0 sec.  
Test Date: April 8, 1949  
Scale: 1:1800  
Dr. No: 16

Drawing 16

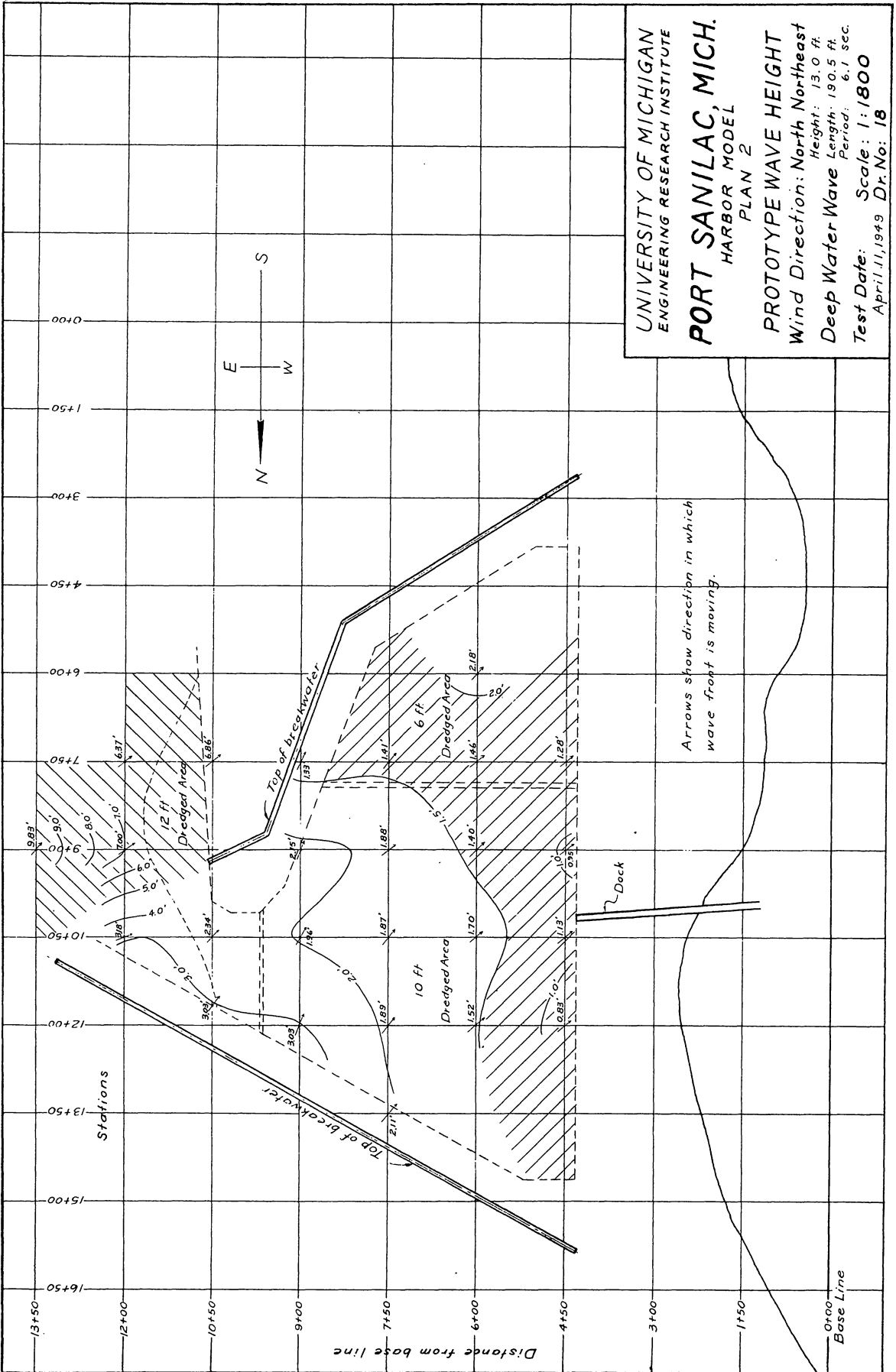




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**PORT SANILAC, MICH.**  
 HARBOR MODEL  
 PLAN 2

**PROTOTYPE WAVE HEIGHT**  
 Wind Direction: East  
 Height: 4.4 ft  
 Deep Water Wave Length: 81.9 ft.  
 Period: 4.0 Sec.  
 Test Date: April 8, 1949  
 Scale: 1:1800  
 Dr. No: 17



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**PORT SANILAC, MICH.**  
HARBOR MODEL  
PLAN 2

PROTOTYPE WAVE HEIGHT

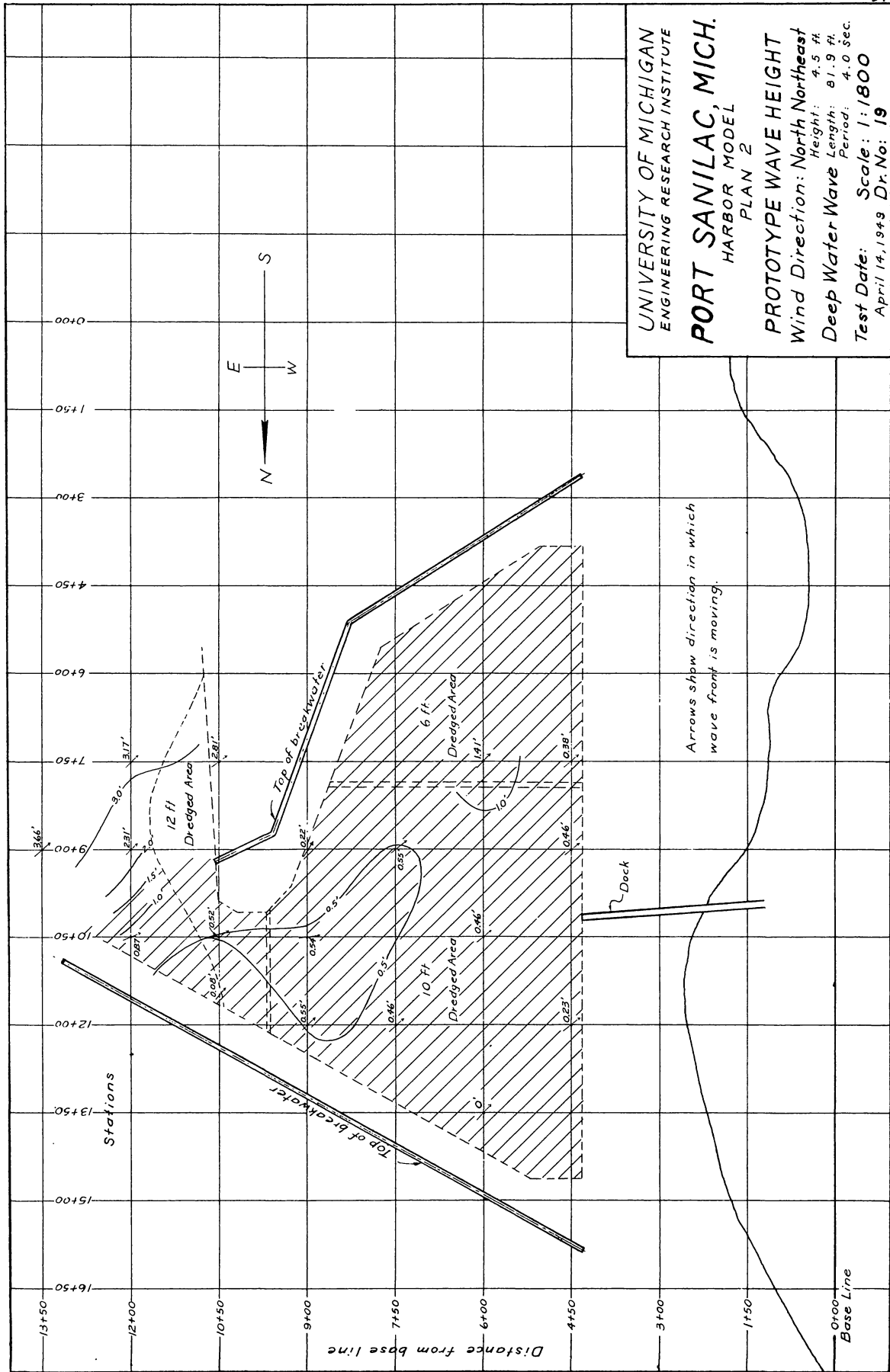
Wind Direction: North-Northeast  
Height: 13.0 ft  
Length: 190.5 ft  
Period: 6.1 sec.

Test Date: April 11, 1949  
Scale: 1:1800  
Dr. No: 18

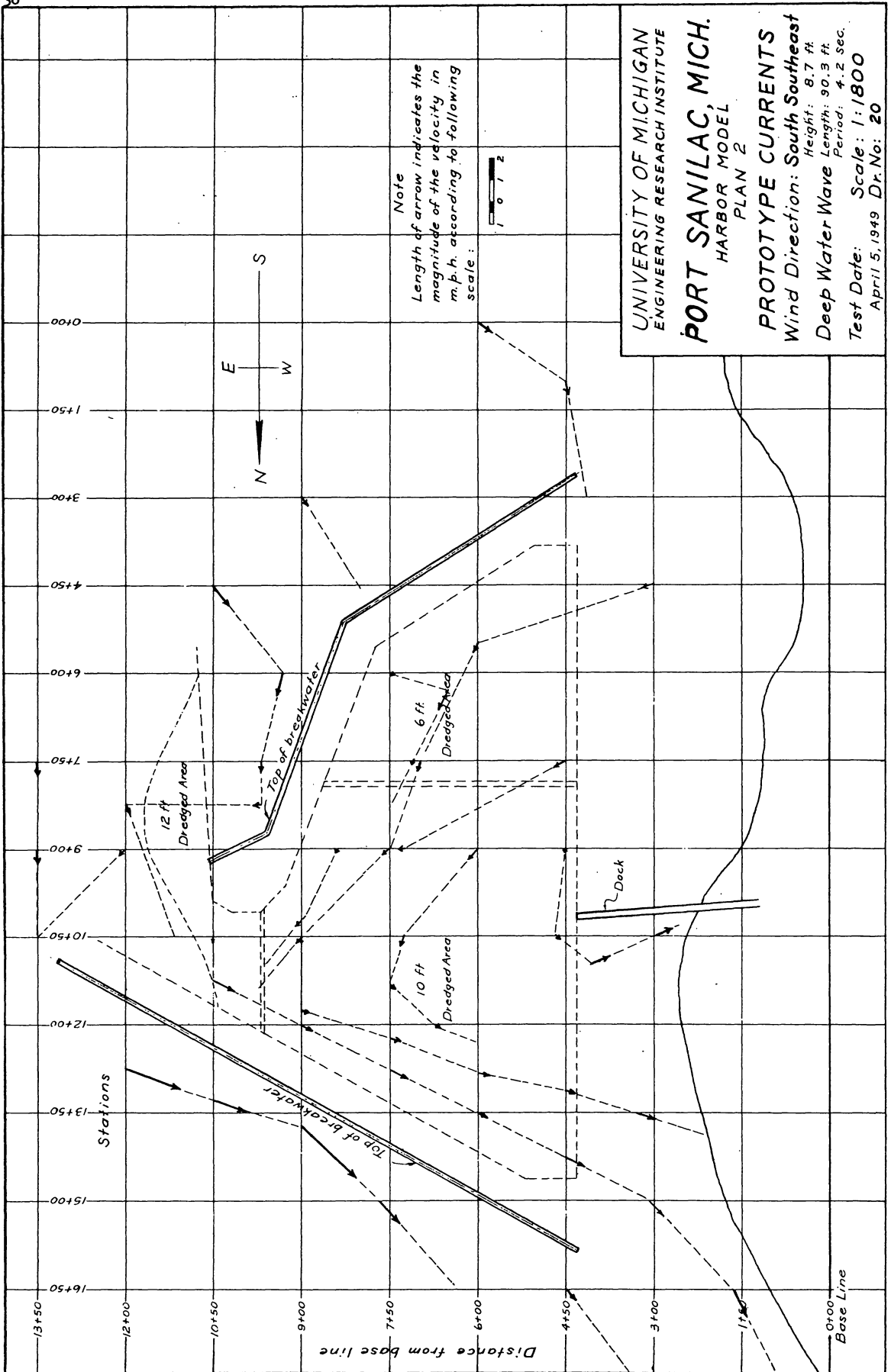
Arrows show direction in which  
wave front is moving.

Drawing 18

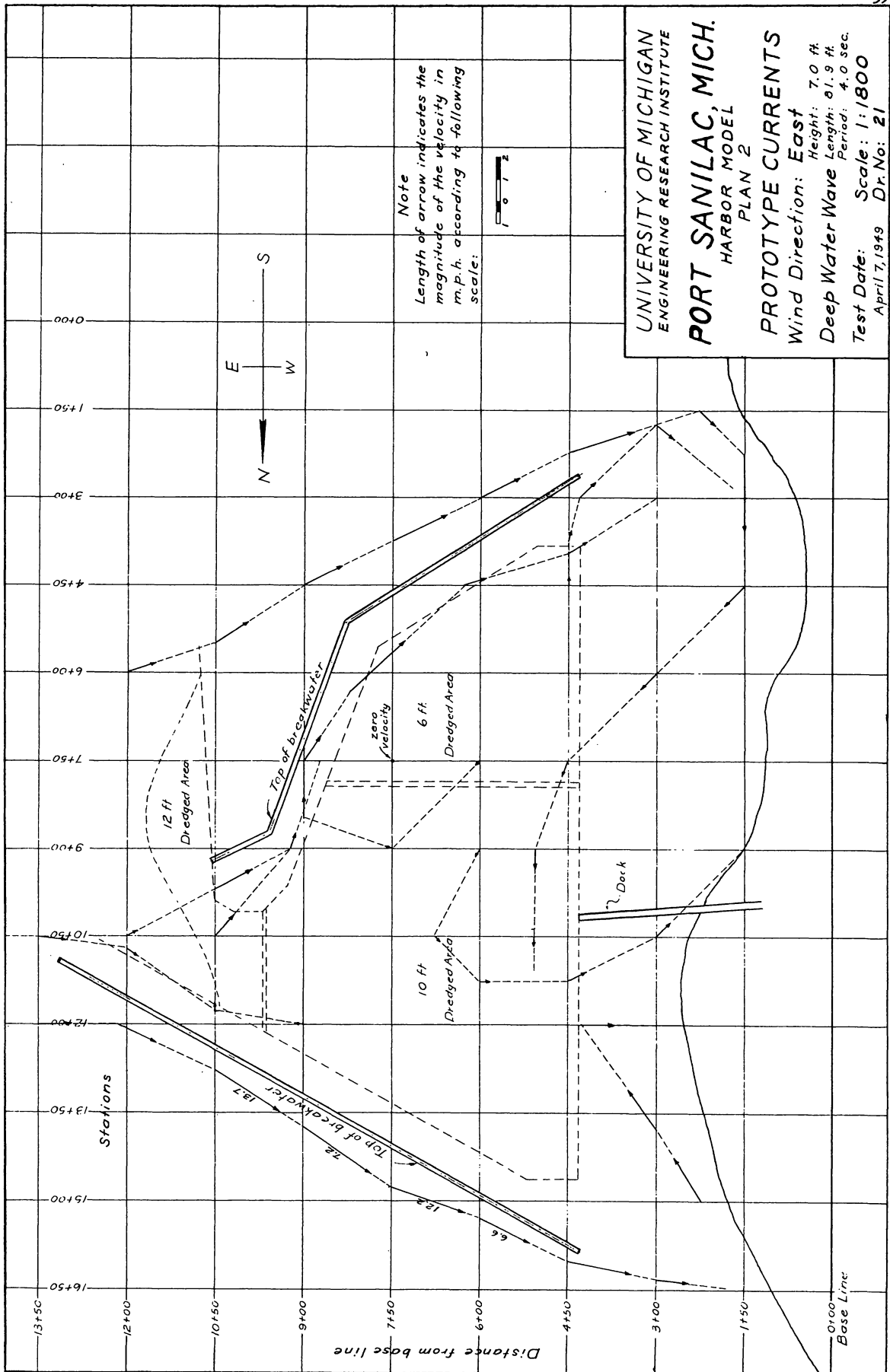
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**PORT SANILAC, MICH.**  
 HARBOR MODEL  
 PLAN 2  
**PROTOTYPE WAVE HEIGHT**  
 Wind Direction: North Northeast  
 Height: 4.5 ft.  
 Deep Water Wave Length: 81.9 ft.  
 Period: 4.0 Sec.  
 Test Date: Scale: 1:1800  
 April 14, 1949 Dr. No: 19



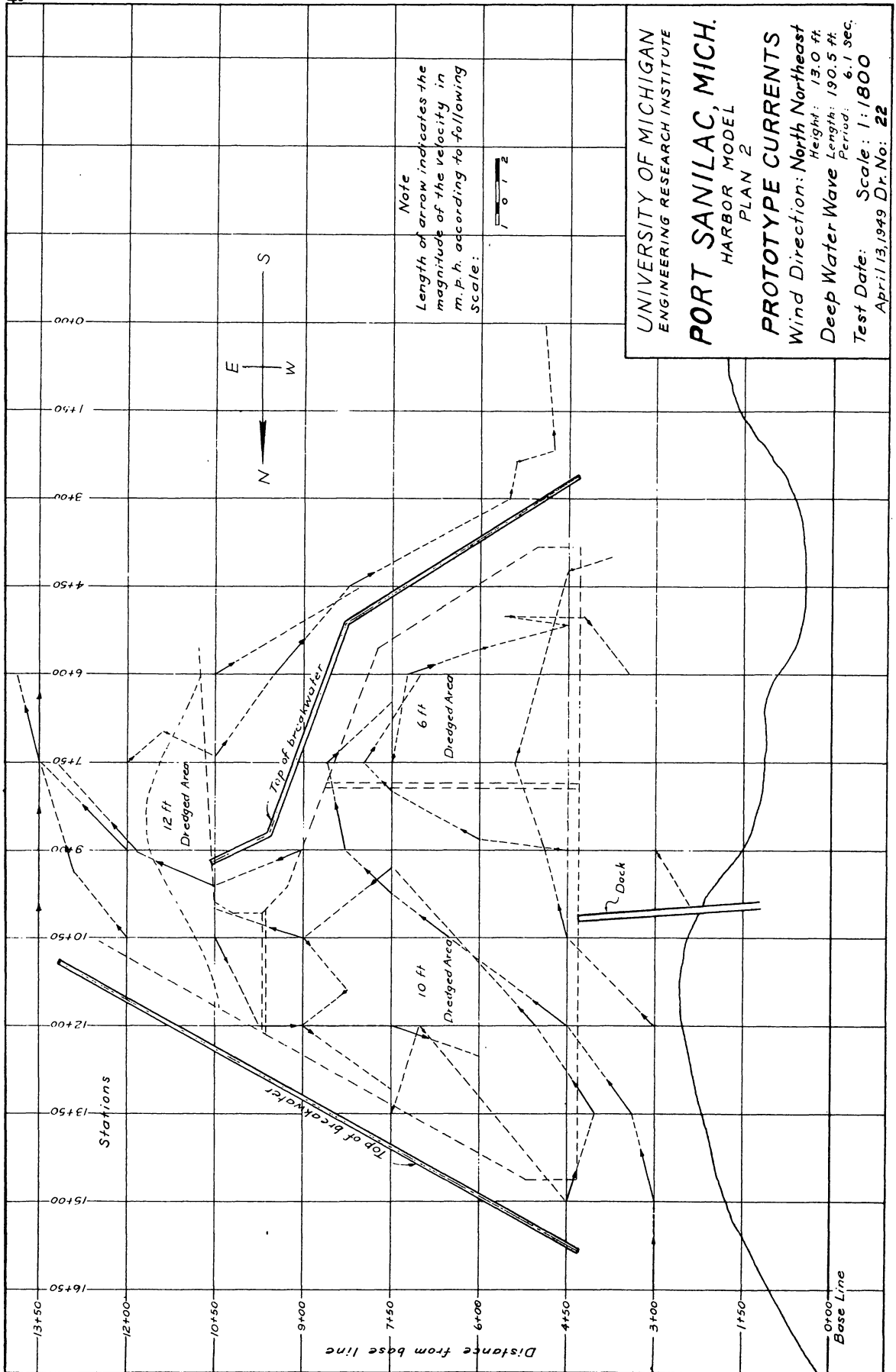
Drawing 19



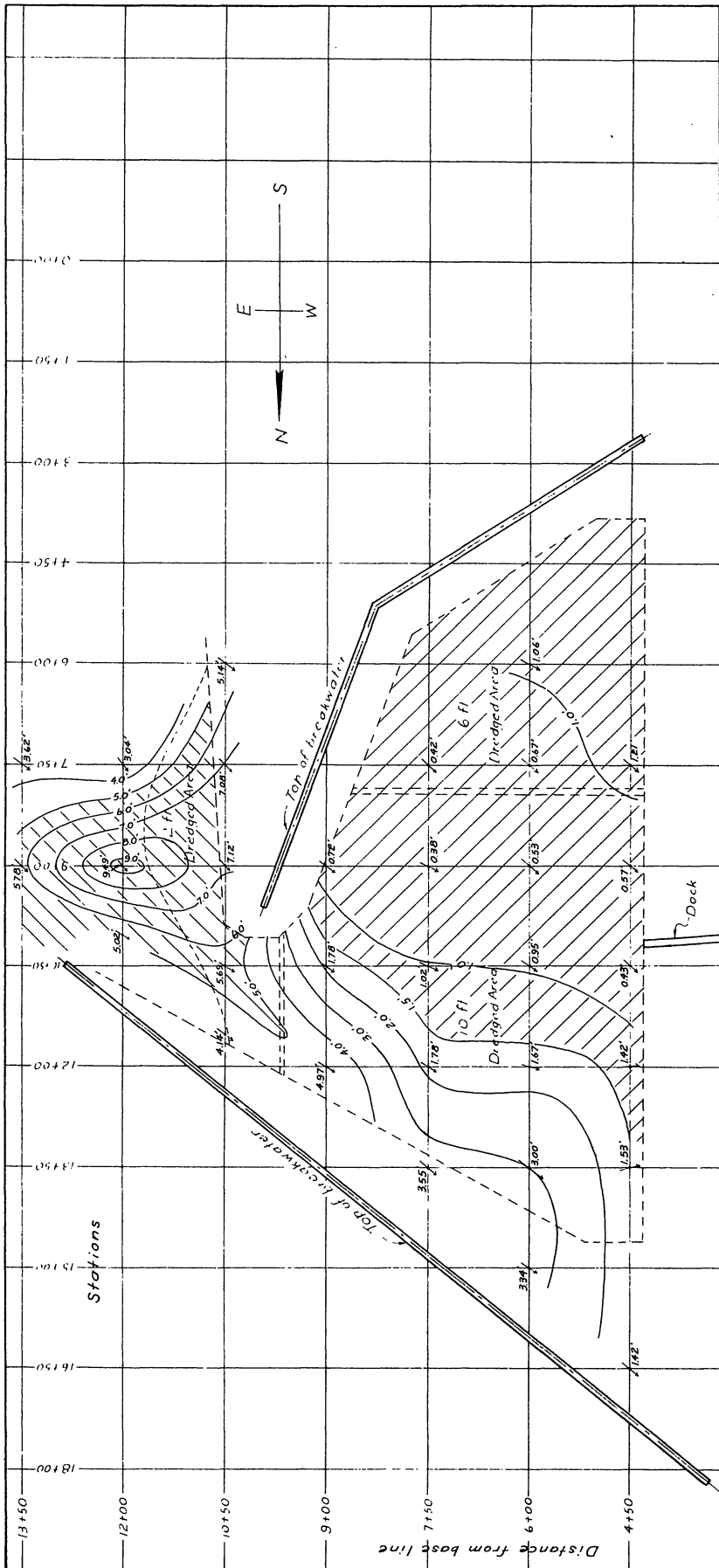
Drawing 20



Drawing 21



Drawing 22



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**PORT SANILAC, MICH.**  
HARBOR MODEL  
PLAN 3

**PROTOTYPE WAVE HEIGHT**

Wind Direction: South Southeast  
Height: 8.7 ft.

Deep Water Wave Length: 90.3 ft.  
Period: 4.2 sec.

Test Date: April 29, 1949  
Scale: 1:1800  
Dr. No: 23

Arrows show direction in which wave front is moving.

Base Line

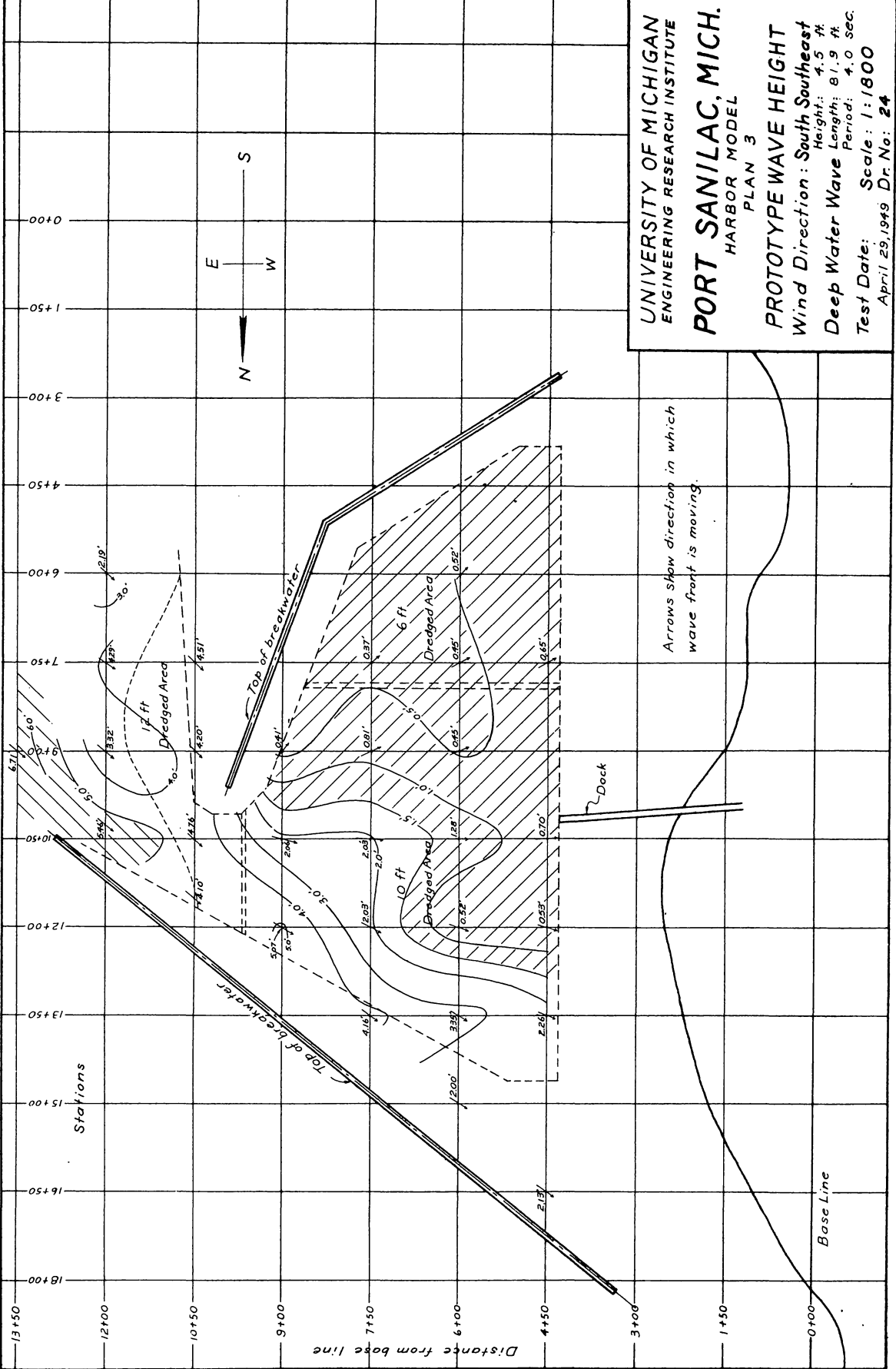
Dock

Top of Breakwater

Top of Pier

Distance from base line

Stations



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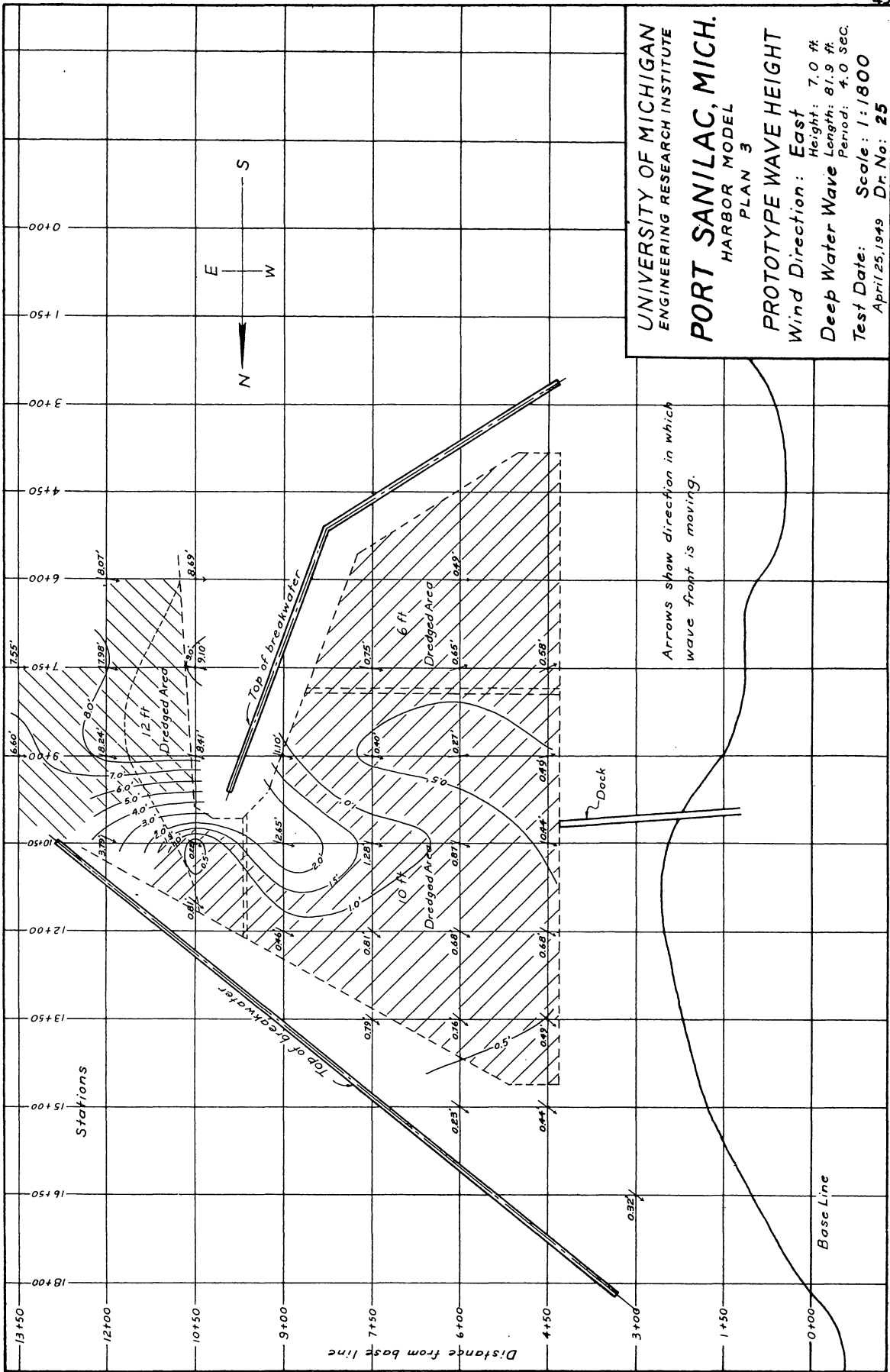
**PORT SANILAC, MICH.**  
HARBOR MODEL  
PLAN 3

**PROTOTYPE WAVE HEIGHT**  
Wind Direction: South Southeast  
Height: 4.5 ft  
Deep Water Wave Length: 81.9 ft  
Period: 4.0 sec.  
Test Date: April 29, 1949  
Scale: 1:1800  
Dr. No: 24

Arrows show direction in which wave front is moving.

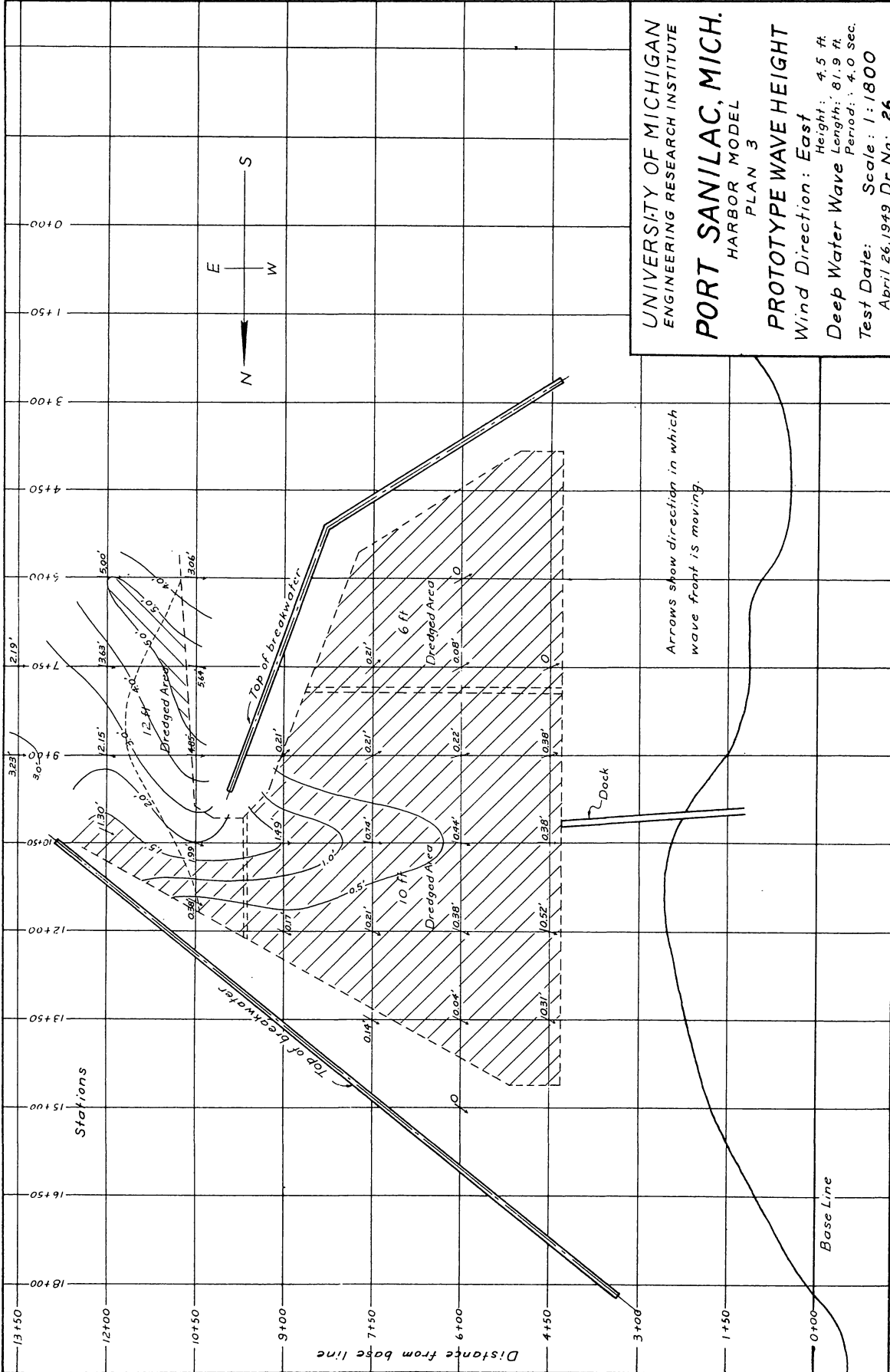
Drawing 24





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**PORT SANILAC, MICH.**  
 HARBOR MODEL  
 PLAN 3  
**PROTOTYPE WAVE HEIGHT**  
 Wind Direction: East Height: 7.0 ft  
 Deep Water Wave Length: 81.9 ft  
 Test Date: April 25, 1949 Period: 4.0 sec  
 Scale: 1:1800 Dr. No: 25

Arrows show direction in which wave front is moving.



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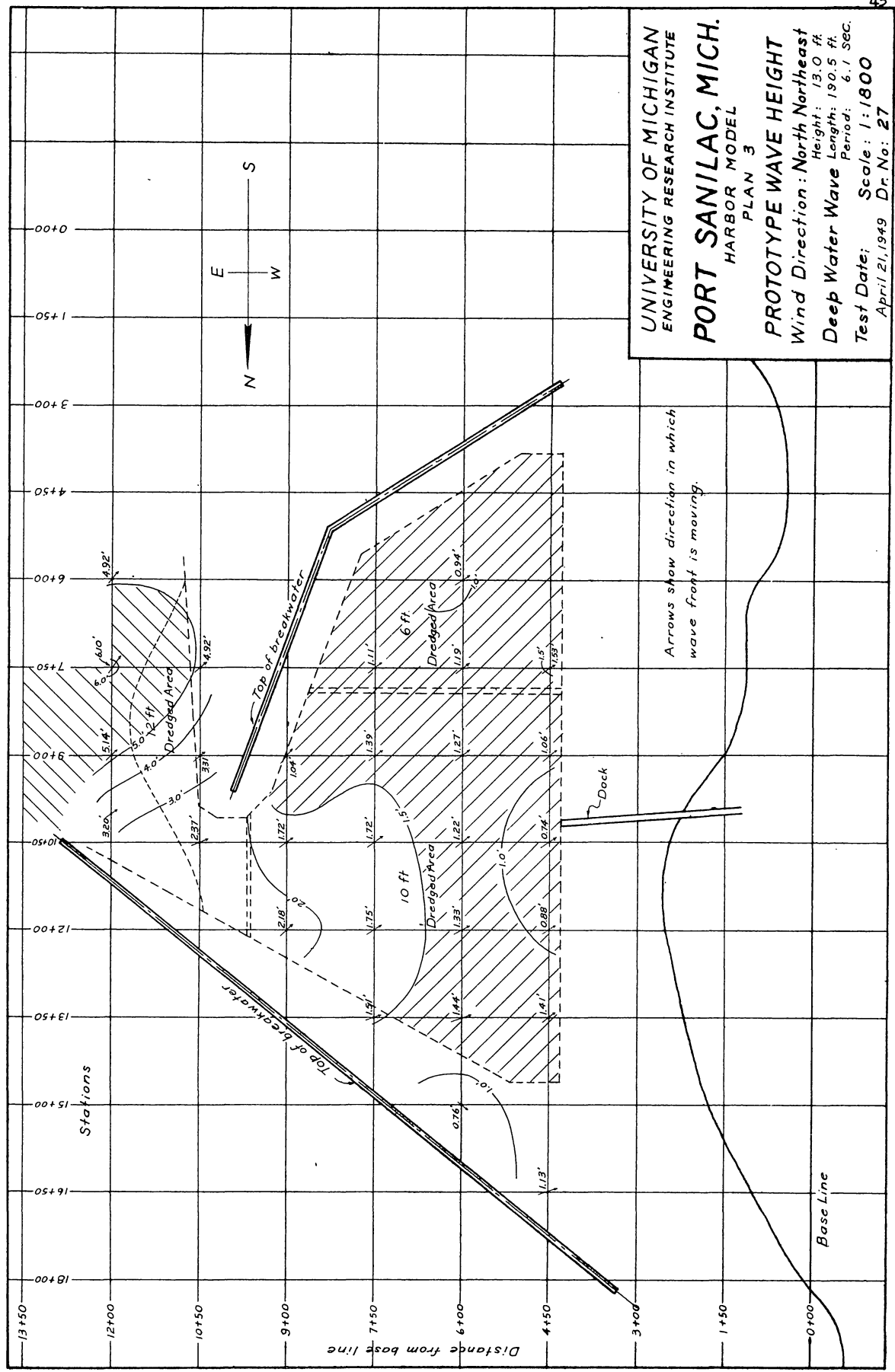
**PORT SANILAC, MICH.**  
HARBOR MODEL  
PLAN 3

**PROTOTYPE WAVE HEIGHT**

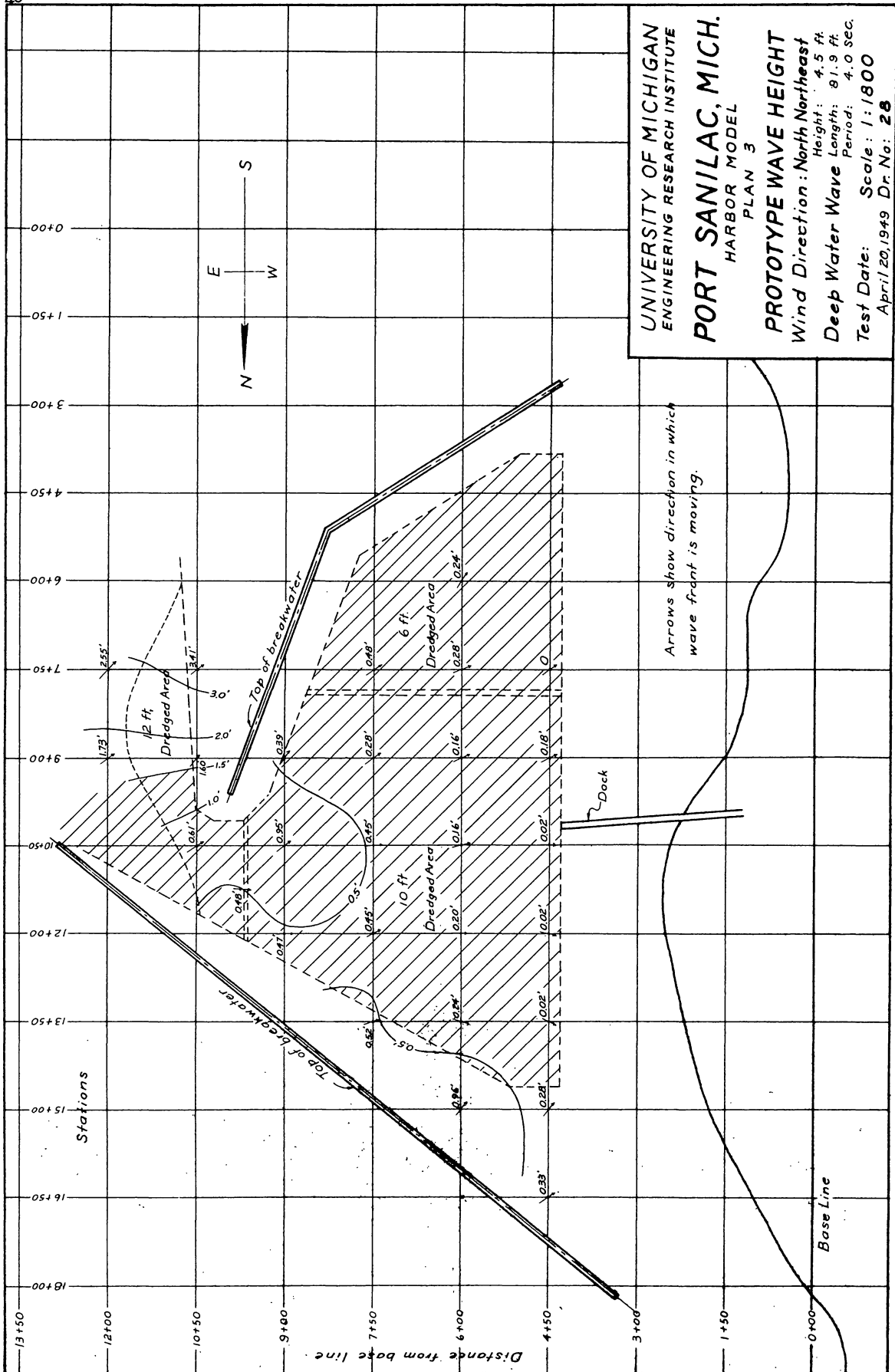
Wind Direction: East  
Height: 4.5 ft.  
Deep Water Wave Length: 81.9 ft.  
Period: 4.0 sec.

Test Date: April 26, 1949  
Scale: 1:1800  
Dr. No: 26

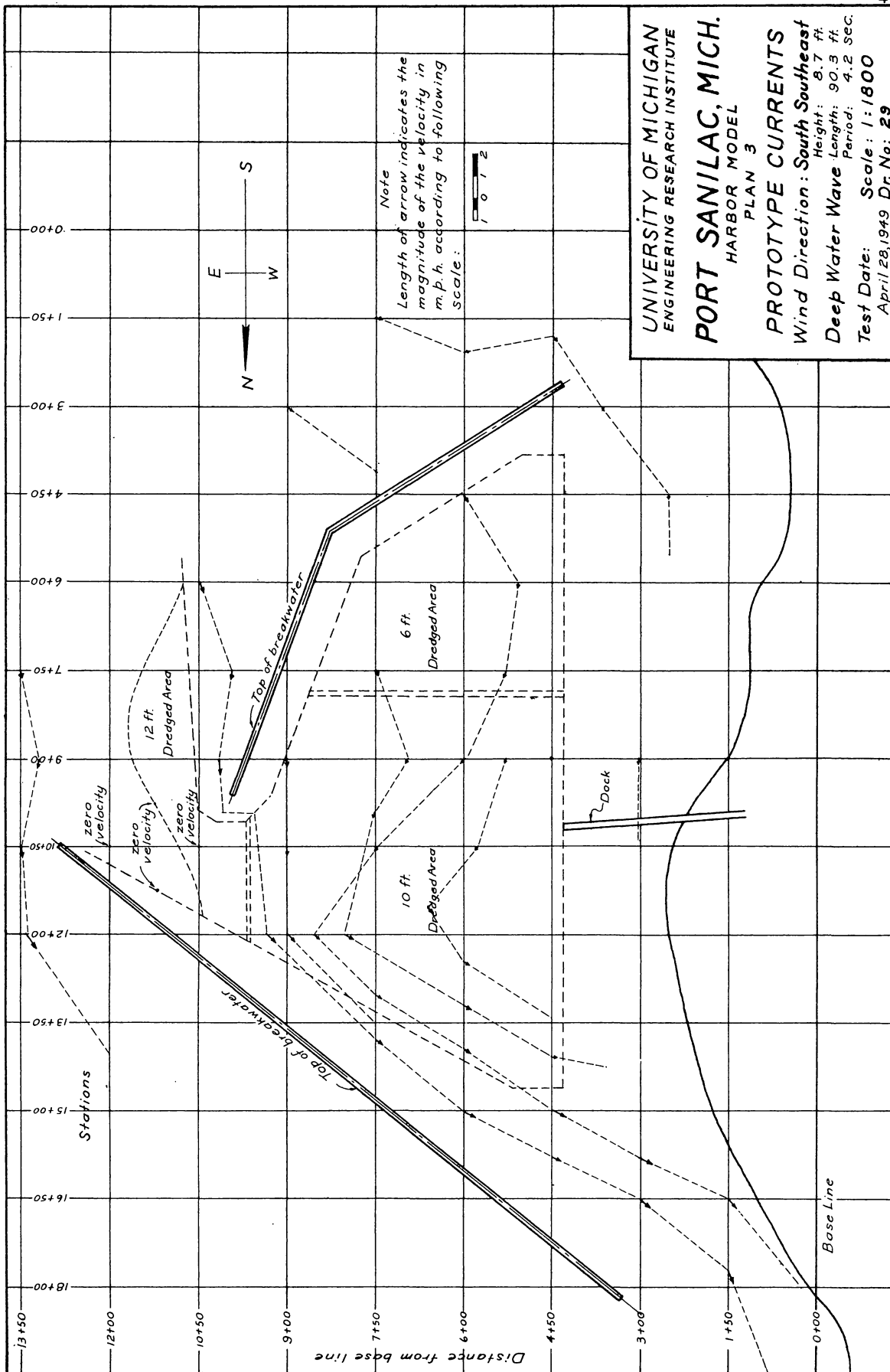
Drawing 26



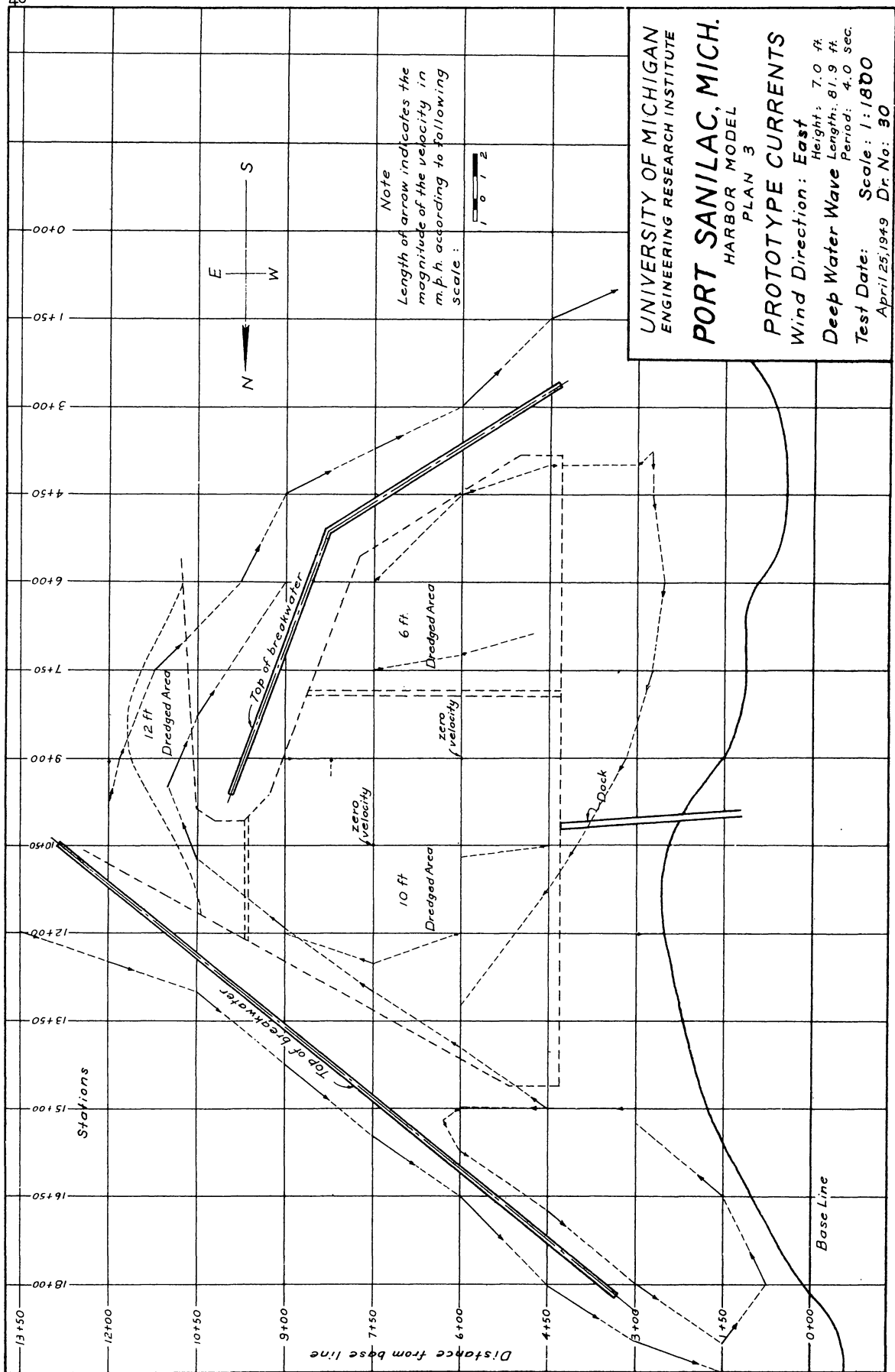
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**PORT SANILAC, MICH.**  
 HARBOR MODEL  
 PLAN 3  
**PROTOTYPE WAVE HEIGHT**  
 Wind Direction: North Northeast  
 Height: 13.0 ft  
 Deep Water Wave Length: 190.5 ft  
 Period: 6.1 Sec.  
 Test Date: Scale: 1:1800  
 April 21, 1949 Dr. No: 27



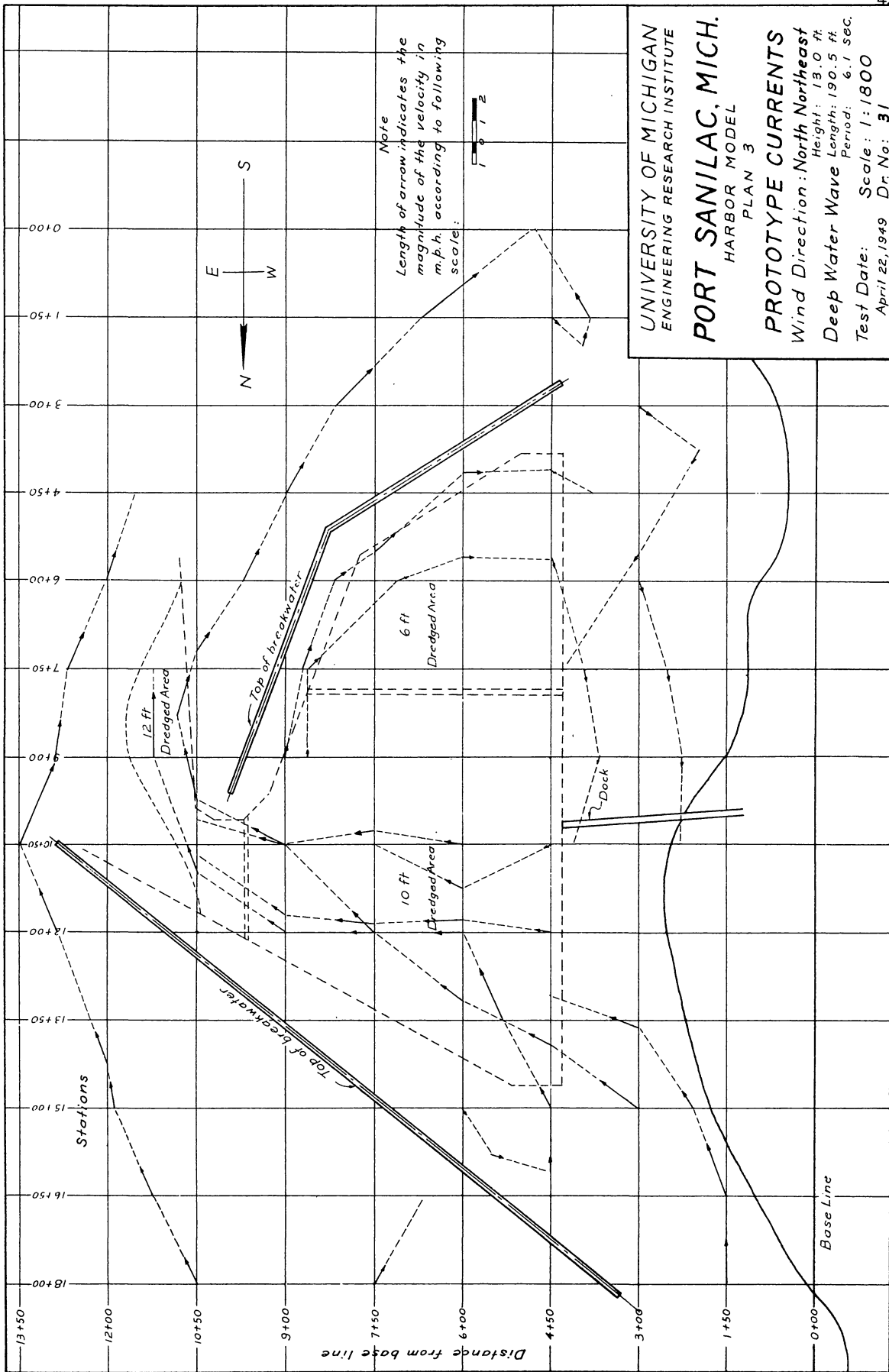
Drawing 28

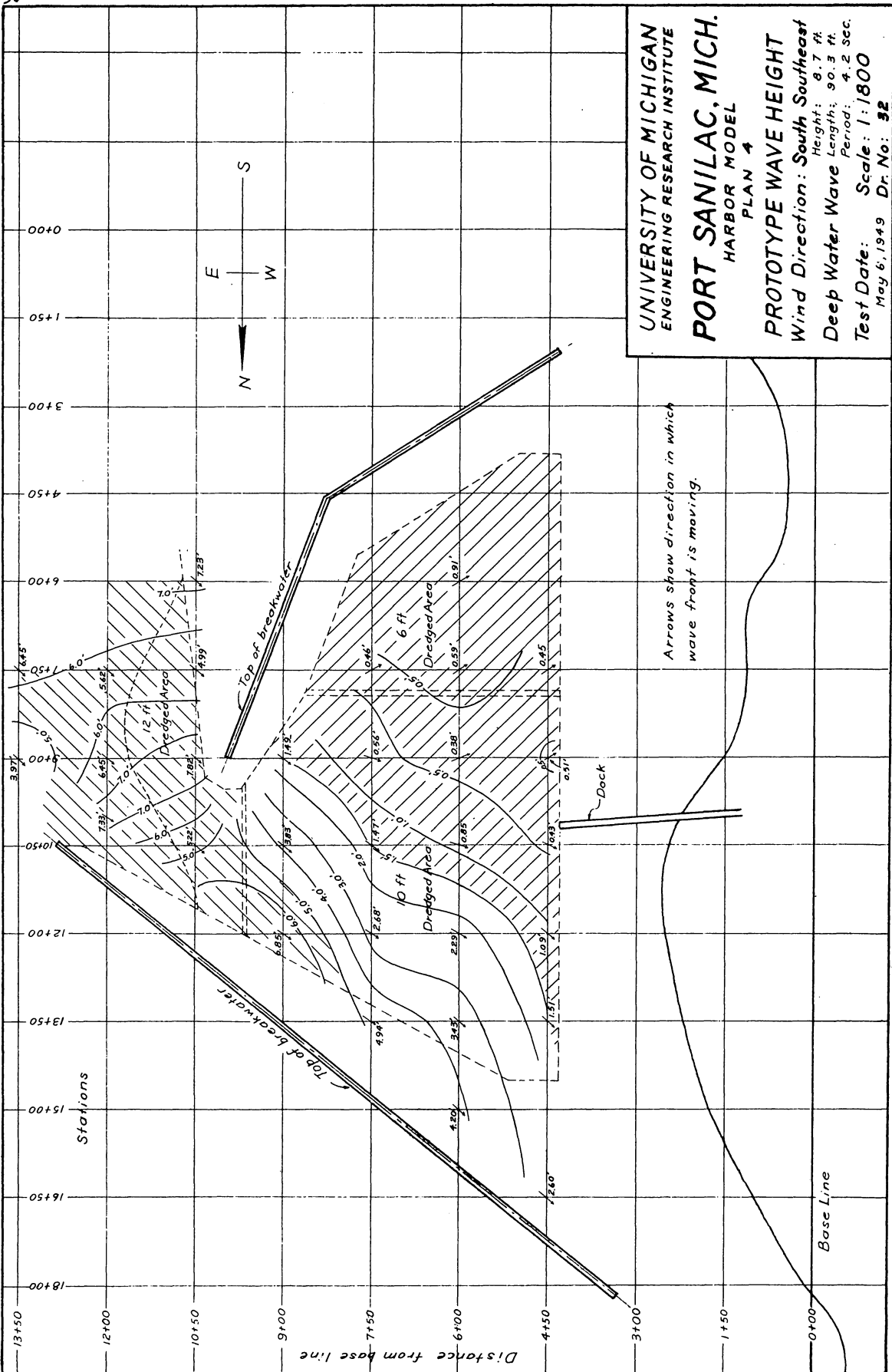


Drawing 29



Drawing 30





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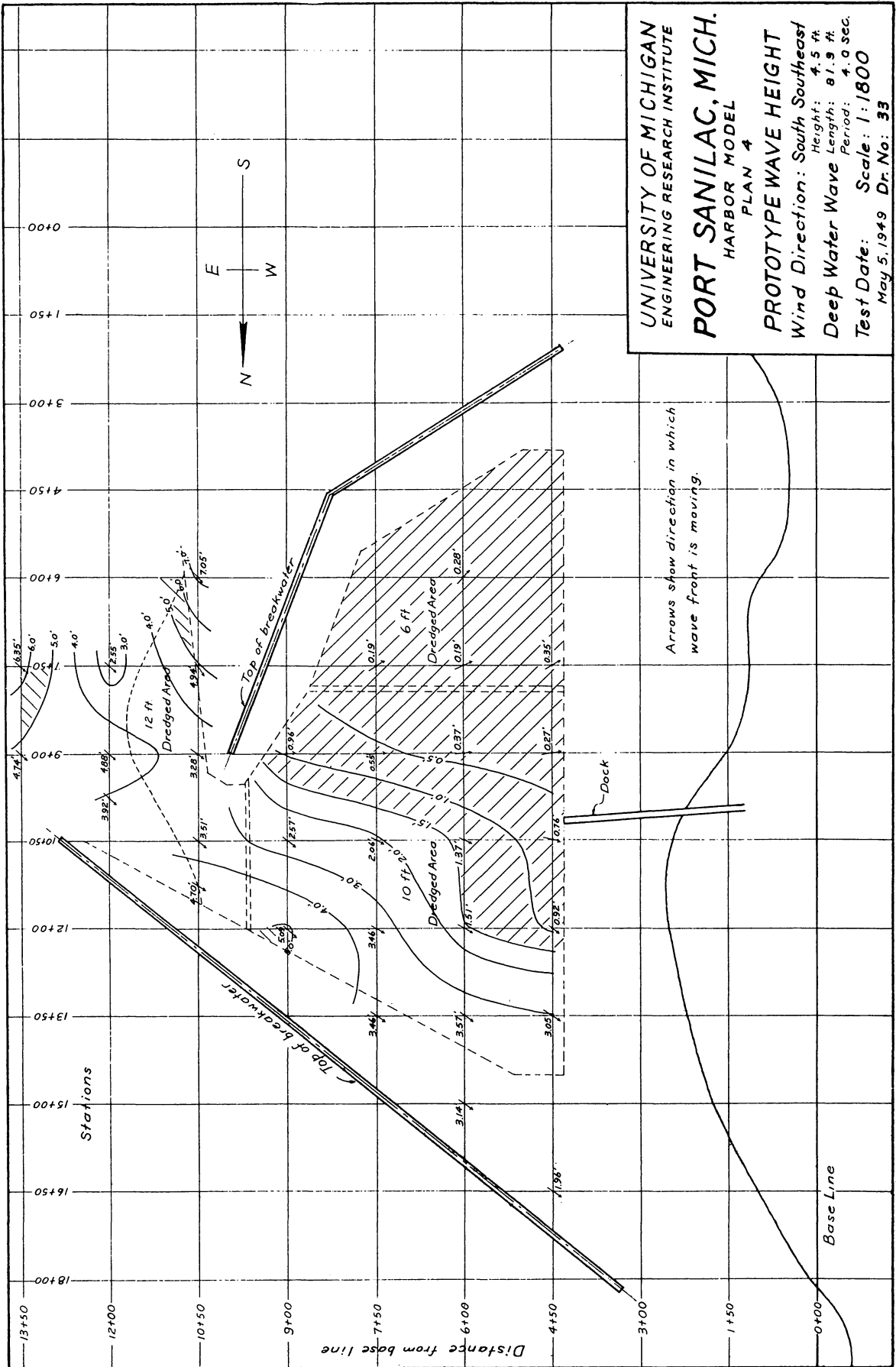
**PORT SANILAC, MICH.**  
 HARBOR MODEL  
 PLAN 4

**PROTOTYPE WAVE HEIGHT**  
 Wind Direction: South Southeast  
 Height: 8.7 ft.  
 Deep Water Wave Length: 90.3 ft.  
 Period: 4.2 sec.

Test Date: May 6, 1949  
 Scale: 1:1800  
 Dr. No: 32

Drawing 32





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**PORT SANILAC, MICH.**

HARBOR MODEL  
PLAN 4

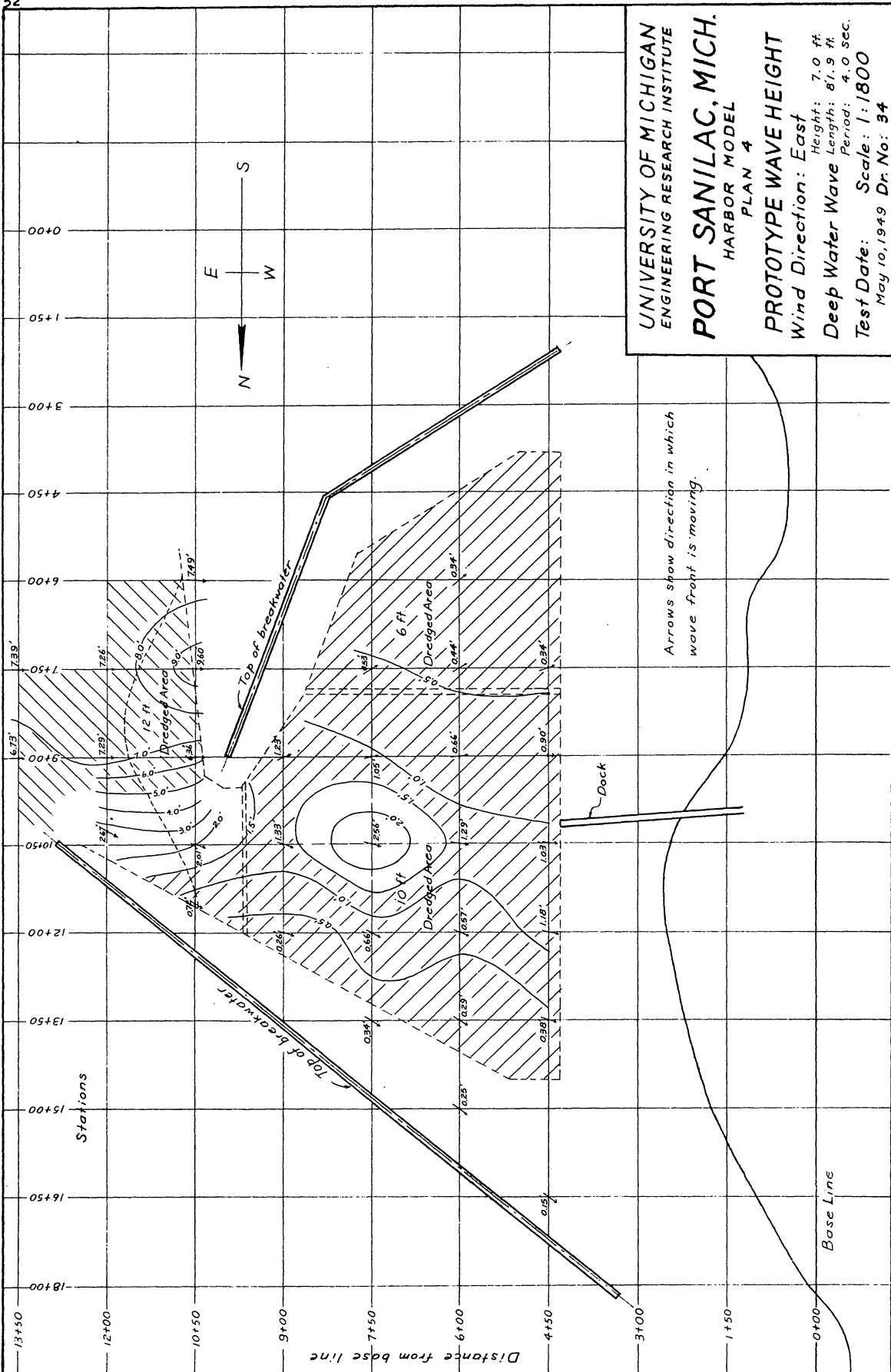
**PROTOTYPE WAVE HEIGHT**

Wind Direction: South Southeast  
Height: 4.5 ft

Deep Water Wave Length: 81.9 ft

Period: 4.0 sec

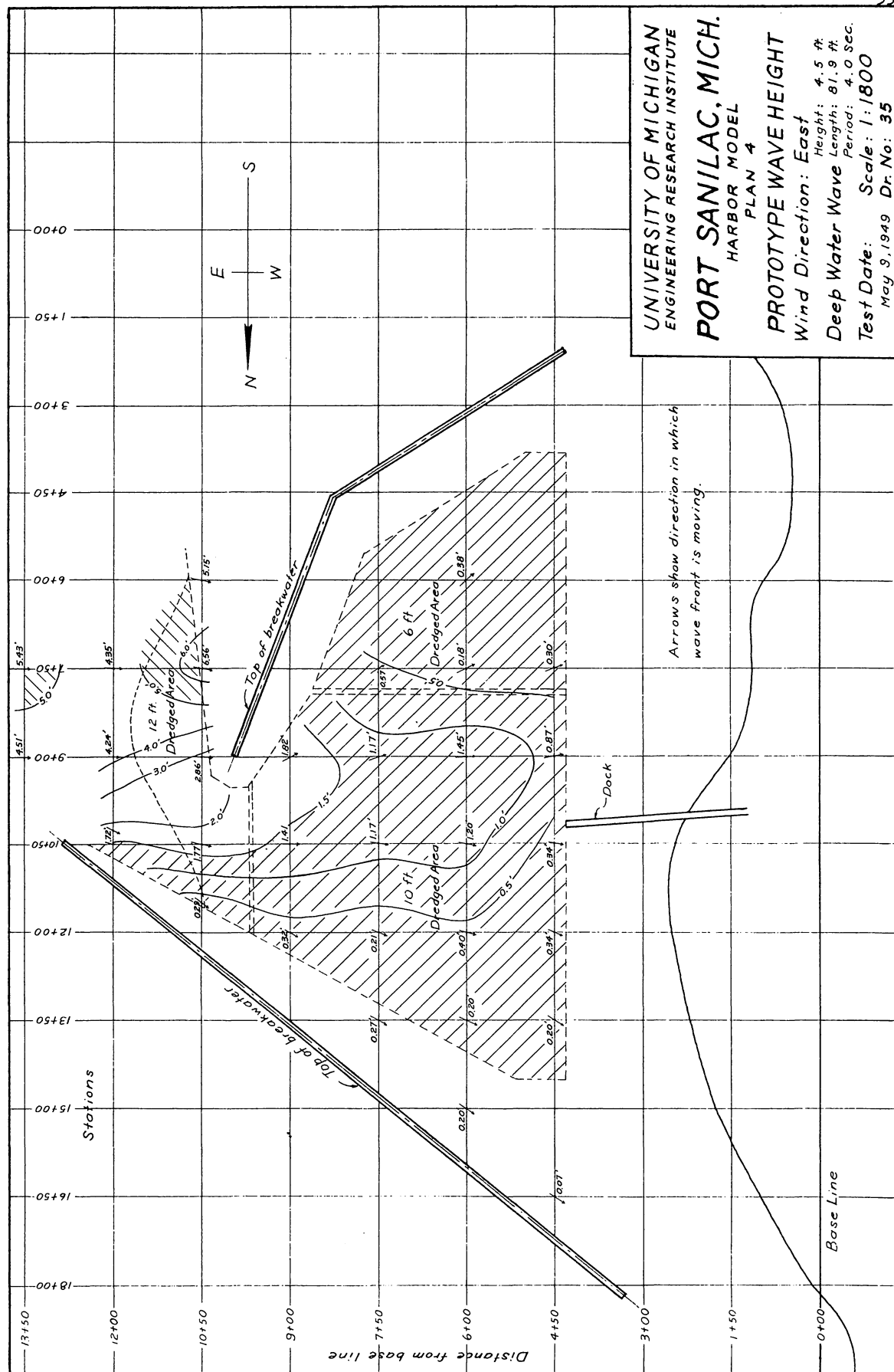
Test Date: May 5, 1949  
Scale: 1:1800  
Dr. No: 33

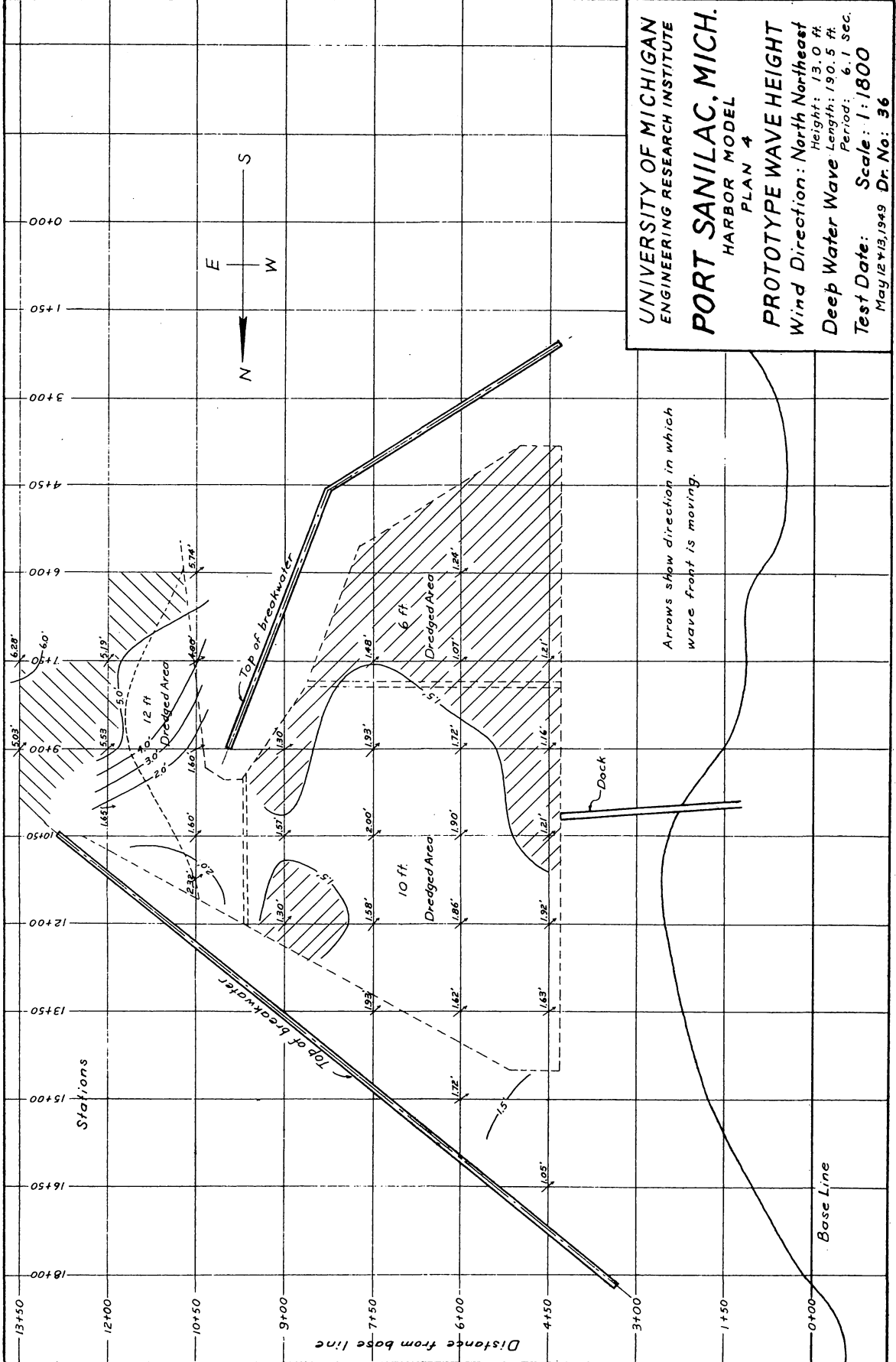


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**PORT SANILAC, MICH.**  
 HARBOR MODEL  
 PLAN 4  
**PROTOTYPE WAVE HEIGHT**  
 Wind Direction: East  
 Deep Water Wave Height: 7.0 ft  
 Length: 81.9 ft  
 Period: 4.0 sec.  
 Test Date: May 10, 1949  
 Scale: 1:1800  
 Dr. No. 34

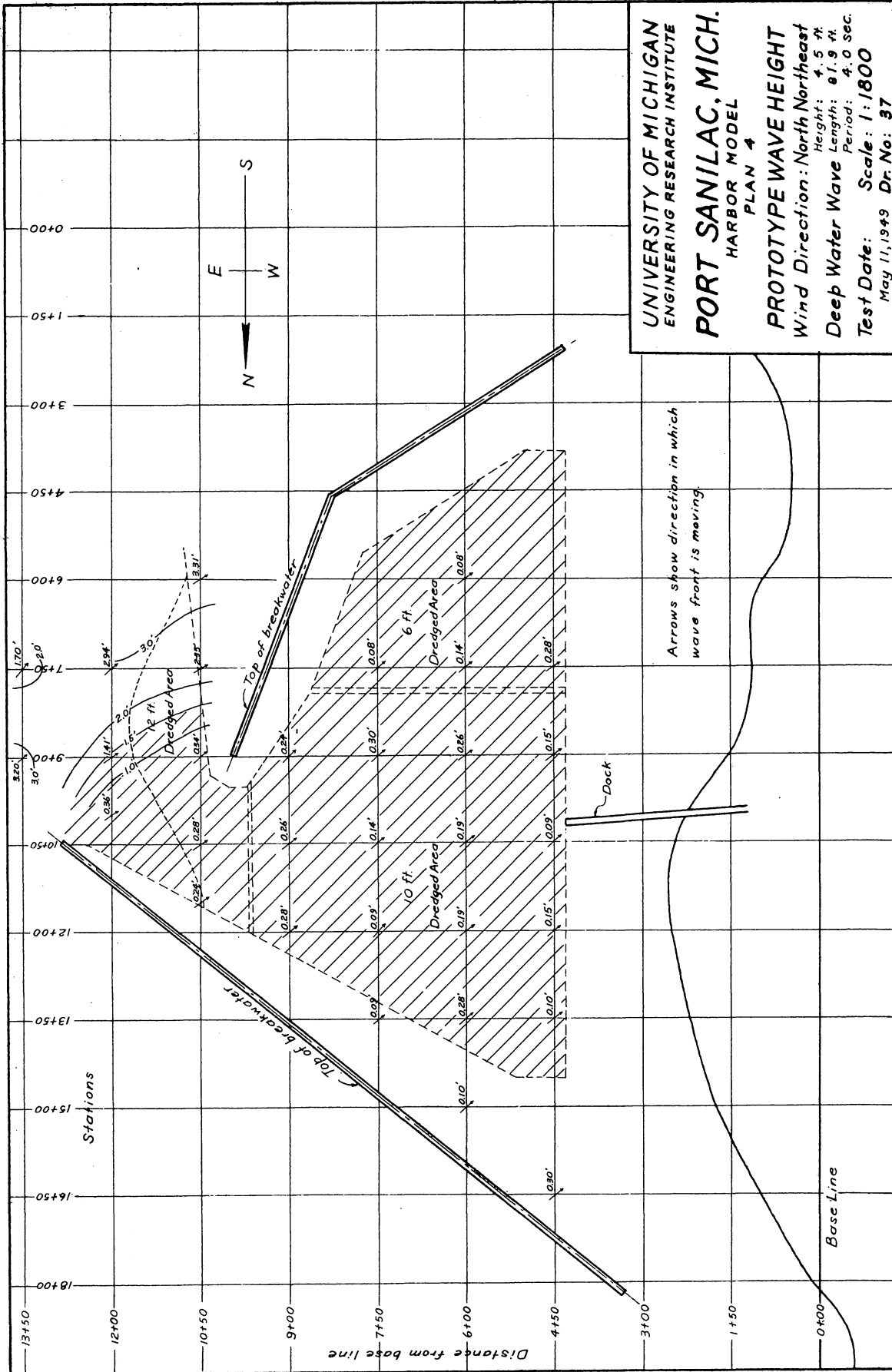
Drawing 34

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**PORT SANILAC, MICH.**  
 HARBOR MODEL  
 PLAN 4  
**PROTOTYPE WAVE HEIGHT**  
 Wind Direction: East  
 Height: 4.5 ft.  
 Length: 81.9 ft.  
 Deep Water Wave Period: 4.0 Sec.  
 Test Date: May 9, 1949  
 Scale: 1:1800  
 Dr. No.: 35



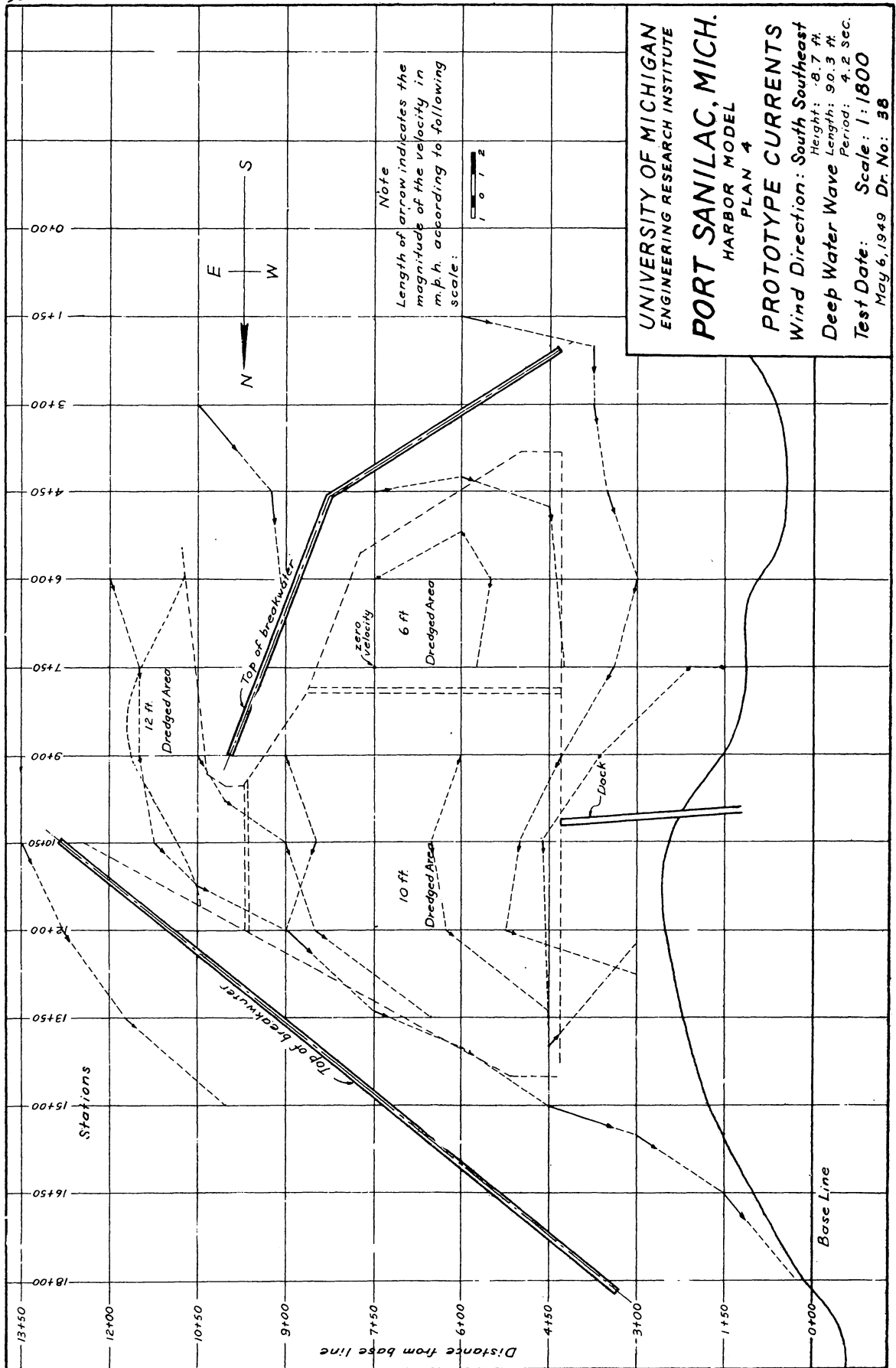


Drawing 36

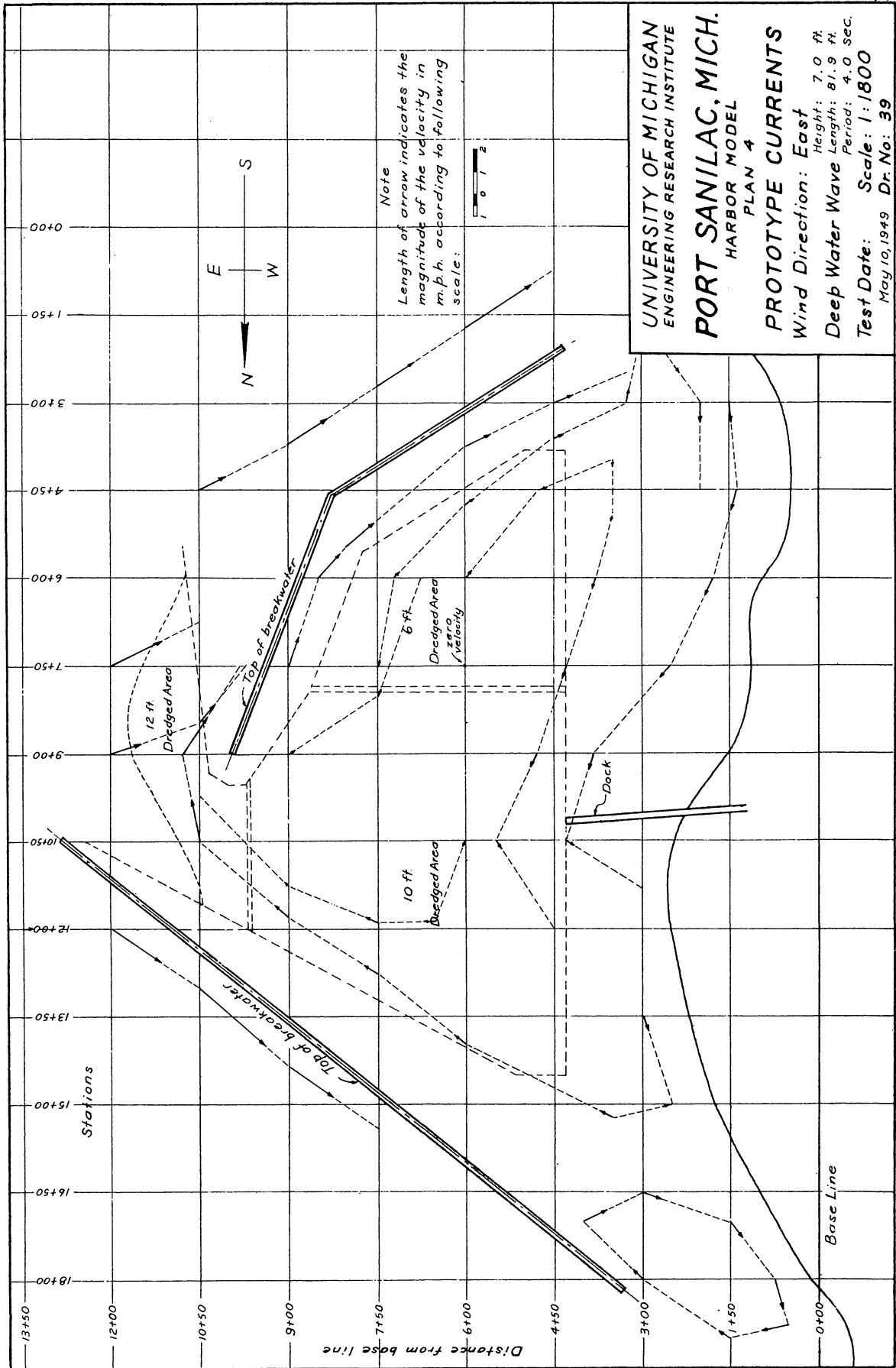


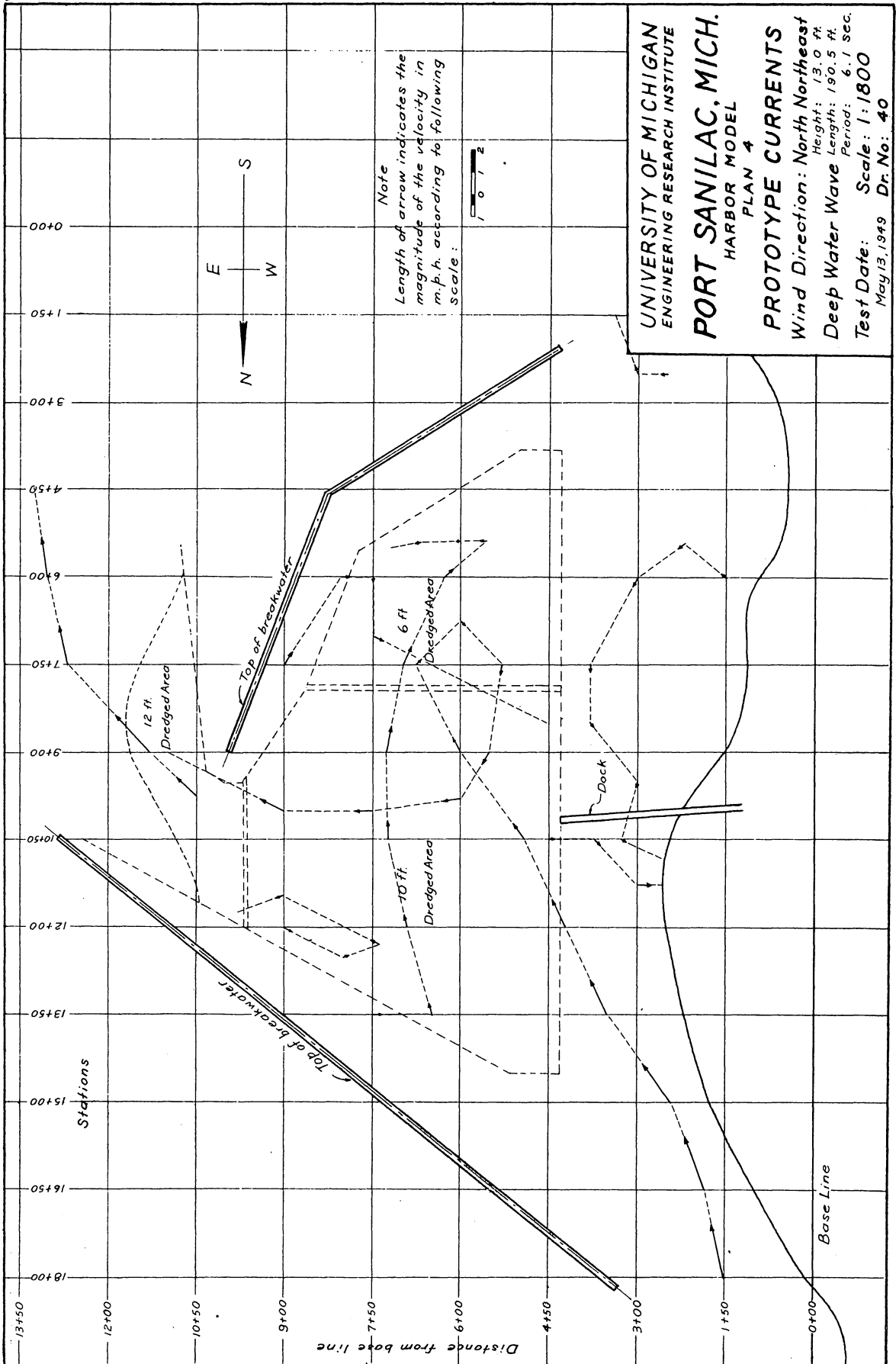
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**PORT SANILAC, MICH.**  
 HARBOR MODEL  
 PLAN 4  
**PROTOTYPE WAVE HEIGHT**  
 Wind Direction: North Northeast  
 Height: 4.5 ft  
 Length: 81.9 ft  
 Deep Water Wave Period: 4.0 sec.  
 Test Date: May 11, 1949  
 Scale: 1:1800  
 Dr. No: 37

Arrows show direction in which wave front is moving.



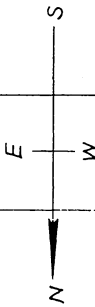
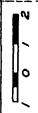
Drawing 38





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**PORT SANILAC, MICH.**  
 HARBOR MODEL  
 PLAN 4  
**PROTOTYPE CURRENTS**  
 Wind Direction: North Northeast  
 Height: 13.0 ft  
 Deep Water Wave Length: 190.5 ft  
 Period: 6.1 sec.  
 Test Date: May 13, 1949  
 Scale: 1:1800  
 Dr. No: 40

Note  
 Length of arrow indicates the  
 magnitude of the velocity in  
 m.p.h. according to following  
 scale:



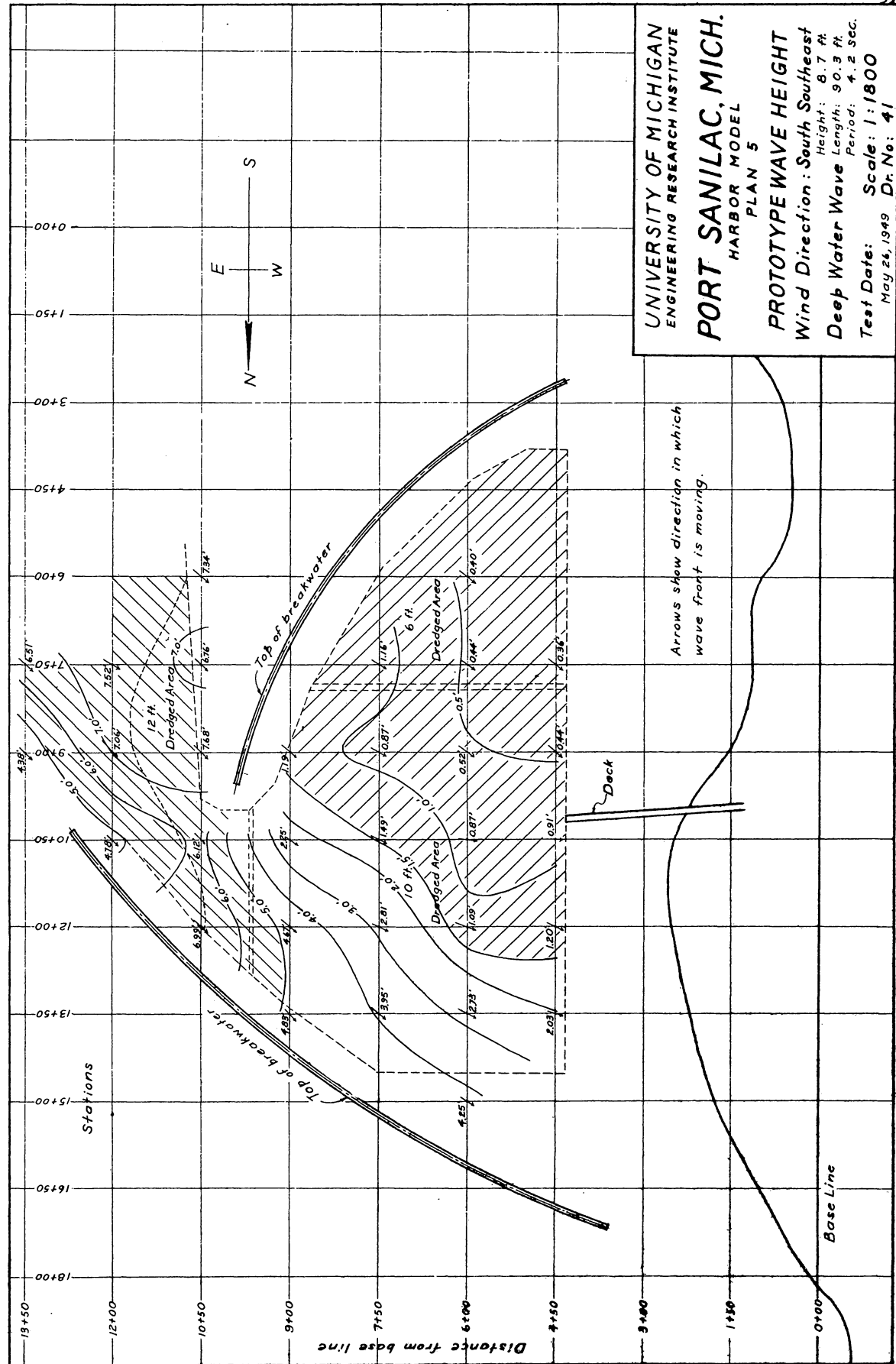
Drawing 40

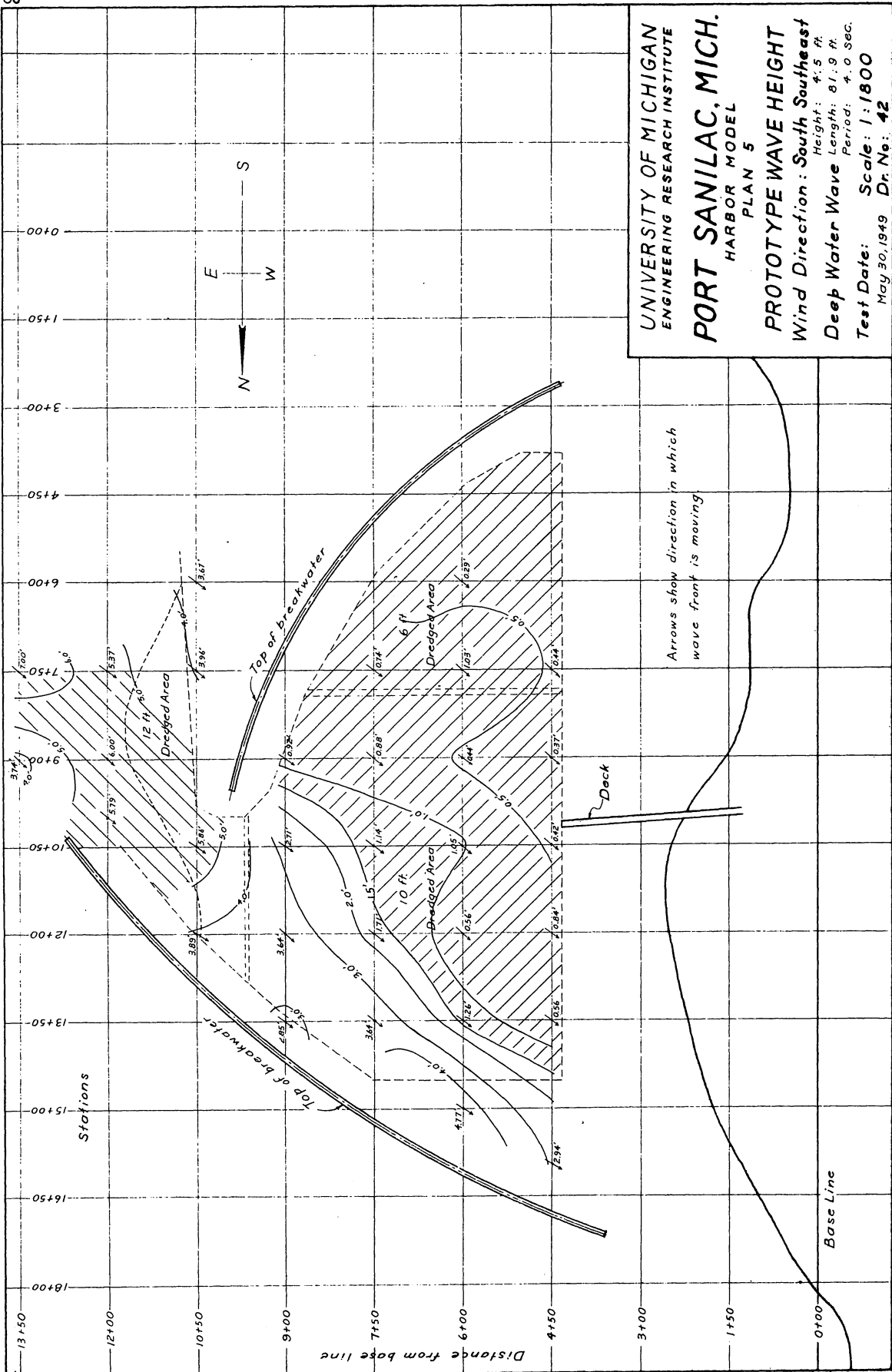


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# PORT SANILAC, MICH. HARBOR MODEL PLAN 5

**PROTOTYPE WAVE HEIGHT**  
Wind Direction: South Southeast  
Height: 8.7 ft.  
Deep Water Wave Length: 90.3 ft.  
Period: 4.2 sec.  
Test Date: May 26, 1949  
Scale: 1:1800  
Dr. No.: 41





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**PORT SANILAC, MICH.**  
HARBOR MODEL  
PLAN 5

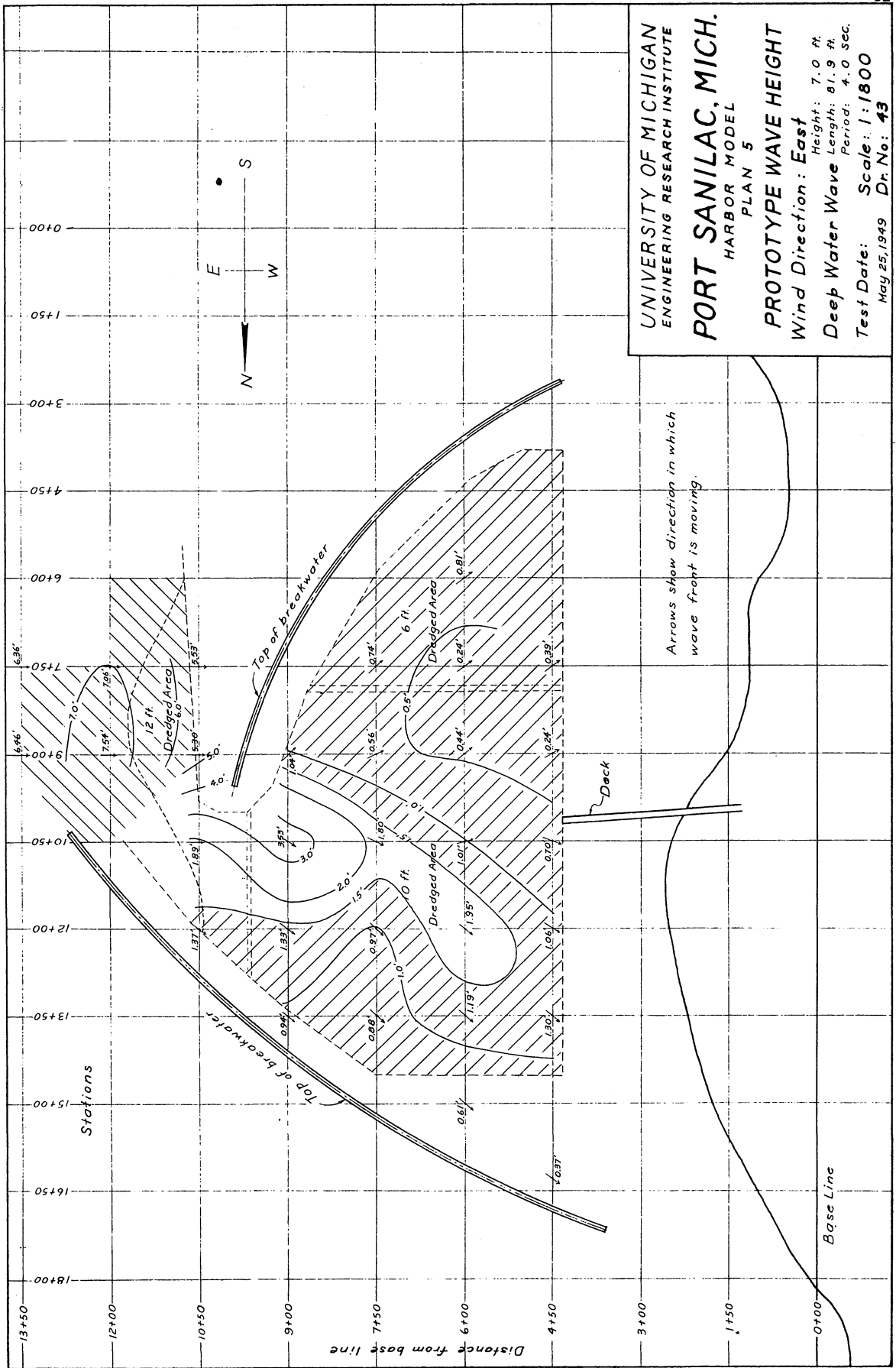
**PROTOTYPE WAVE HEIGHT**

Wind Direction: South Southeast  
Height: 41.5 ft

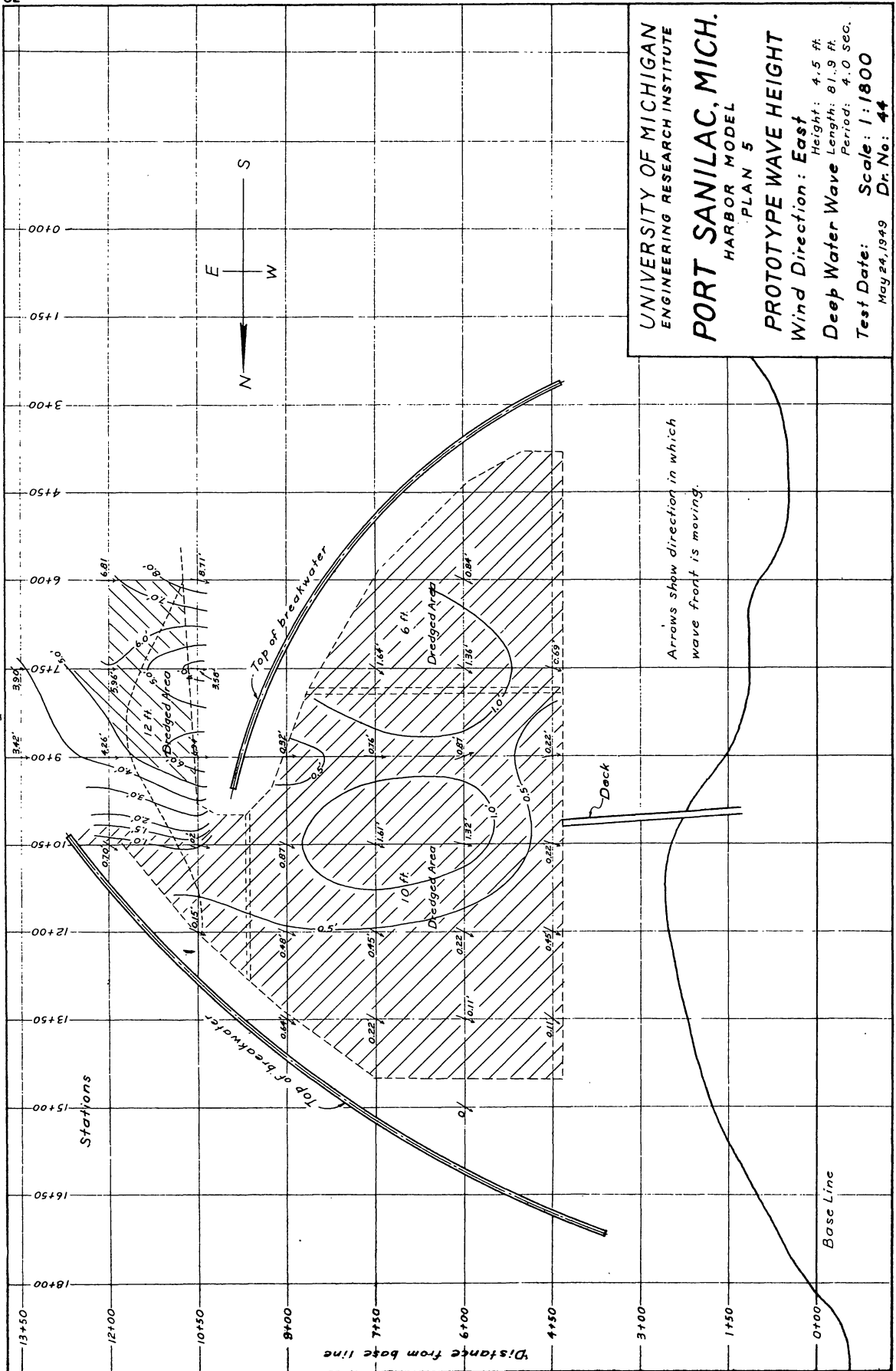
Deep Water Wave Length: 81.9 ft  
Period: 4.0 Sec.

Test Date: May 30, 1949  
Scale: 1:1800  
Dr. No.: 42

Drawing 42



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**PORT SANILAC, MICH.**  
 HARBOR MODEL  
 PLAN 5  
**PROTOTYPE WAVE HEIGHT**  
 Wind Direction: East  
 Height: 7.0 ft  
 Deep Water Wave Length: 81.9 ft  
 Period: 4.0 sec.  
 Test Date: May 25, 1949  
 Scale: 1:1800  
 Dr. No.: 43



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**PORT SANILAC, MICH.**  
HARBOR MODEL  
PLAN 5

**PROTOTYPE WAVE HEIGHT**

Wind Direction: East

Height: 4.5 ft.

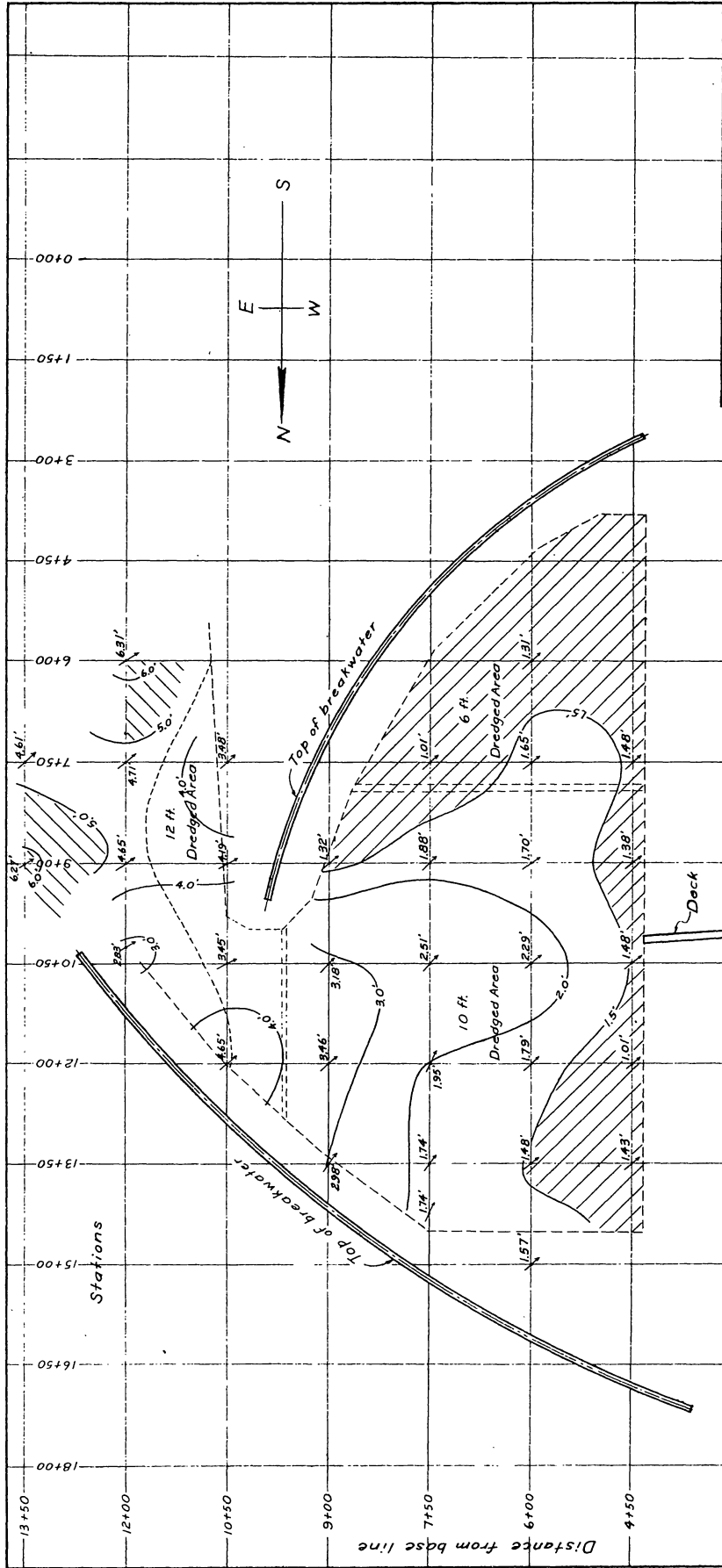
Deep Water Wave Length: 81.9 ft.

Period: 4.0 sec.

Test Date: May 24, 1949

Scale: 1:1800

Dr. No.: 44



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**PORT SANILAC, MICH.**  
HARBOR MODEL  
PLAN 5

**PROTOTYPE WAVE HEIGHT**

Wind Direction: North Northeast  
Deep Water Wave Height: 13.0 ft  
Length: 190.5 ft  
Period: 6.1 Sec.

Test Date: May 1920, 1949  
Scale: 1:1800  
Dr. No. 45

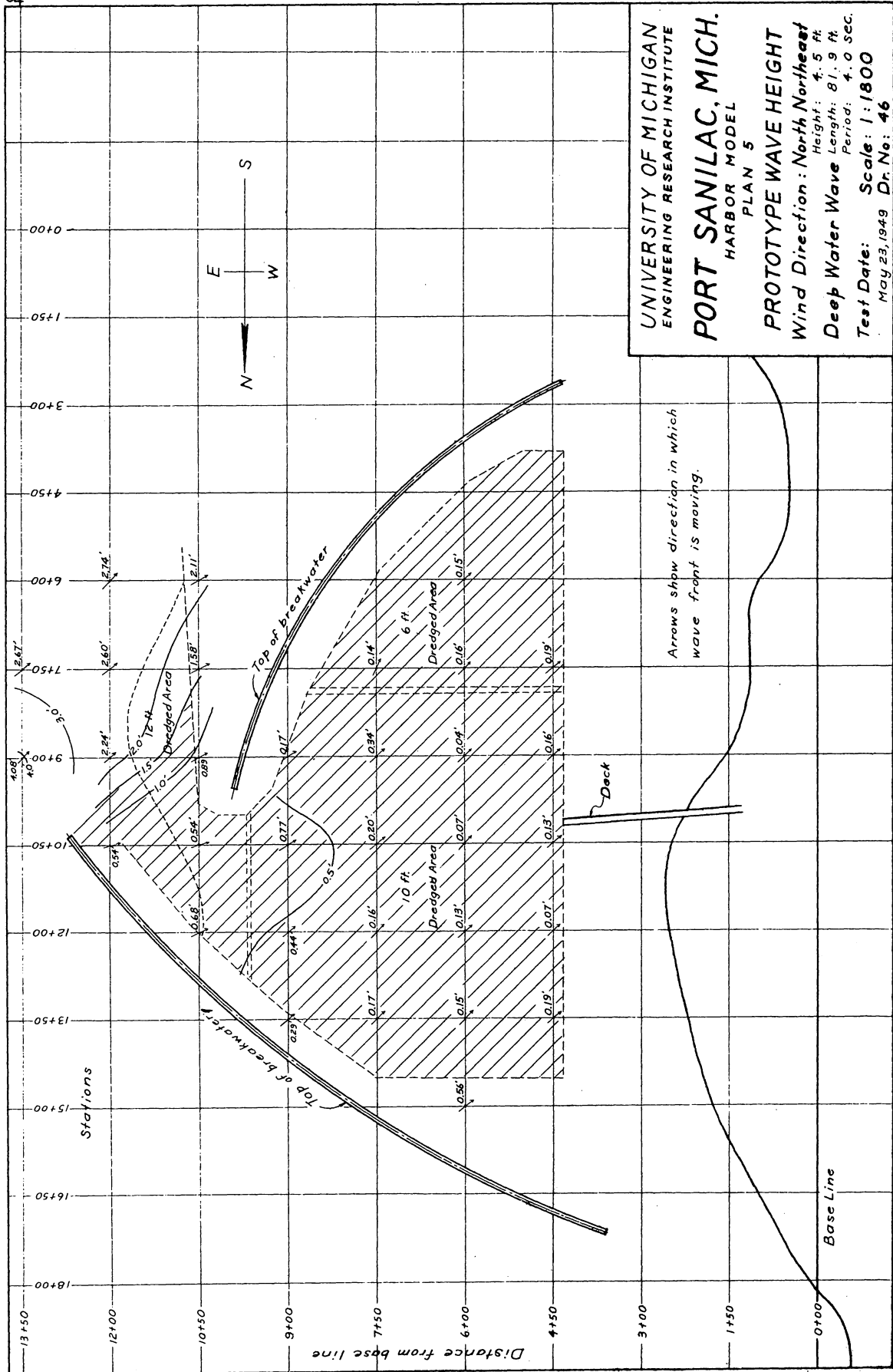
Arrows show direction in which wave front is moving.

Base Line

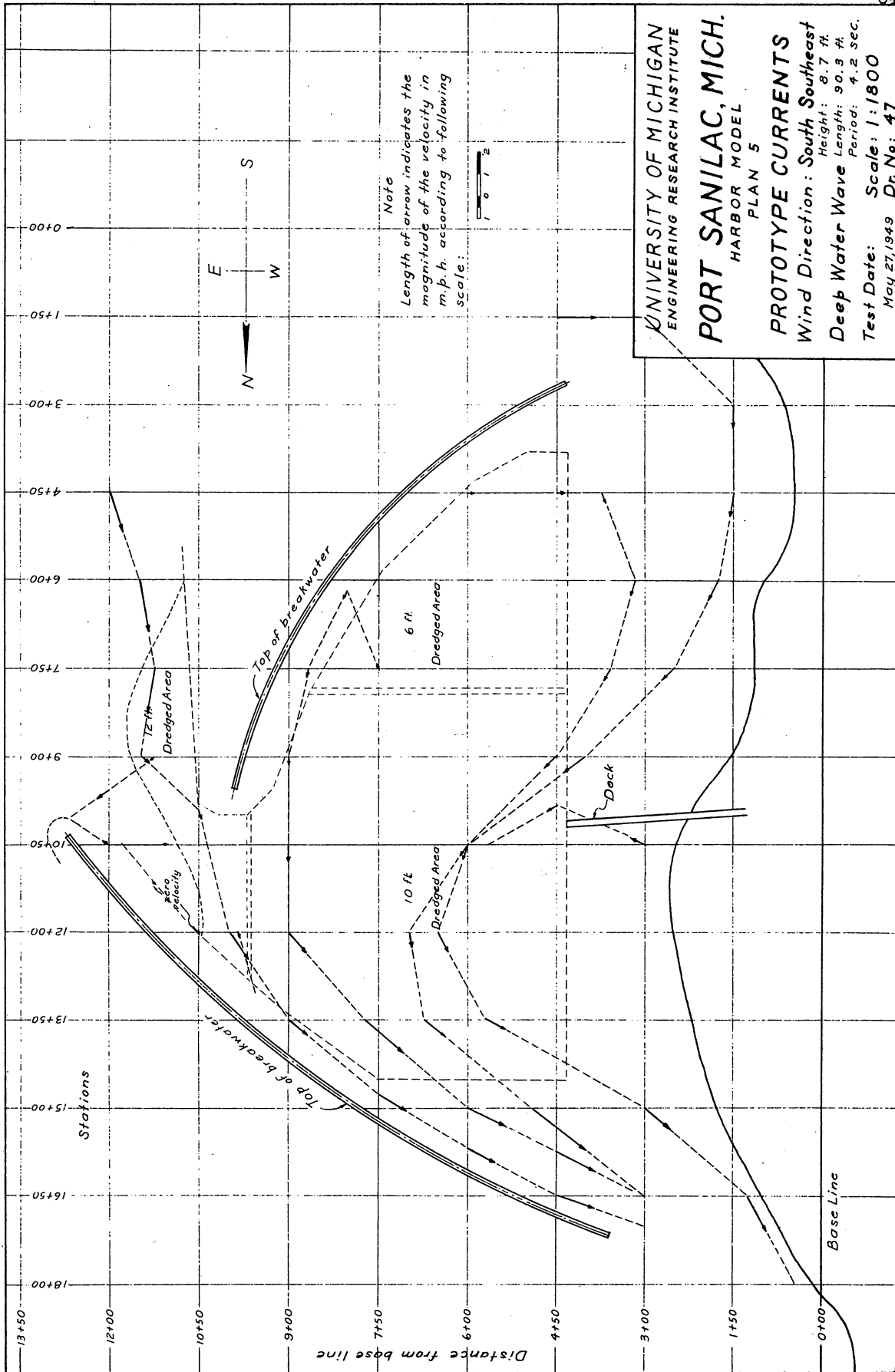
Deck

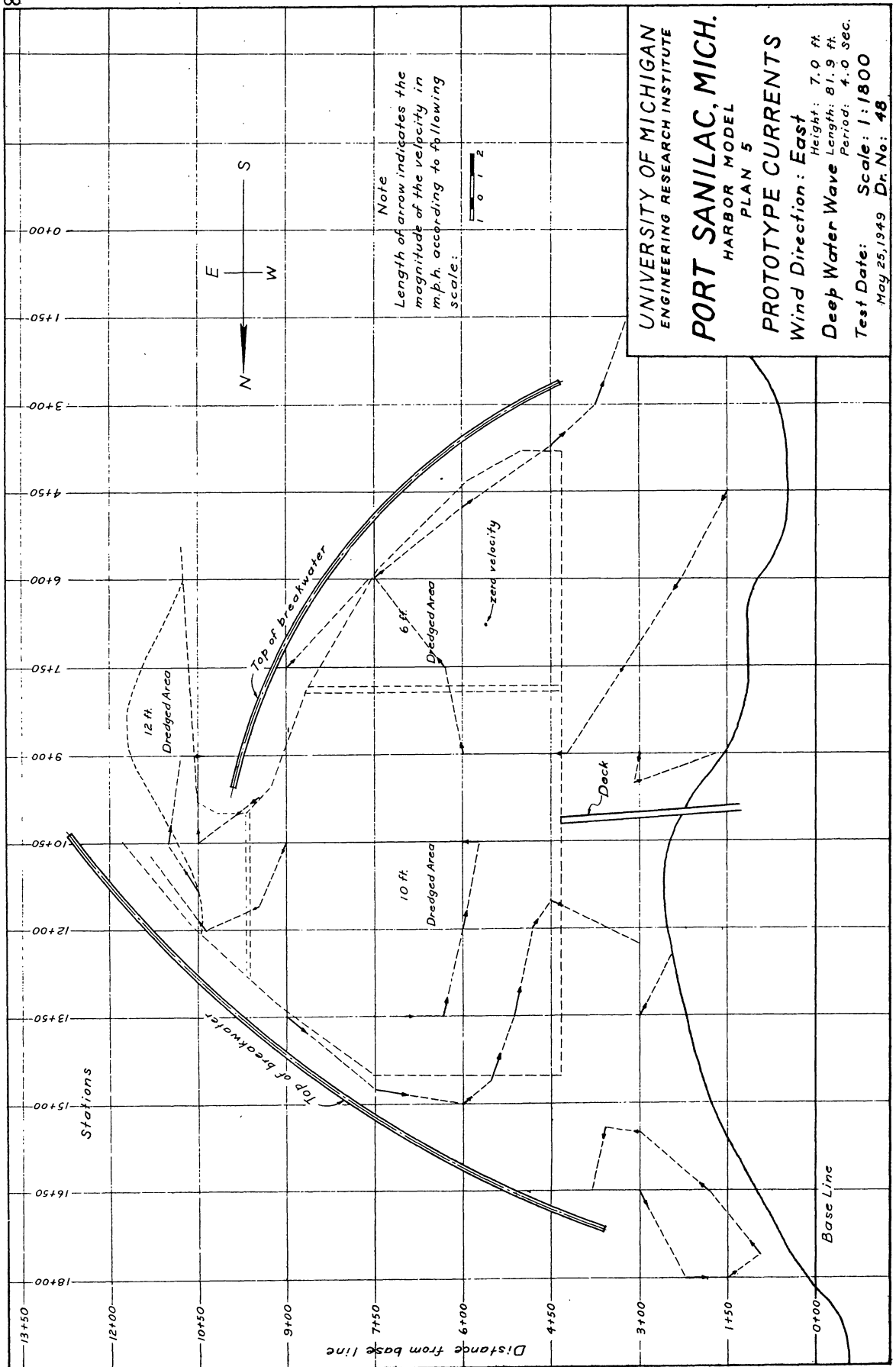
Stations

Distance from base line



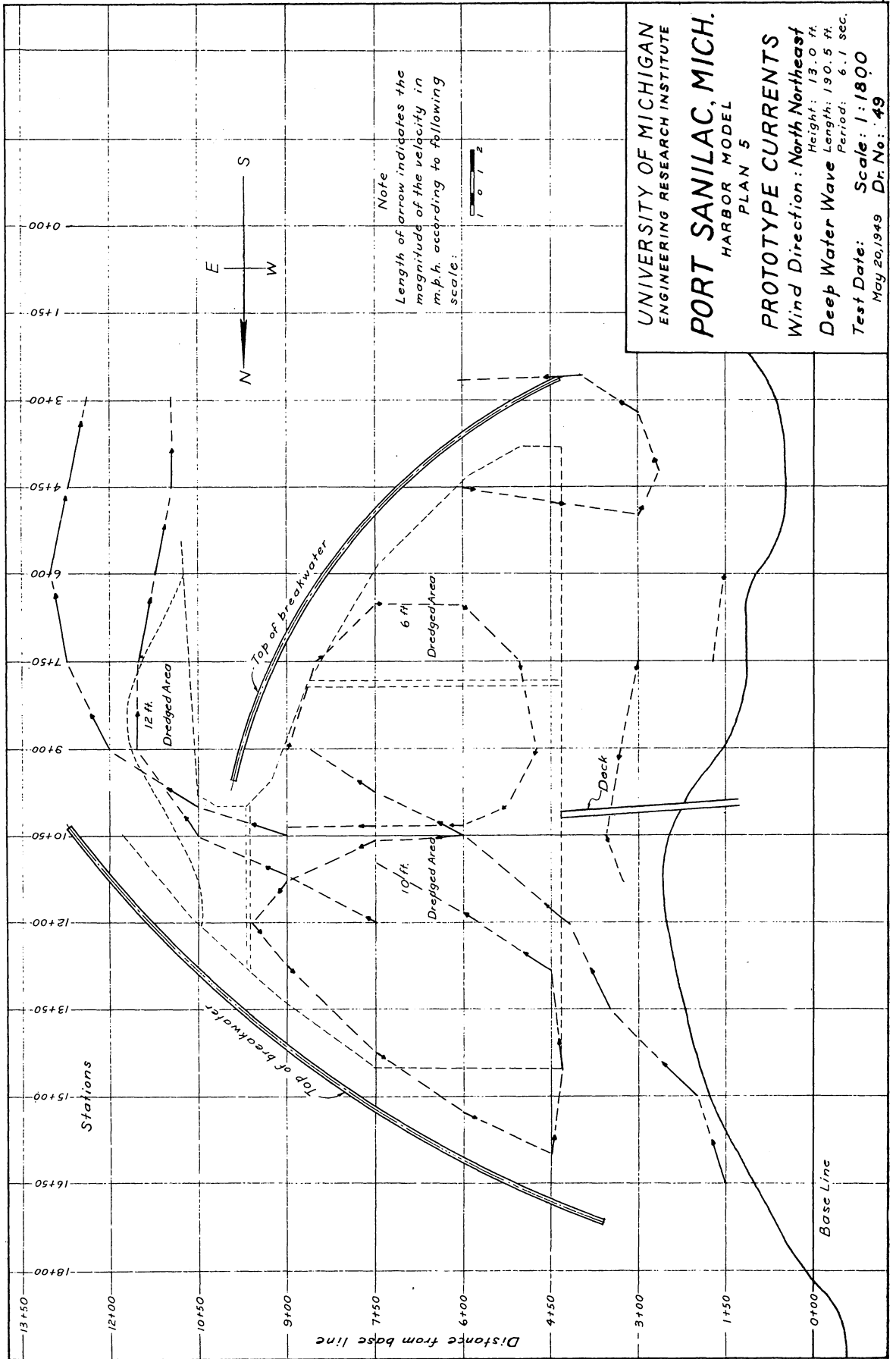
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**PORT SANILAC, MICH.**  
 HARBOR MODEL  
 PLAN 5  
**PROTOTYPE WAVE HEIGHT**  
 Wind Direction: North-Northeast  
 Height: 4.5 ft  
 Length: 81.9 ft  
 Deep Water Wave Period: 4.0 sec.  
 Test Date: Scale: 1:1800  
 May 23, 1949 Dr. No.: 46

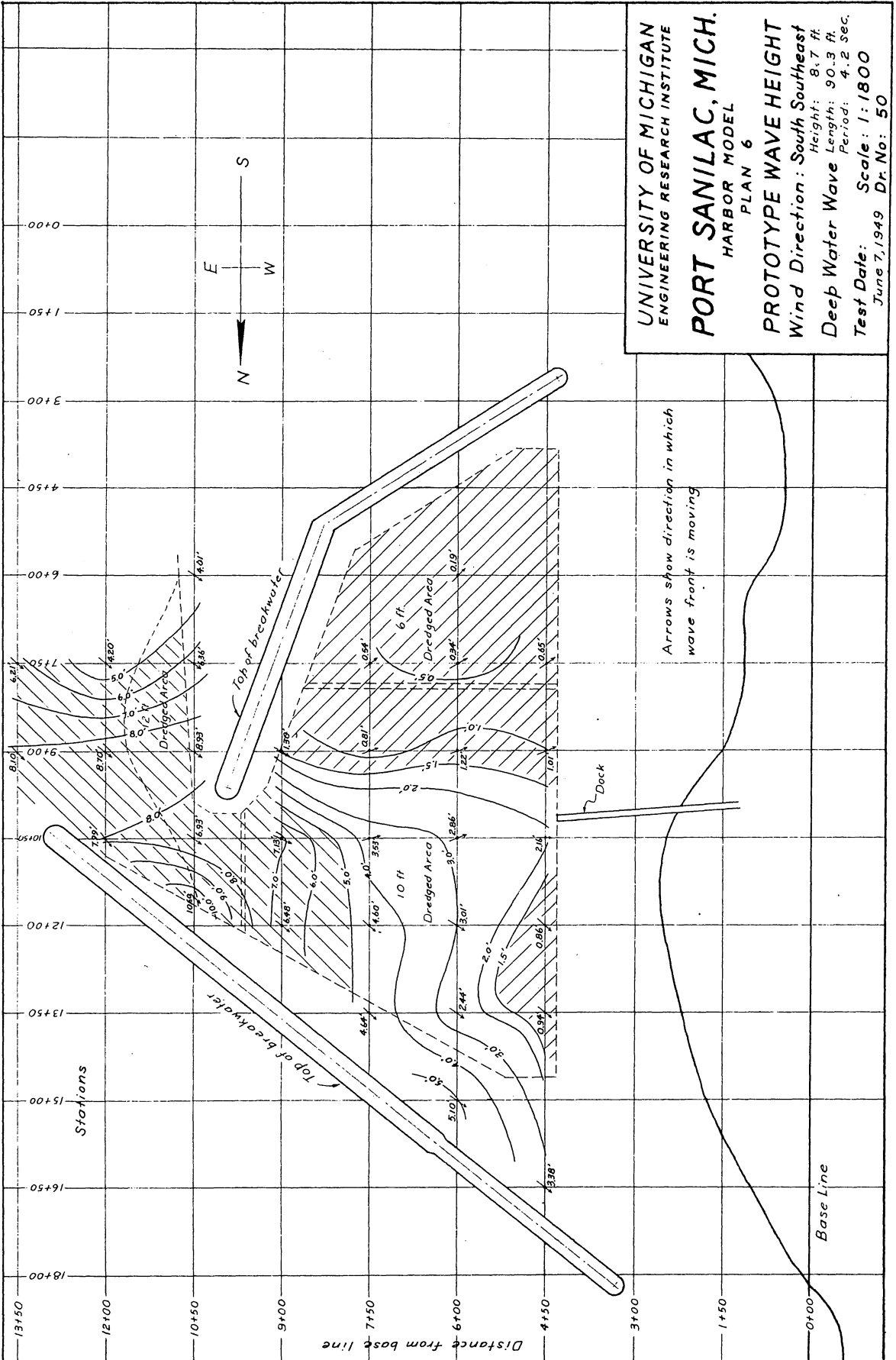




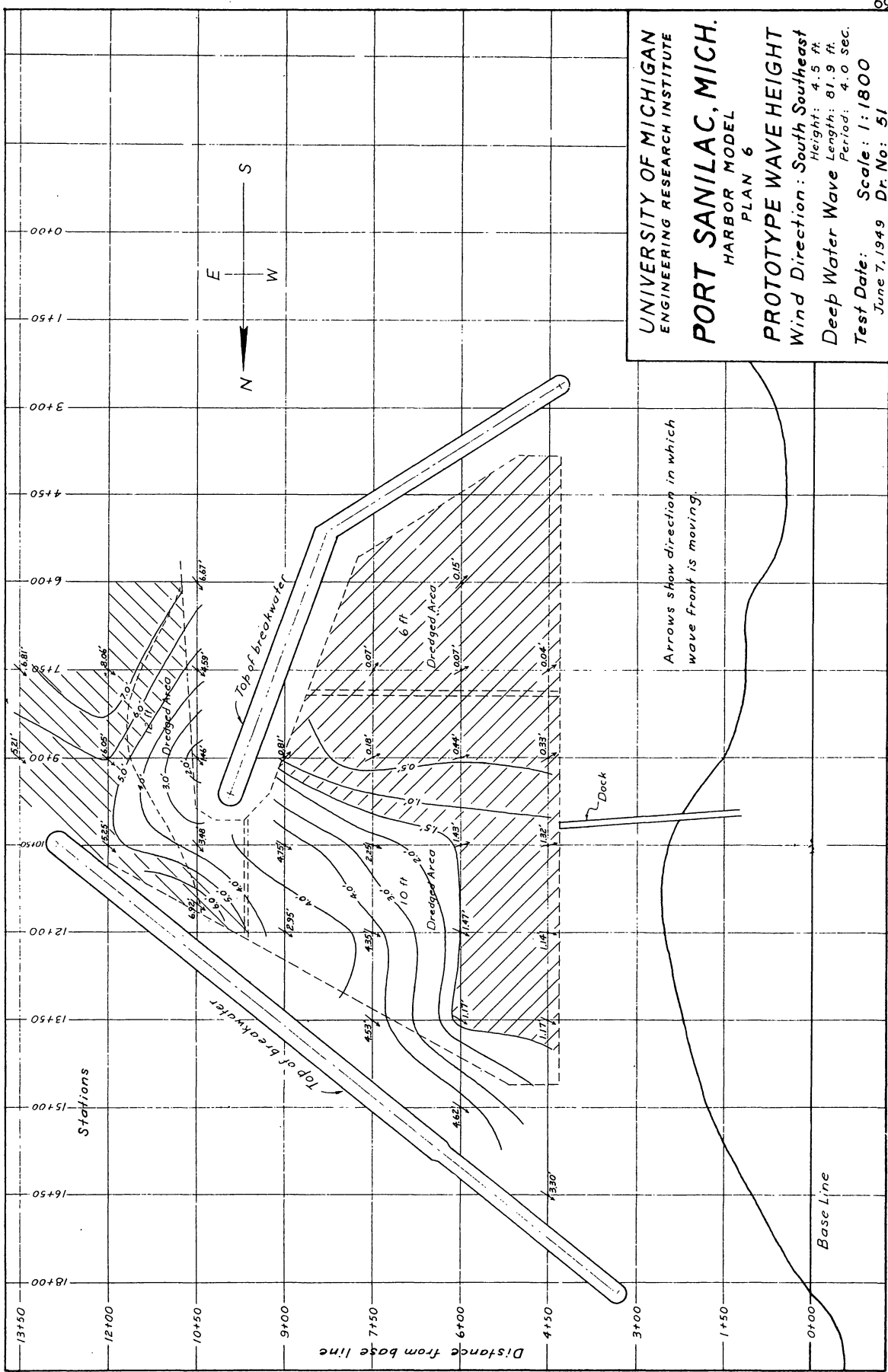
Drawing 87







Drawing 50



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**PORT SANILAC, MICH.**  
HARBOR MODEL  
PLAN 6

**PROTOTYPE WAVE HEIGHT**  
Wind Direction: South Southeast  
Height: 4.5 ft  
Deep Water Wave Length: 81.9 ft  
Period: 4.0 sec.

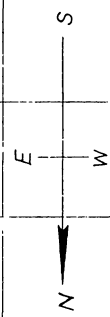
Test Date: June 7, 1949  
Scale: 1:1800  
Dr. No: 51

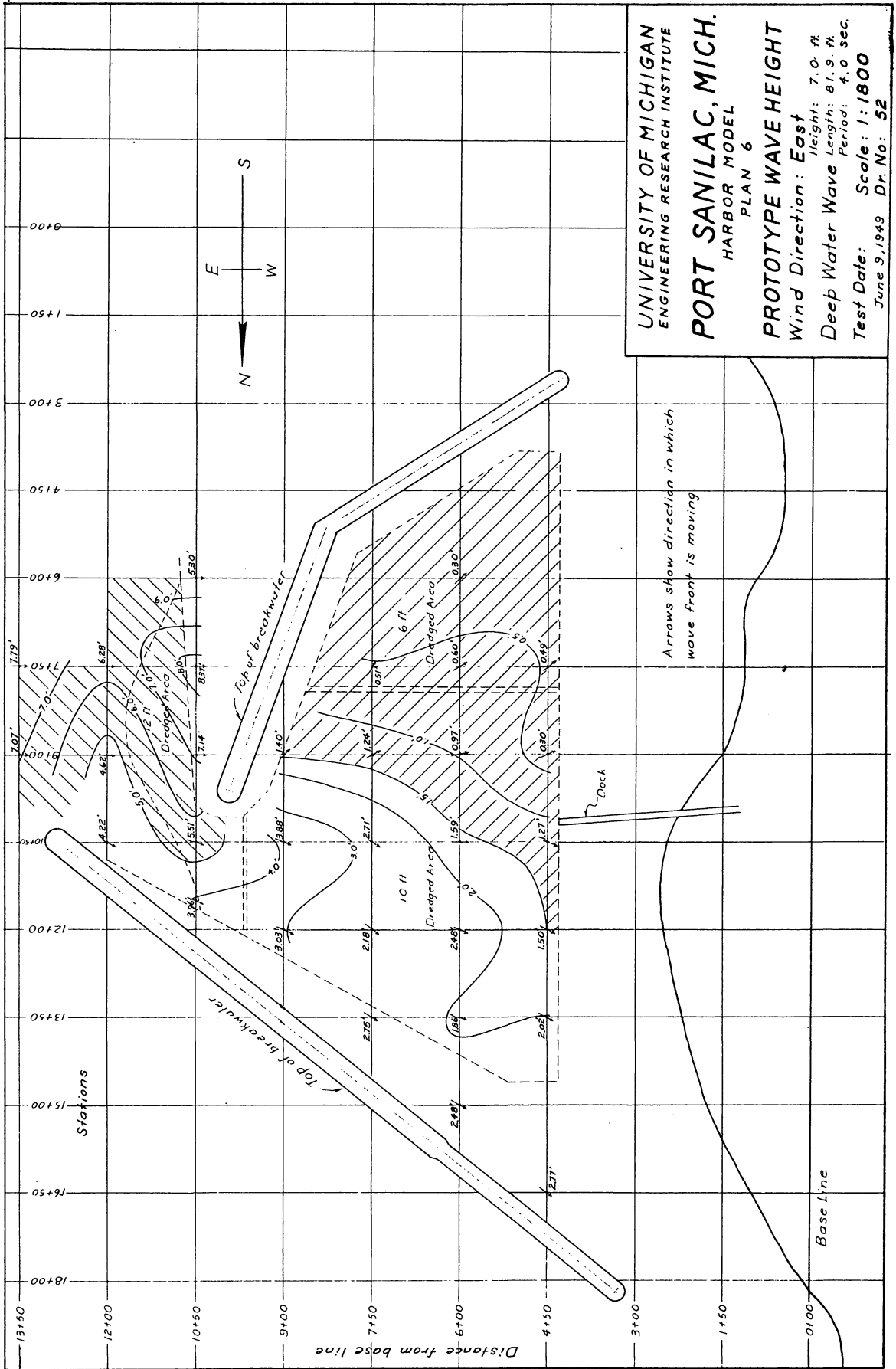
Arrows show direction in which  
wave front is moving.

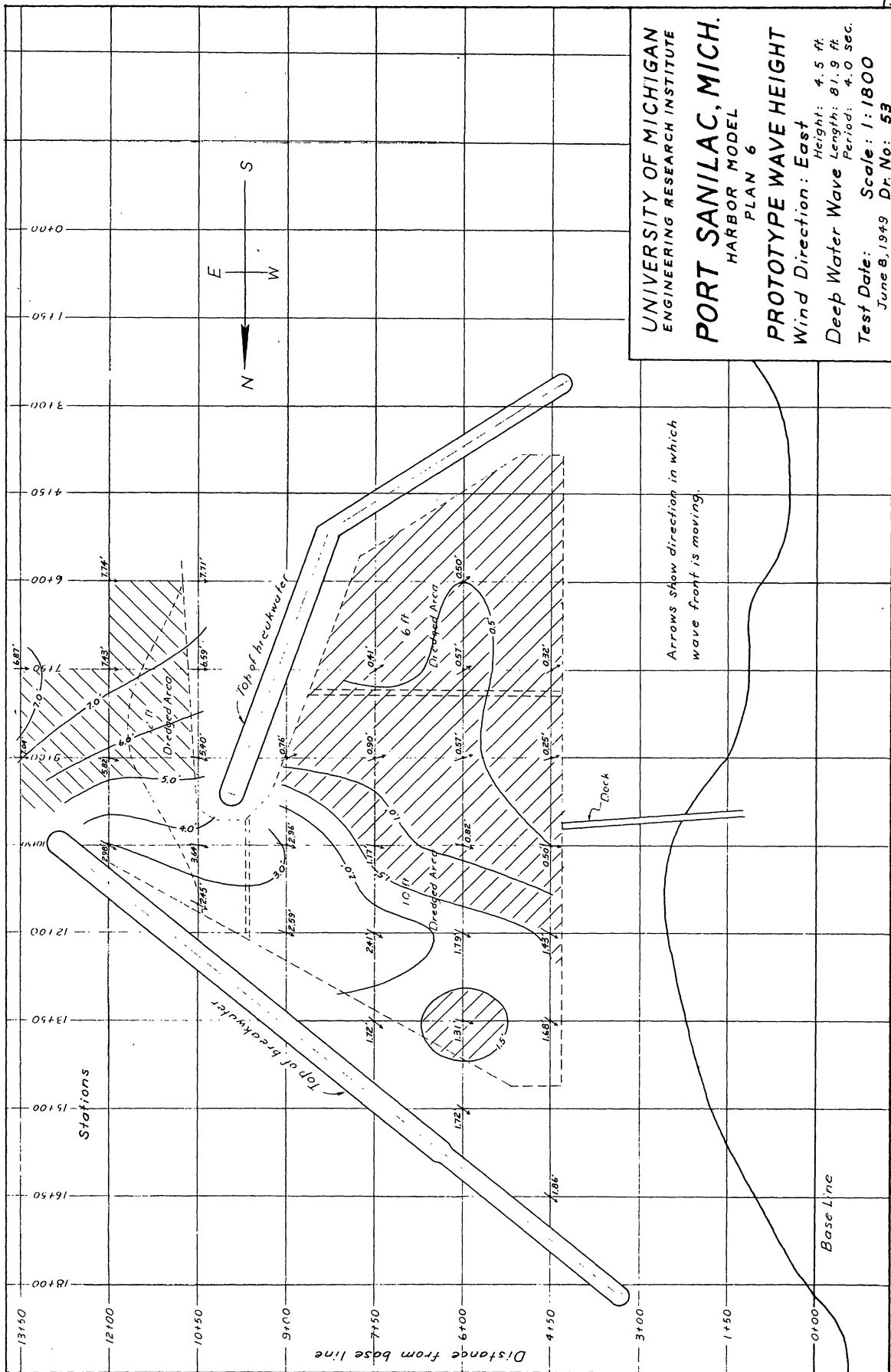
Base Line

Distance from base line

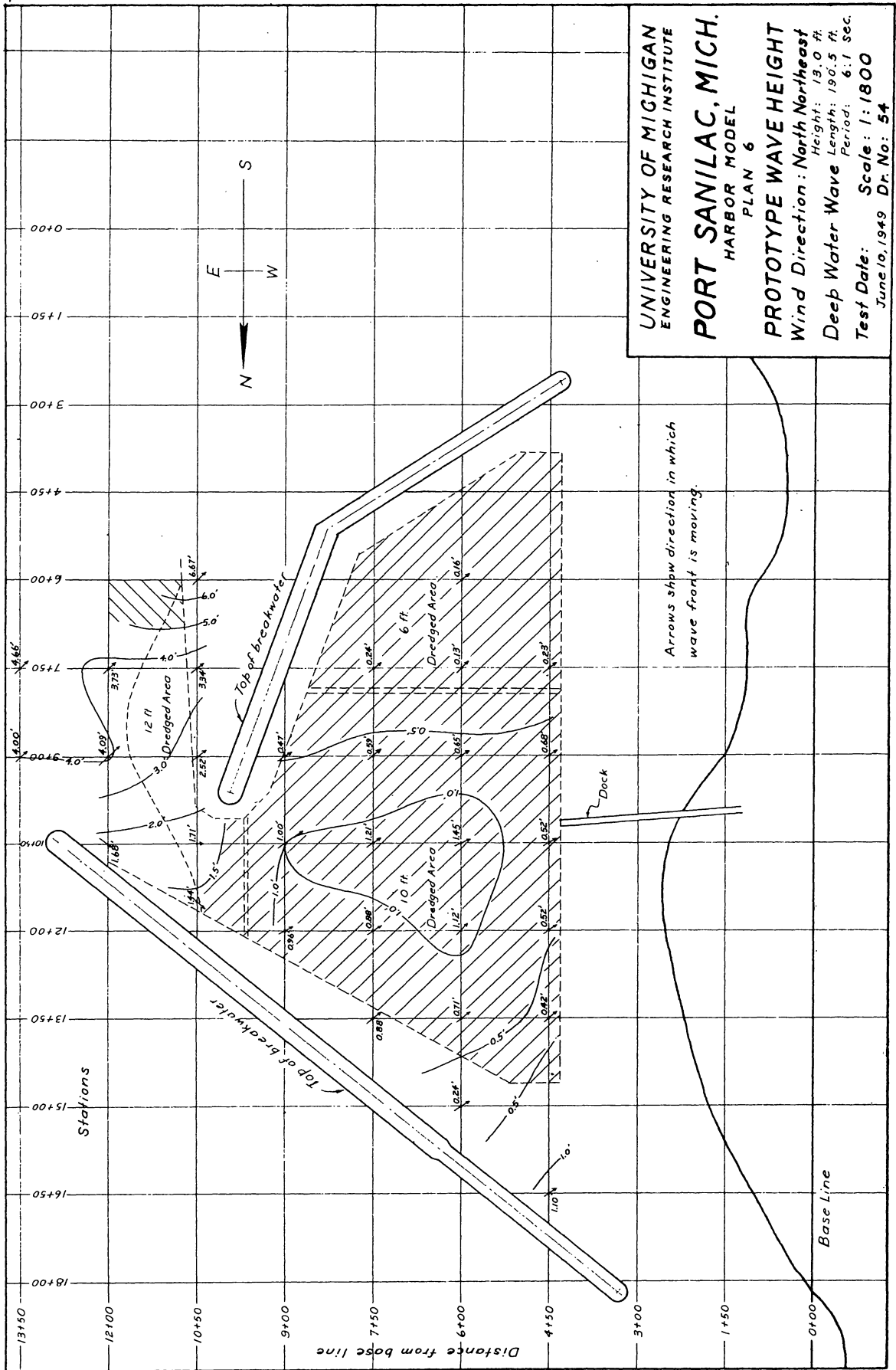
Stations







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**PORT SANILAC, MICH.**  
 HARBOR MODEL  
 PLAN 6  
**PROTOTYPE WAVE HEIGHT**  
 Wind Direction: East  
 Height: 4.5 ft  
 Deep Water Wave Length: 81.9 ft  
 Period: 4.0 Sec.  
 Test Date: June 8, 1949  
 Scale: 1:1800  
 Dr. No.: 53



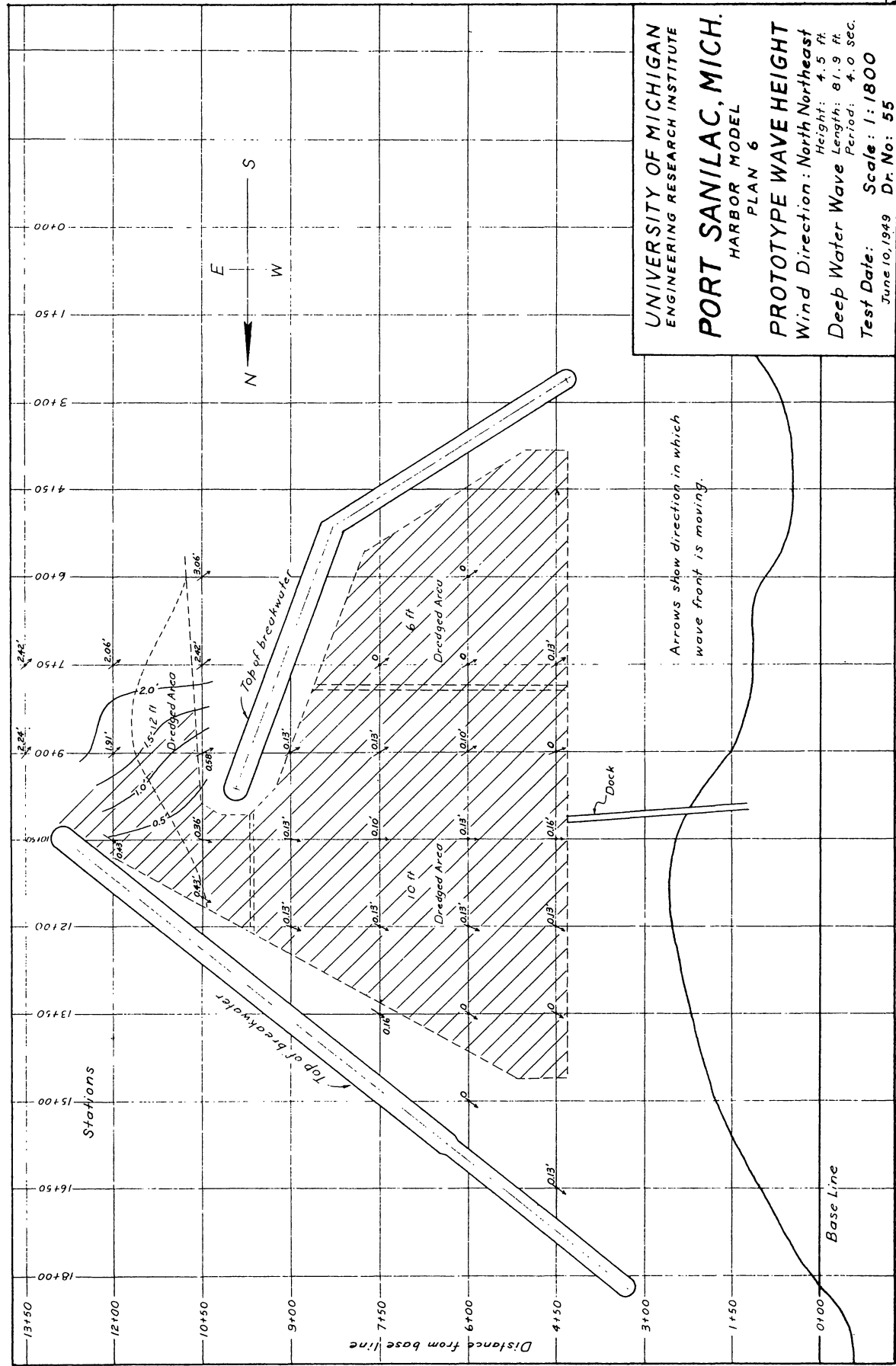
Drawing 54

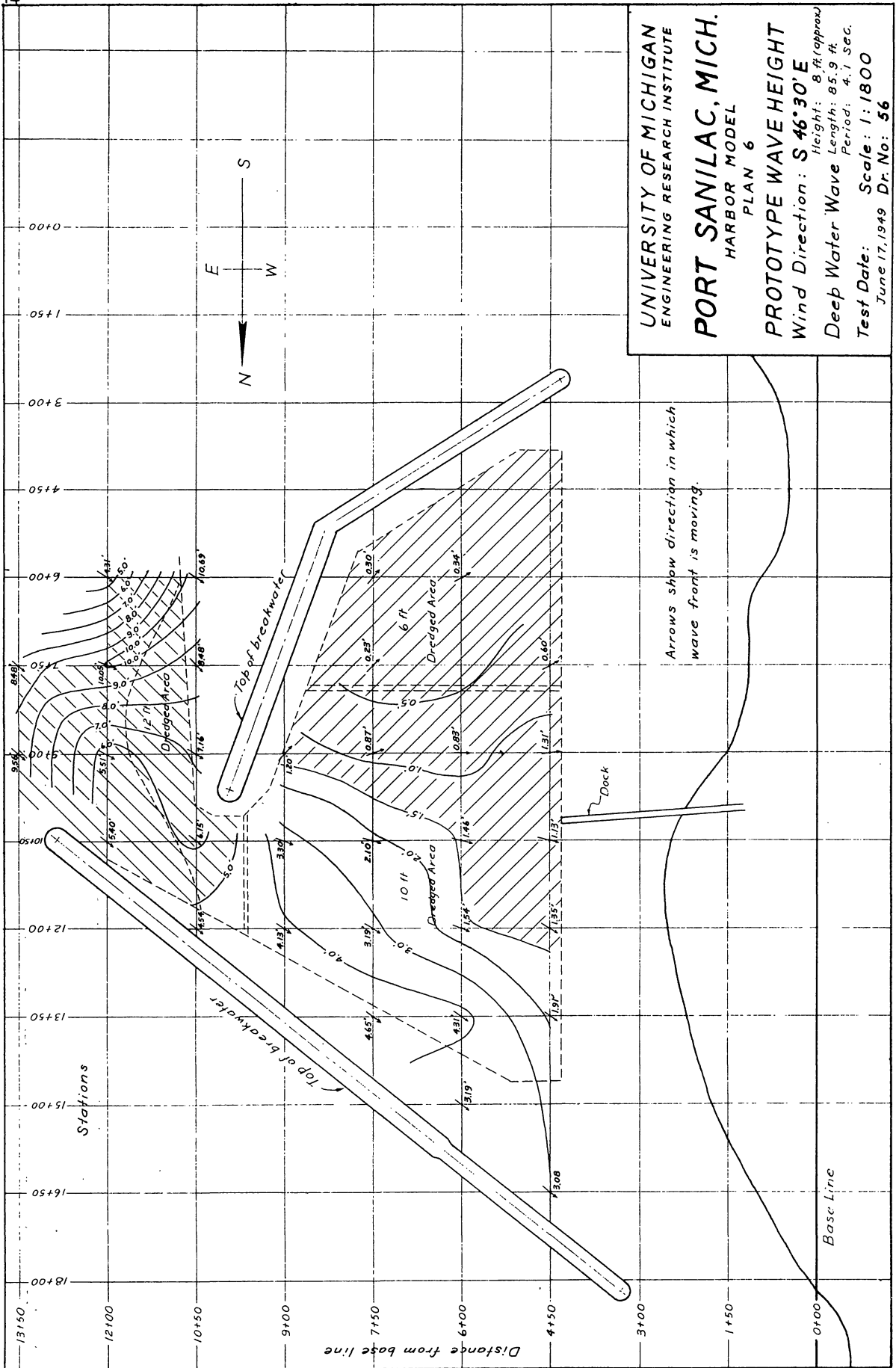
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**PORT SANILAC, MICH.**  
HARBOR MODEL  
PLAN 6

**PROTOTYPE WAVE HEIGHT**  
Wind Direction: North Northeast  
Height: 4.5 ft.  
Deep Water Wave Length: 81.9 ft.  
Period: 4.0 sec.

Test Date: June 10, 1949 Scale: 1:1800  
Dr. No.: 55



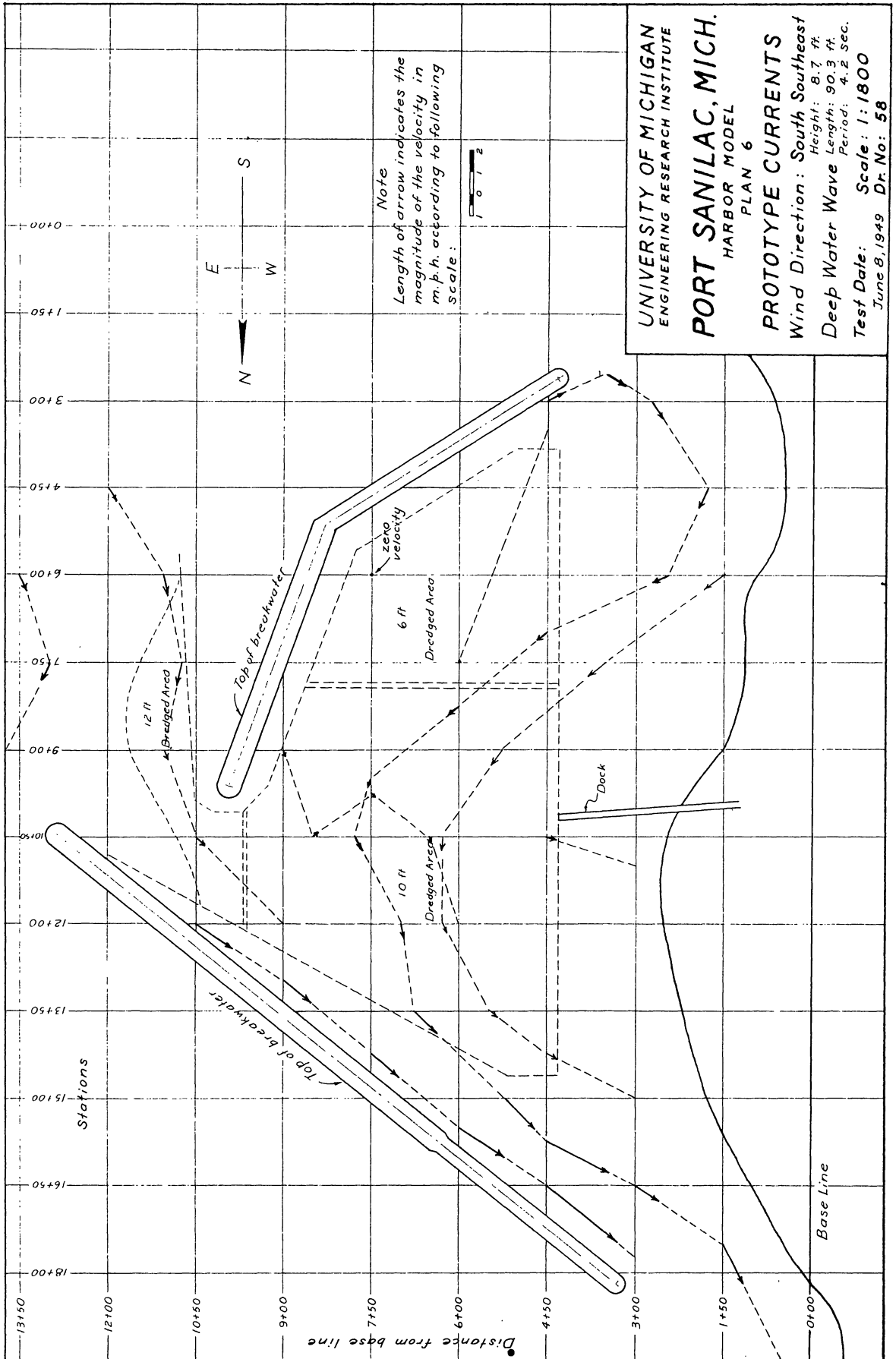


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**PORT SANILAC, MICH.**  
 HARBOR MODEL  
 PLAN 6  
**PROTOTYPE WAVE HEIGHT**  
 Wind Direction: S 46° 30' E  
 Height: 8 ft. (approx)  
 Length: 85.9 ft.  
 Deep Water Wave Period: 4.1 Sec.  
 Test Date: June 17, 1949  
 Scale: 1:1800  
 Dr. No: 56

Drawing 56

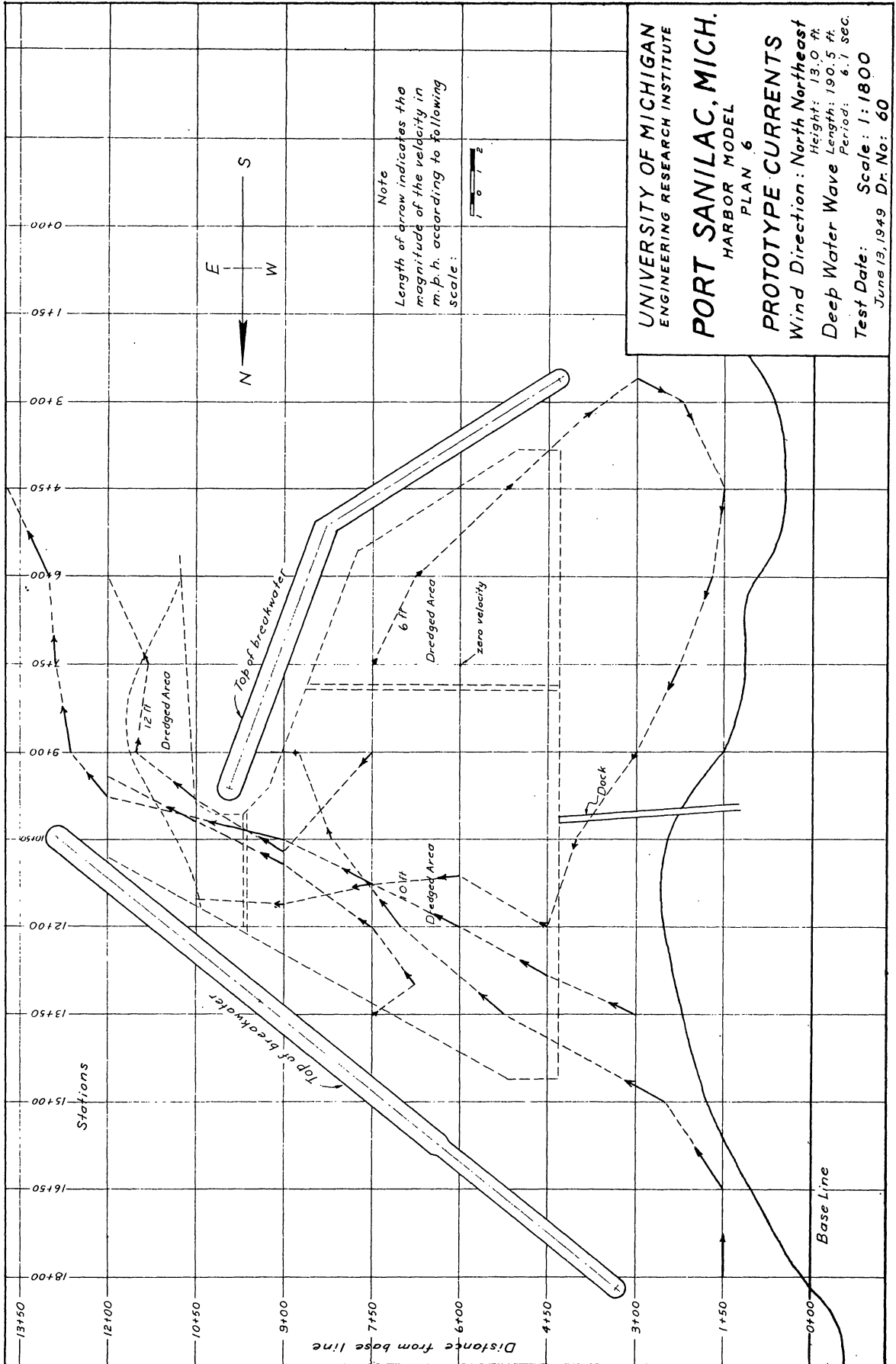


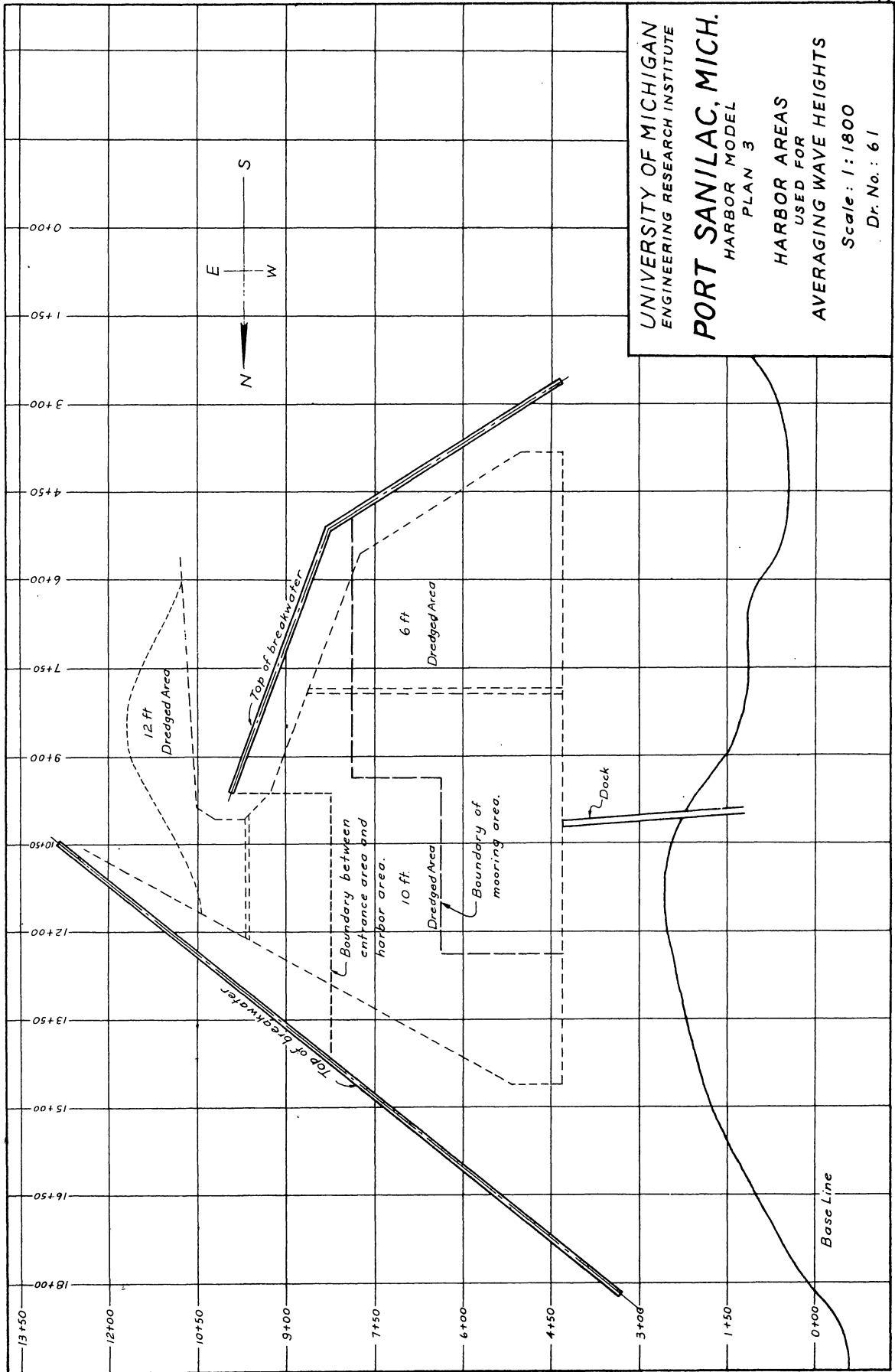




Drawing 58



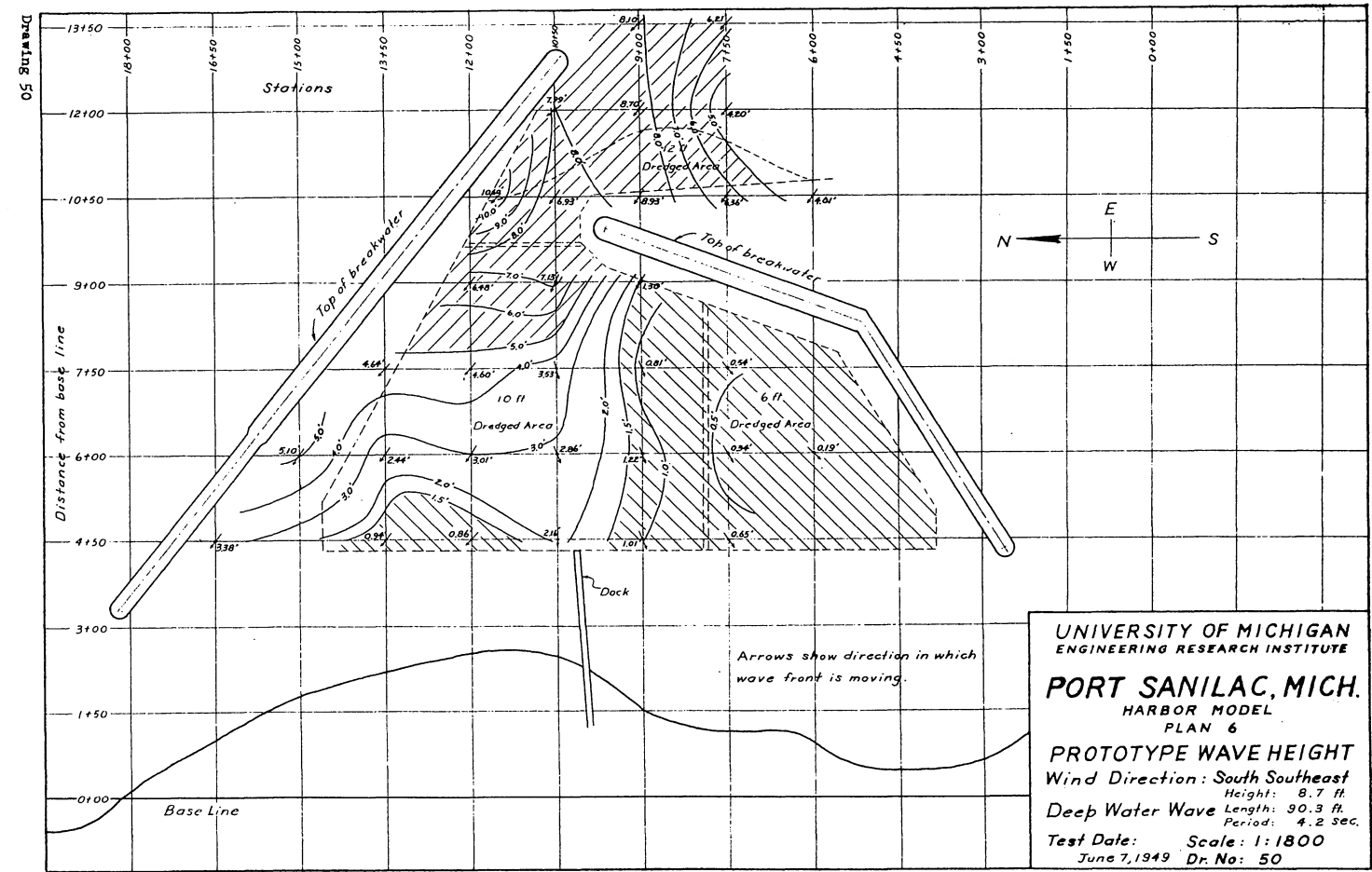
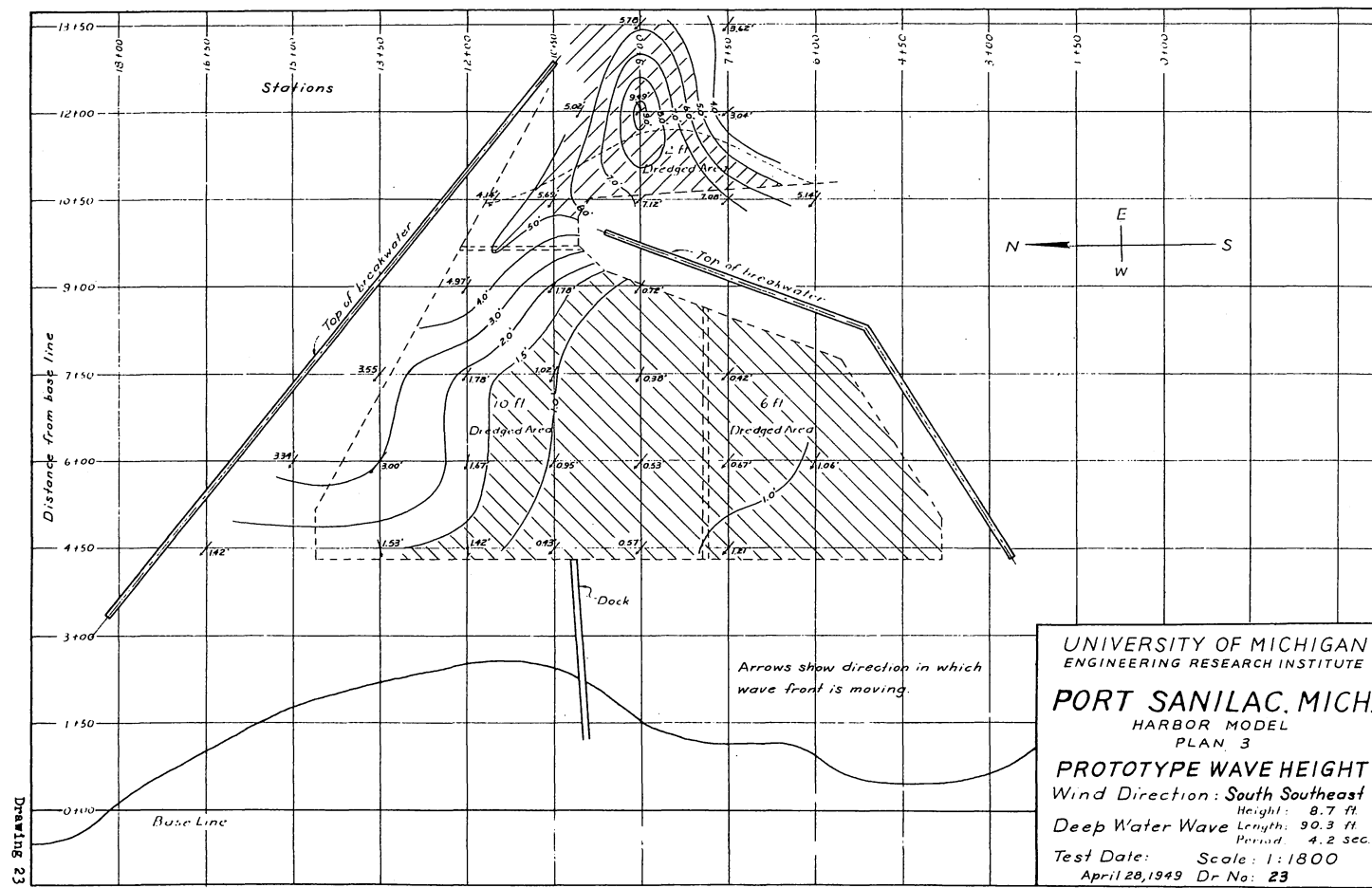
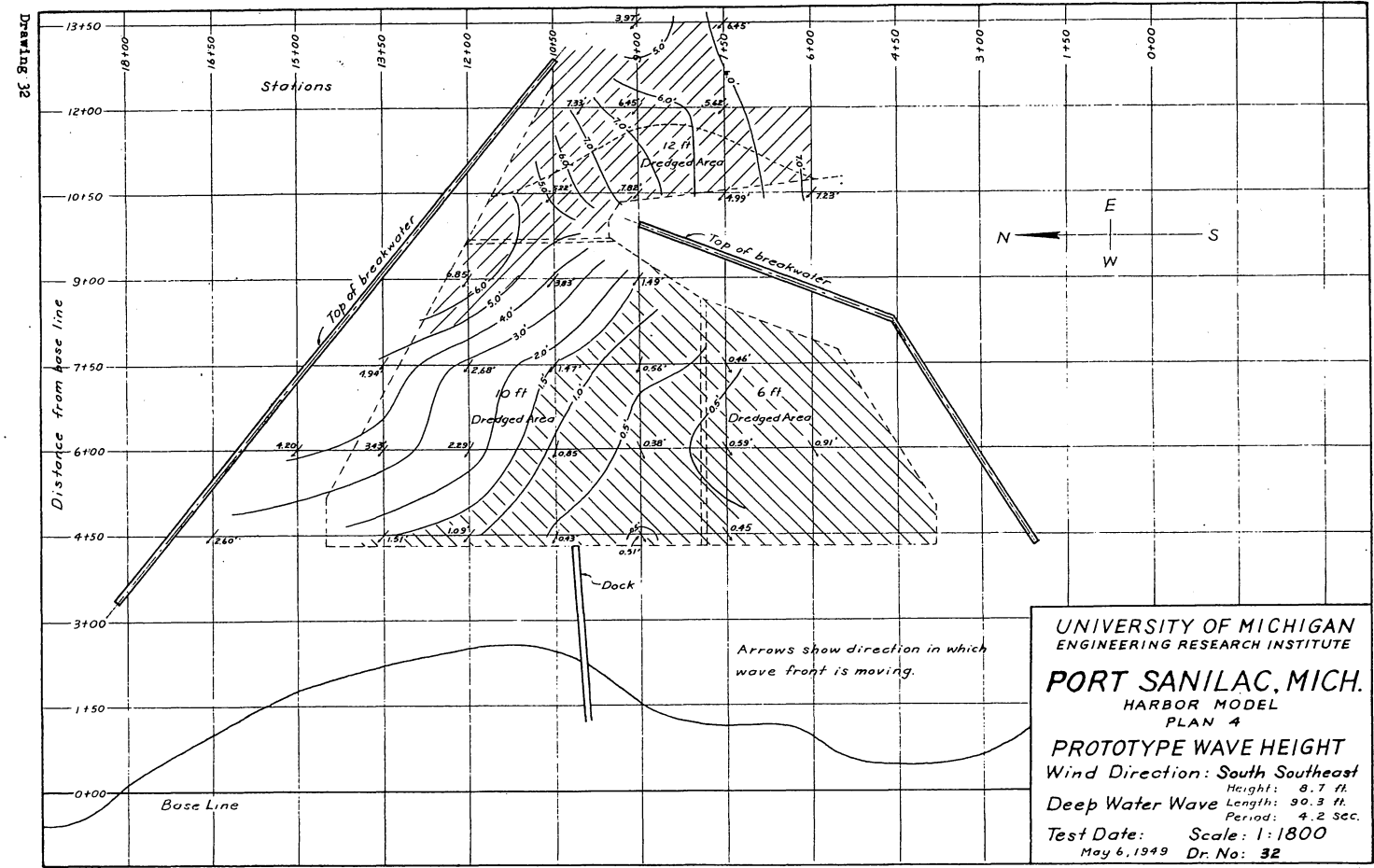
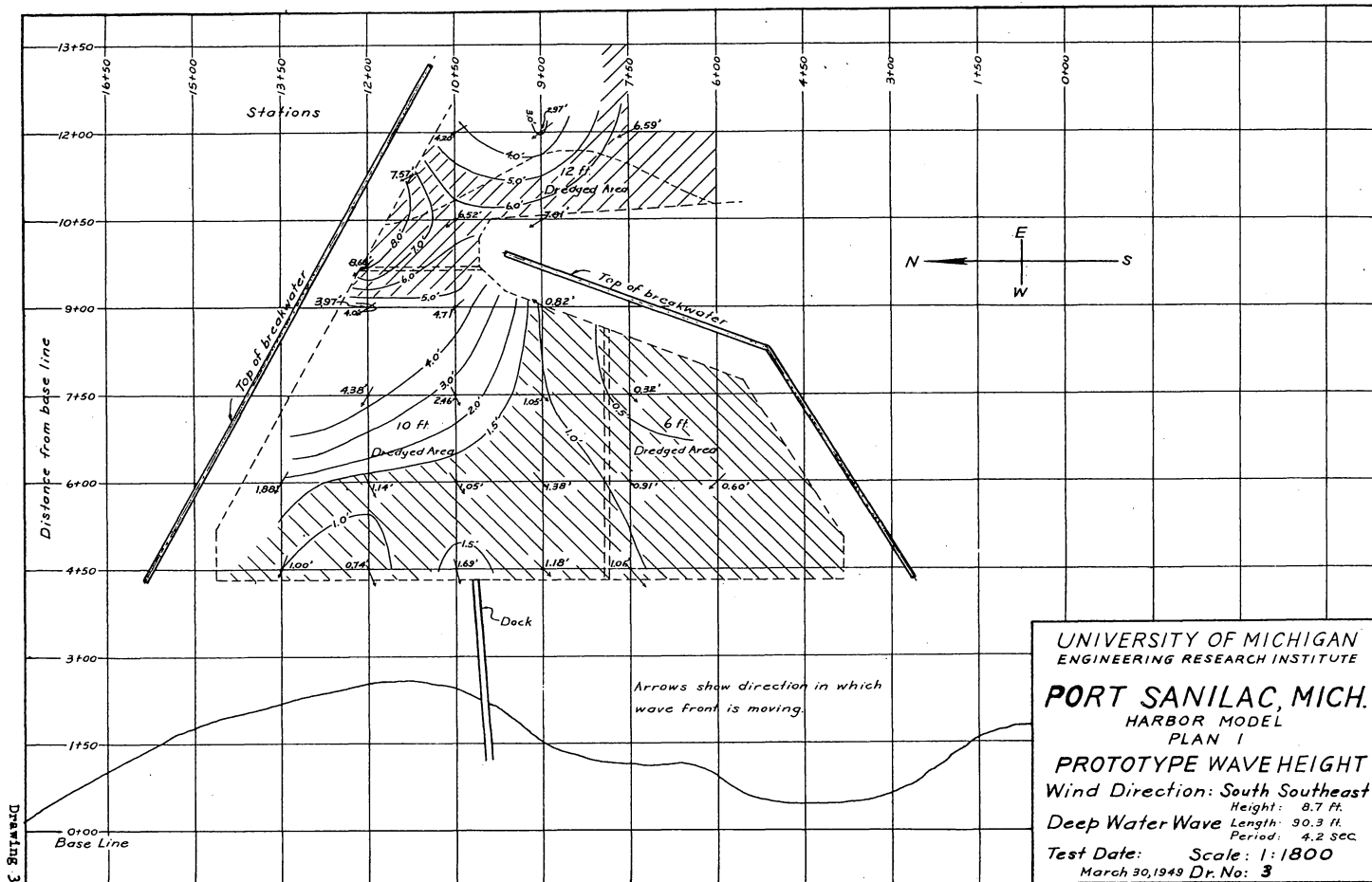




UNIVERSITY OF MICHIGAN  
 ENGINEERING RESEARCH INSTITUTE  
**PORT SANILAC, MICH.**  
 HARBOR MODEL  
 PLAN 3  
 HARBOR AREAS  
 USED FOR  
 AVERAGING WAVE HEIGHTS  
 Scale: 1:1800  
 Dr. No.: 61

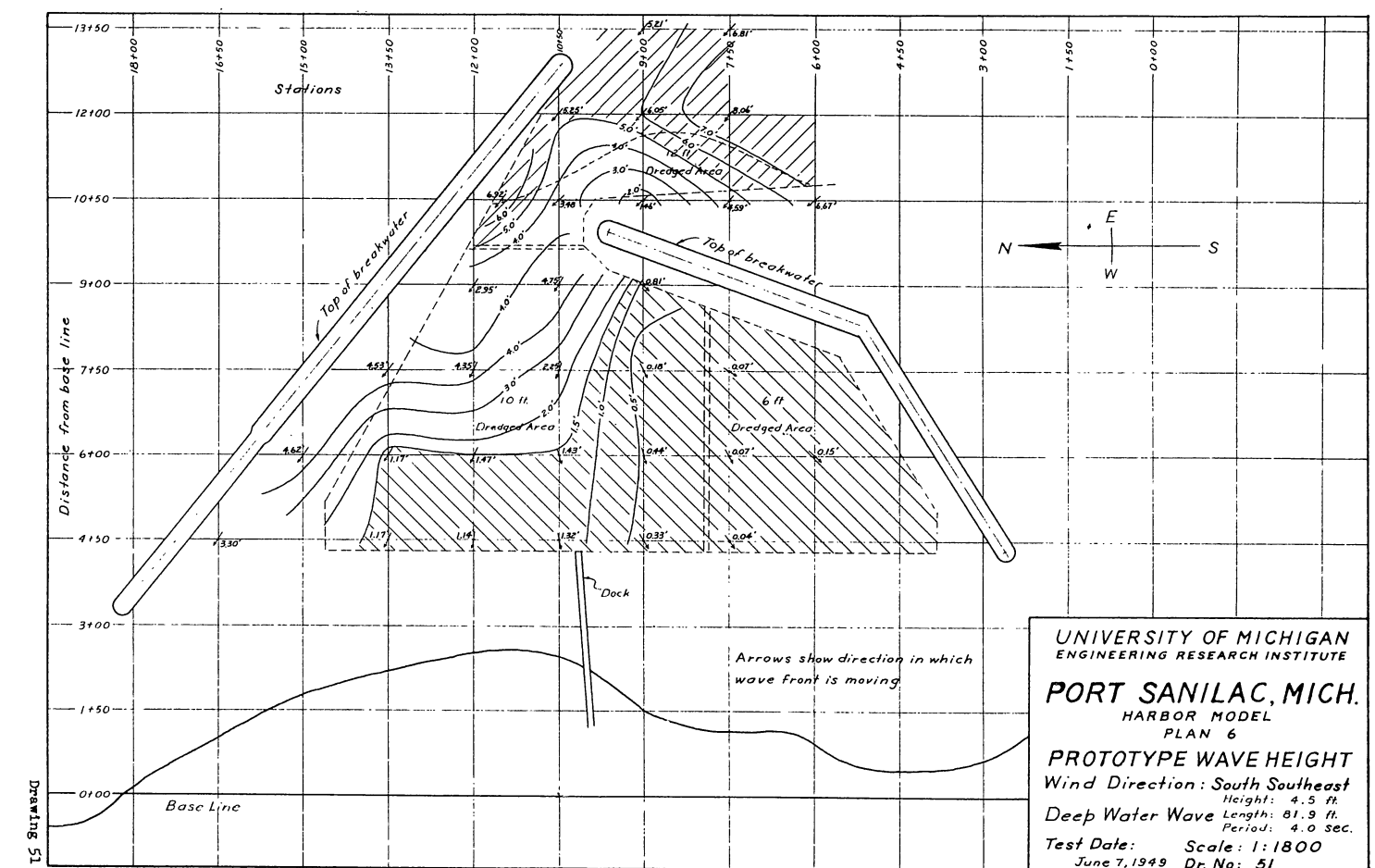
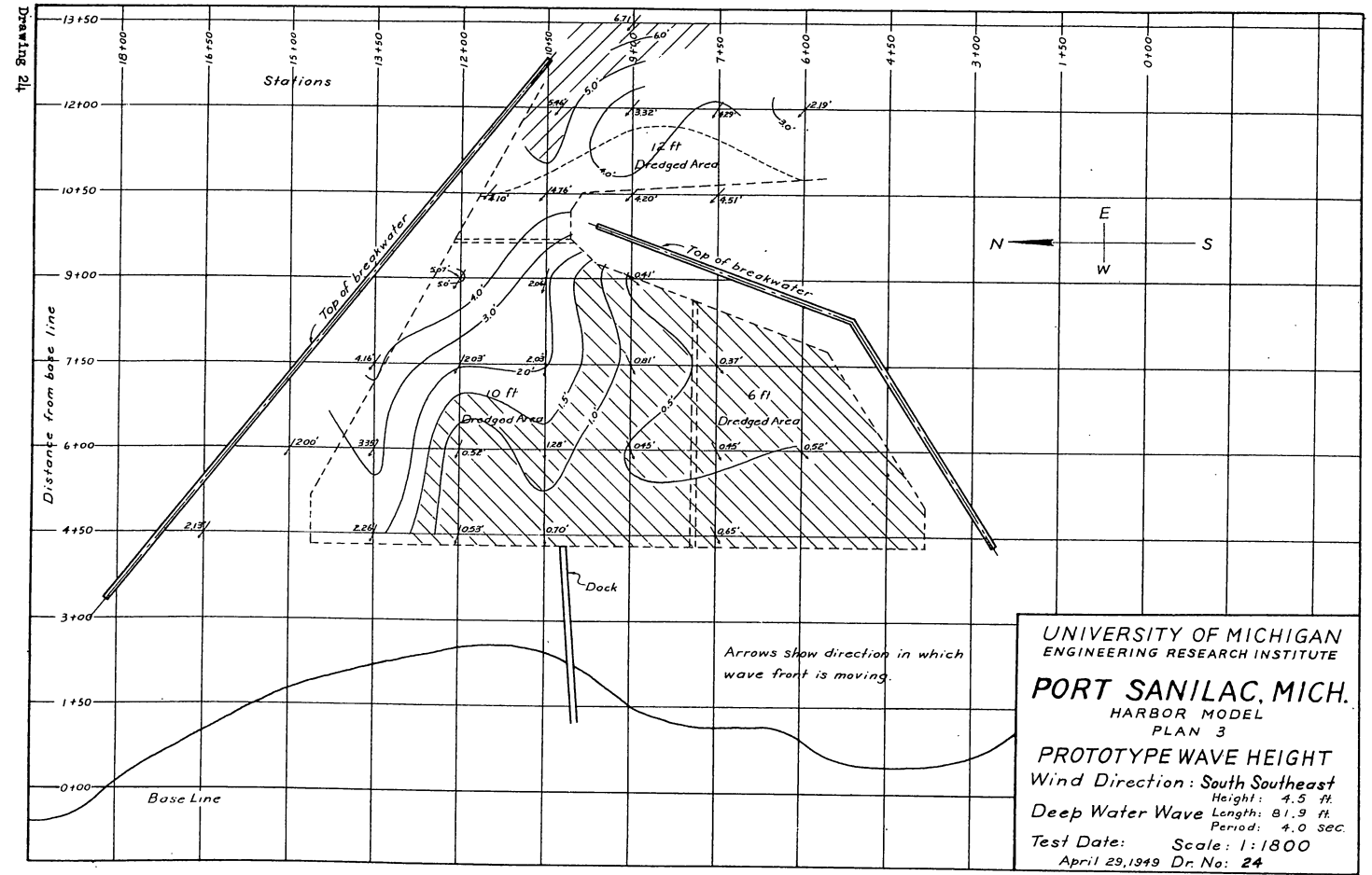
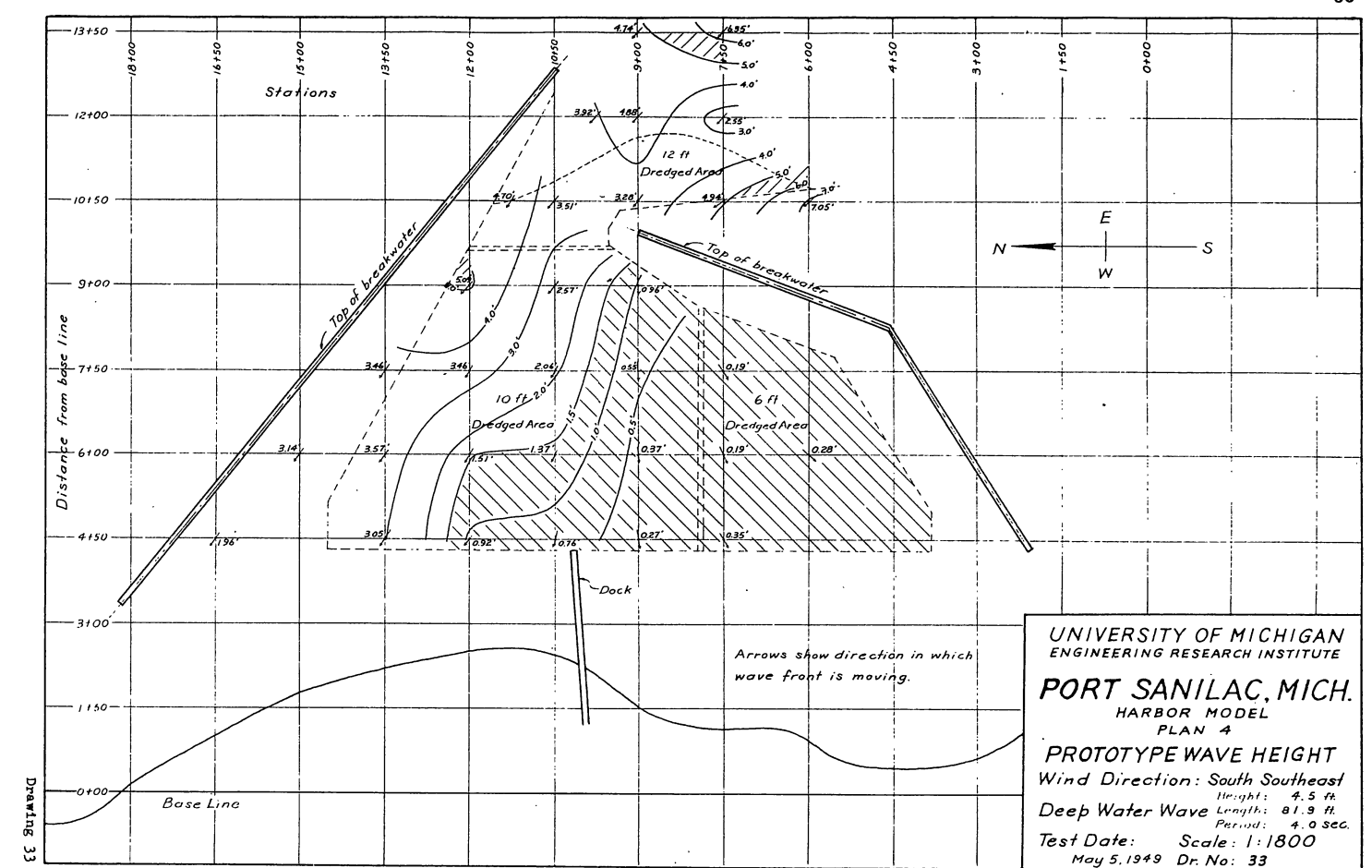
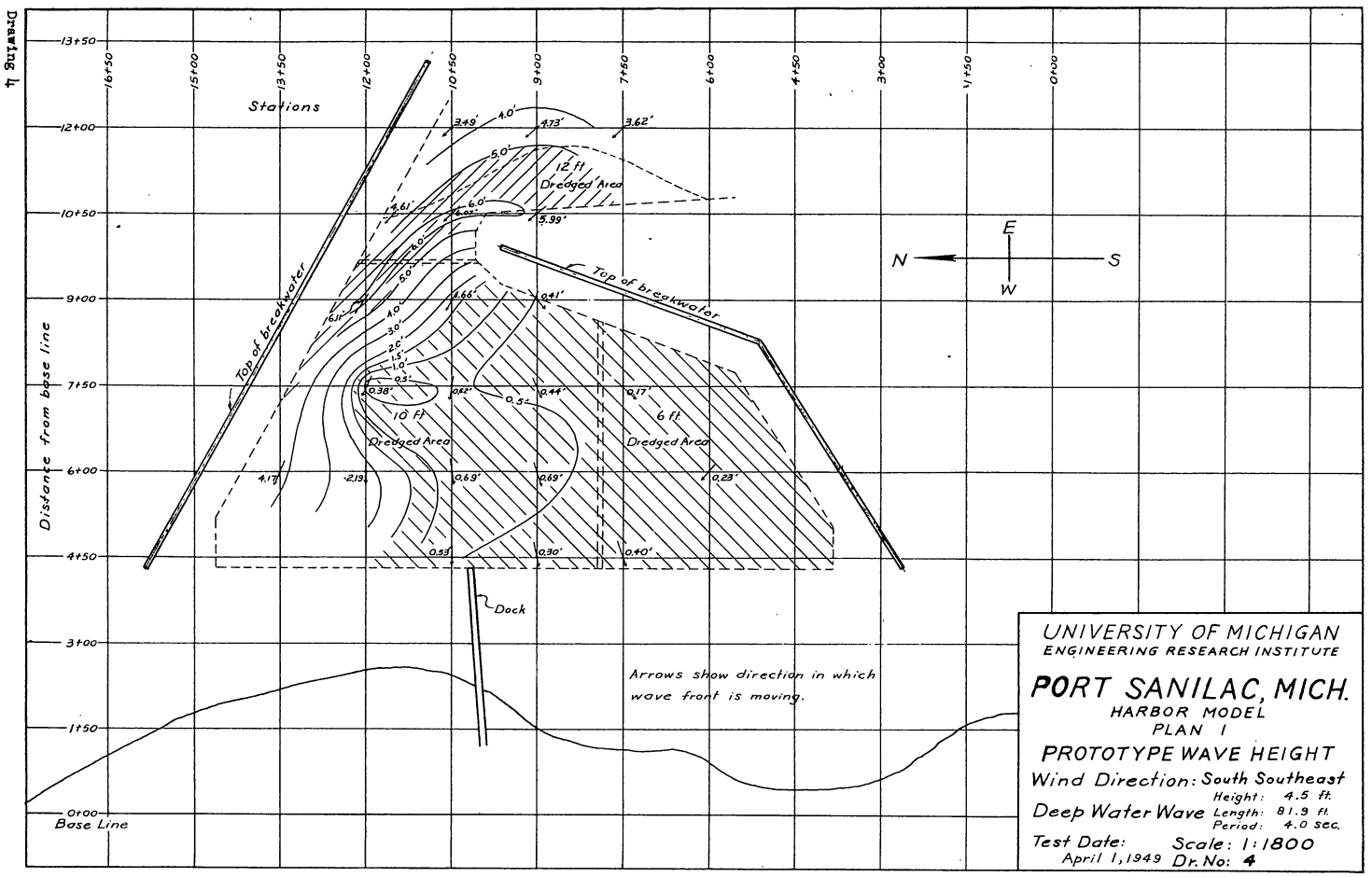
Drawing 61



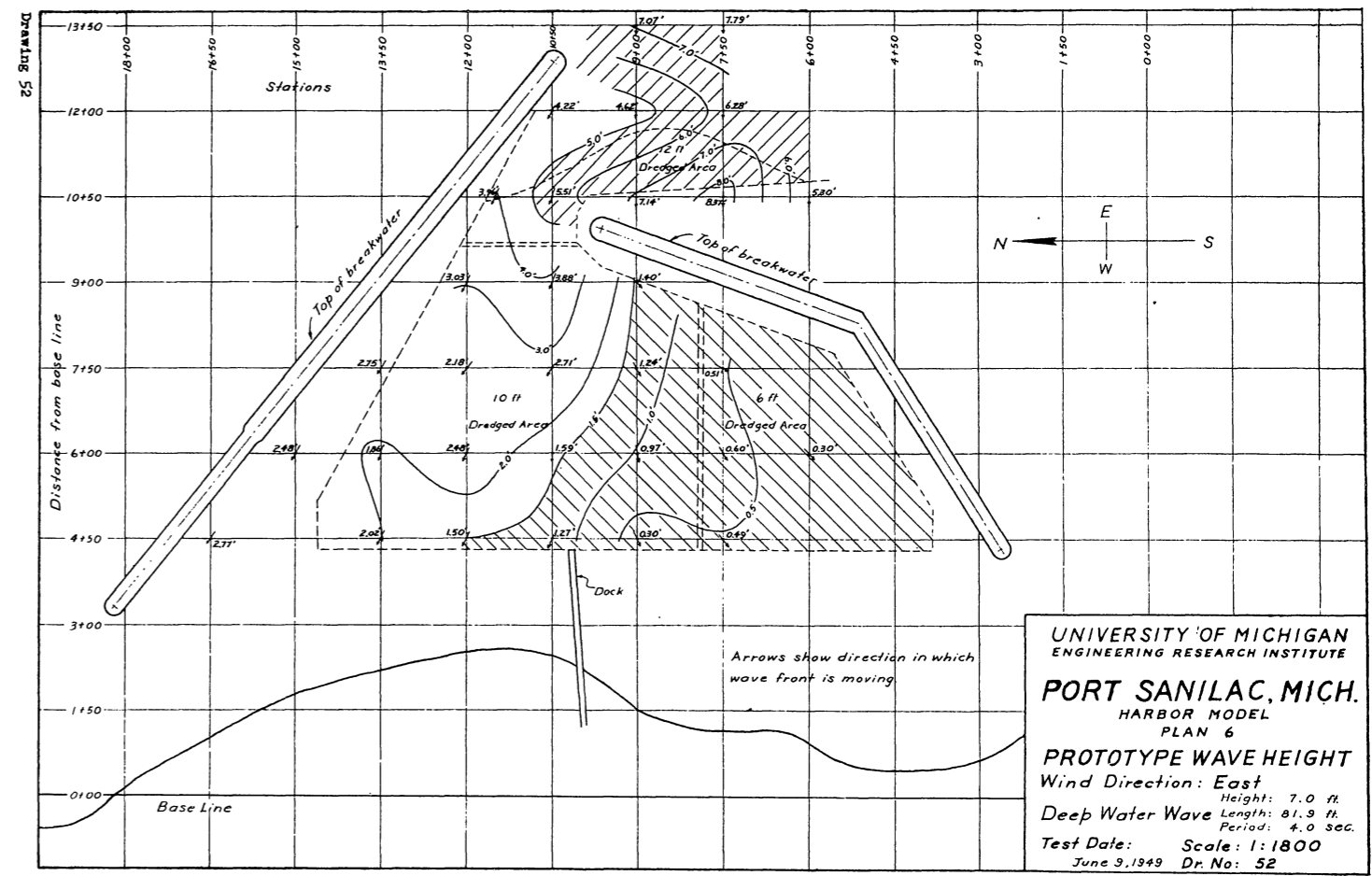
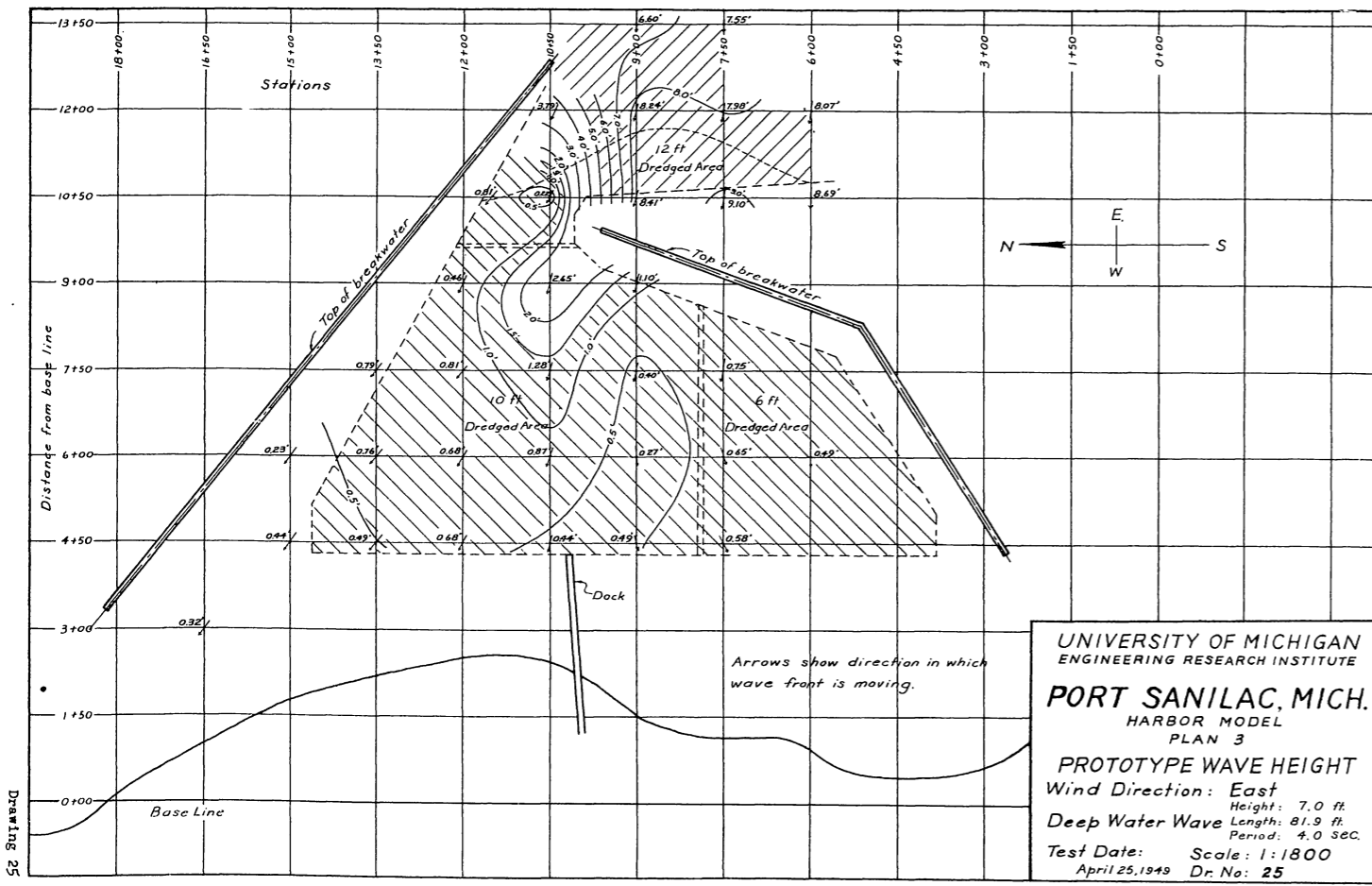
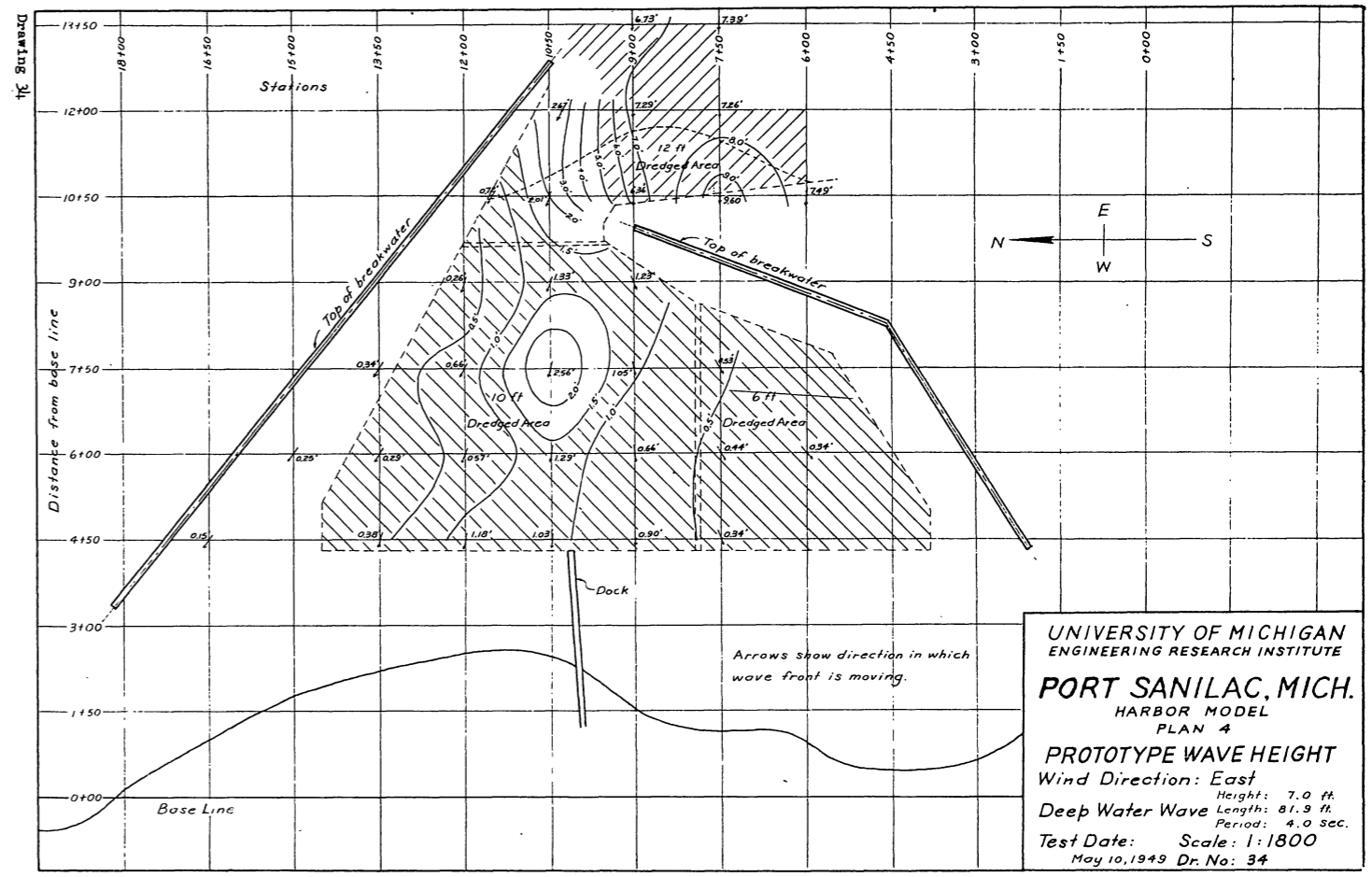
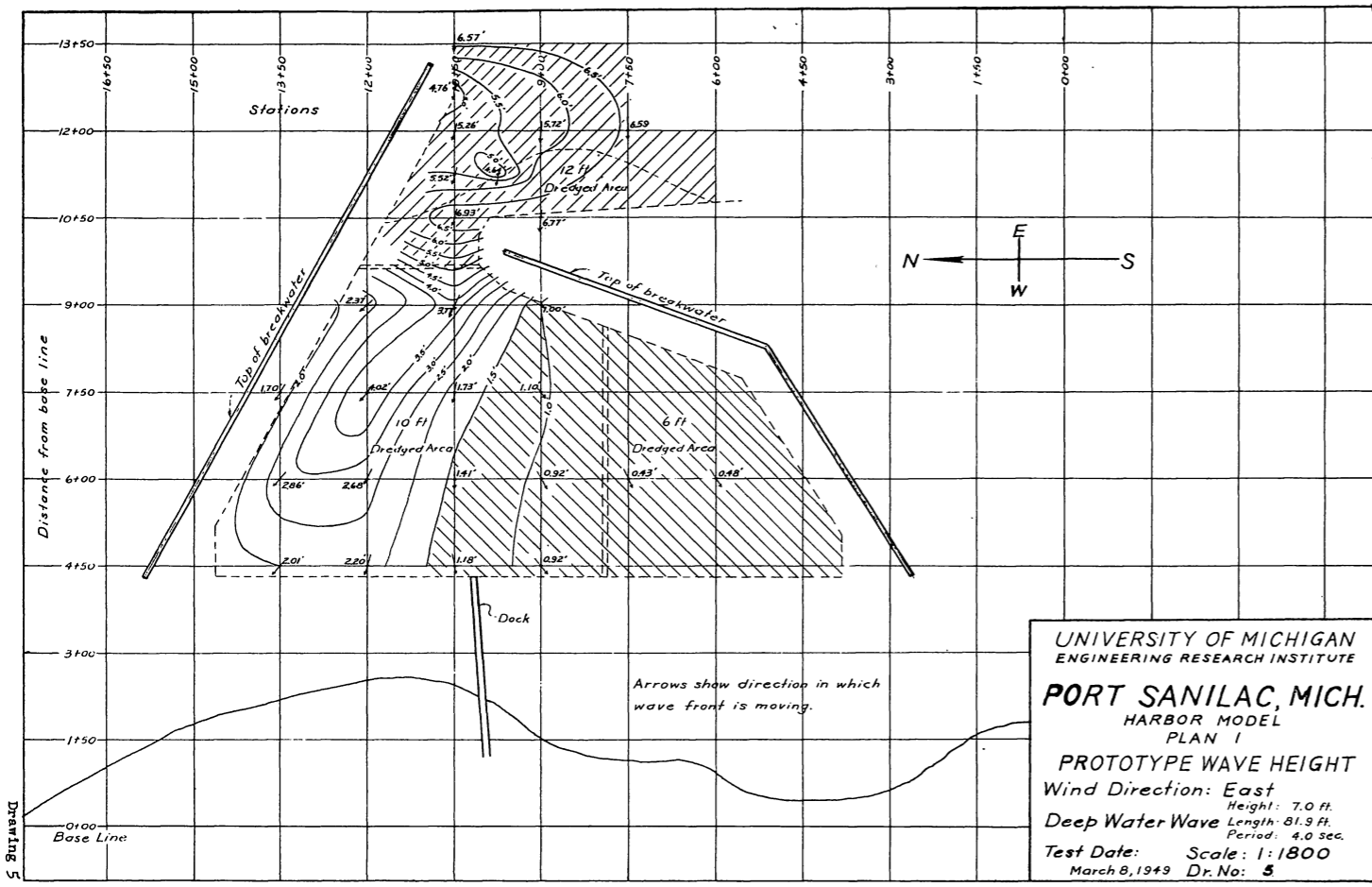








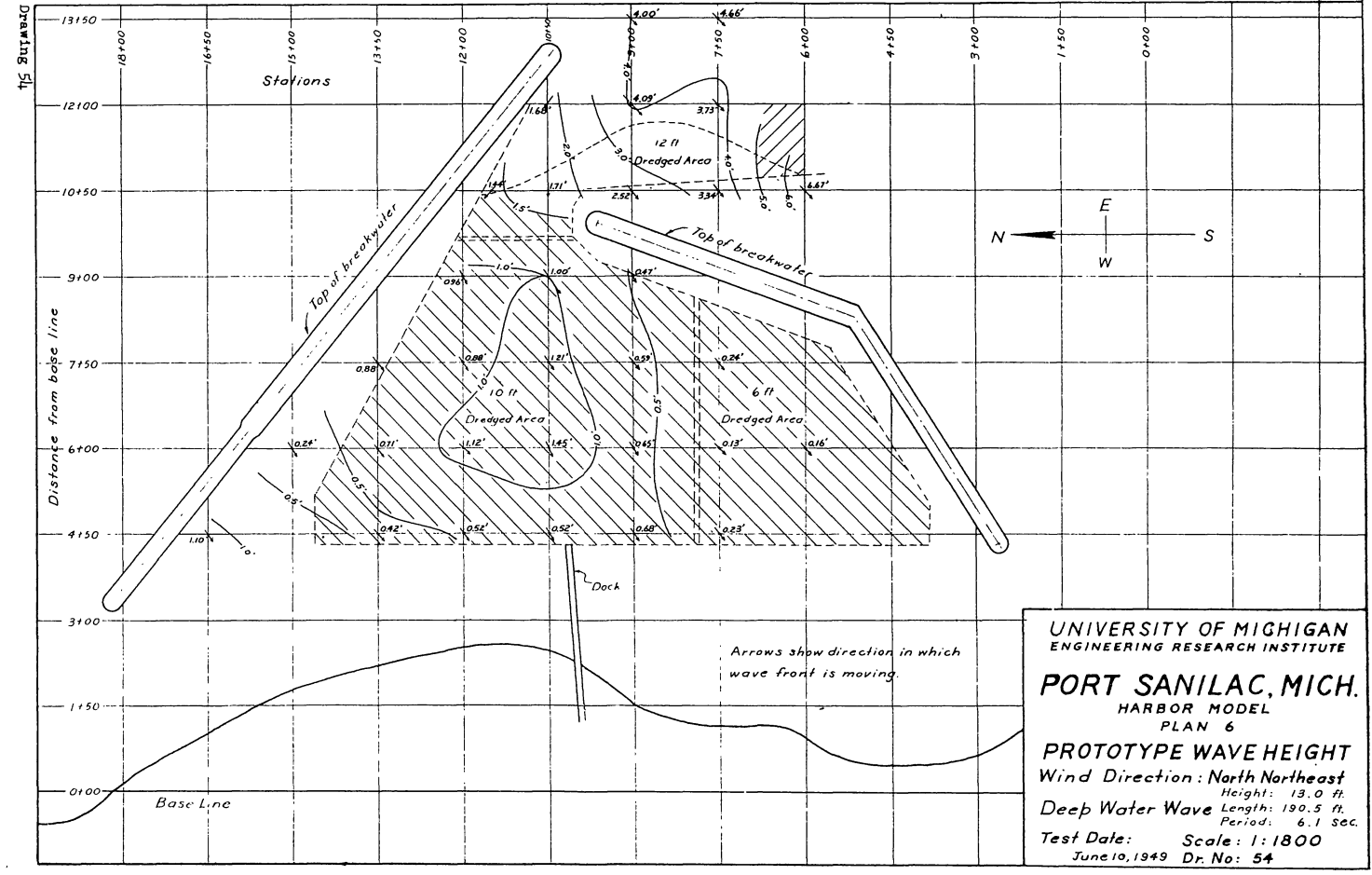
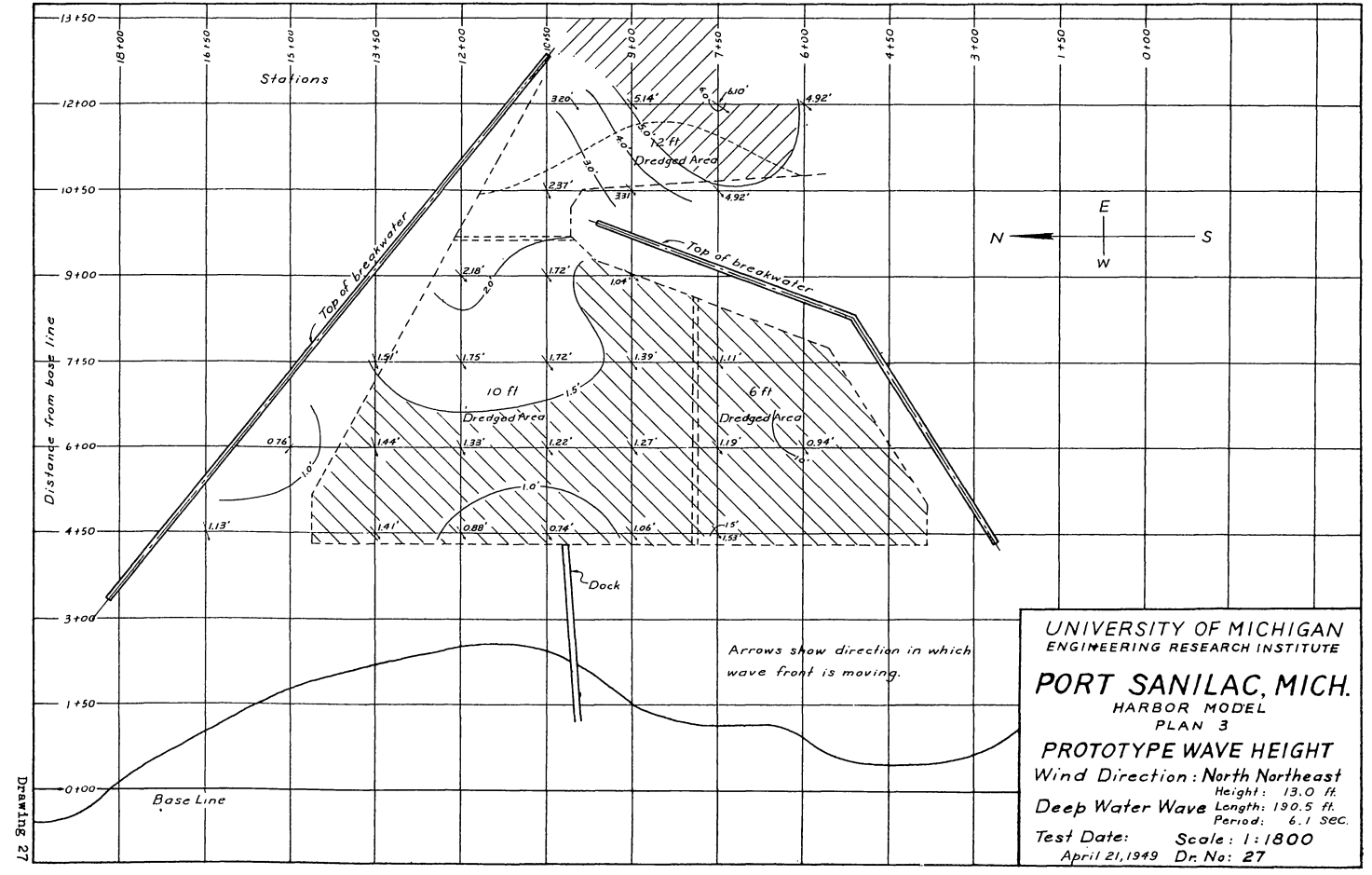
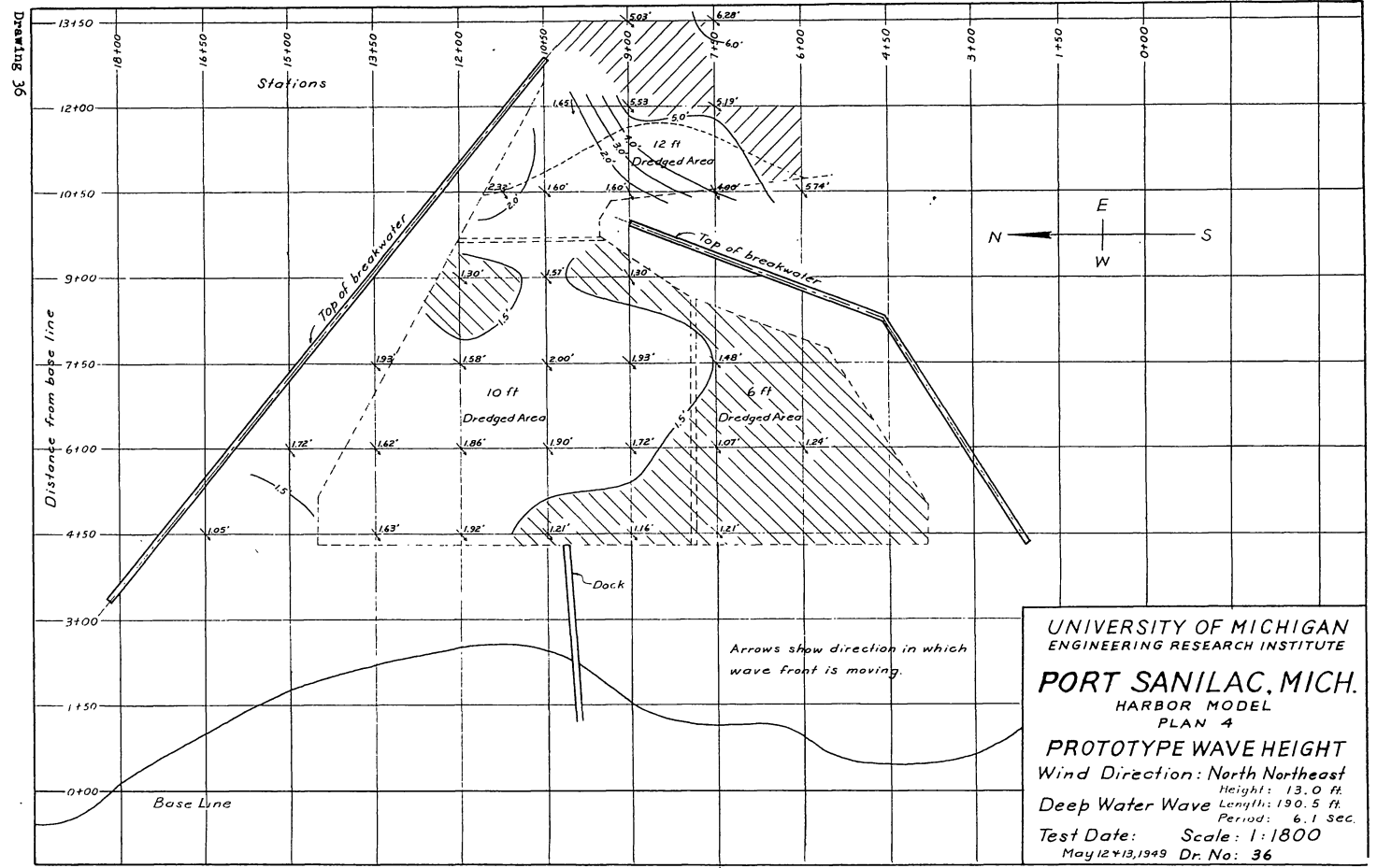
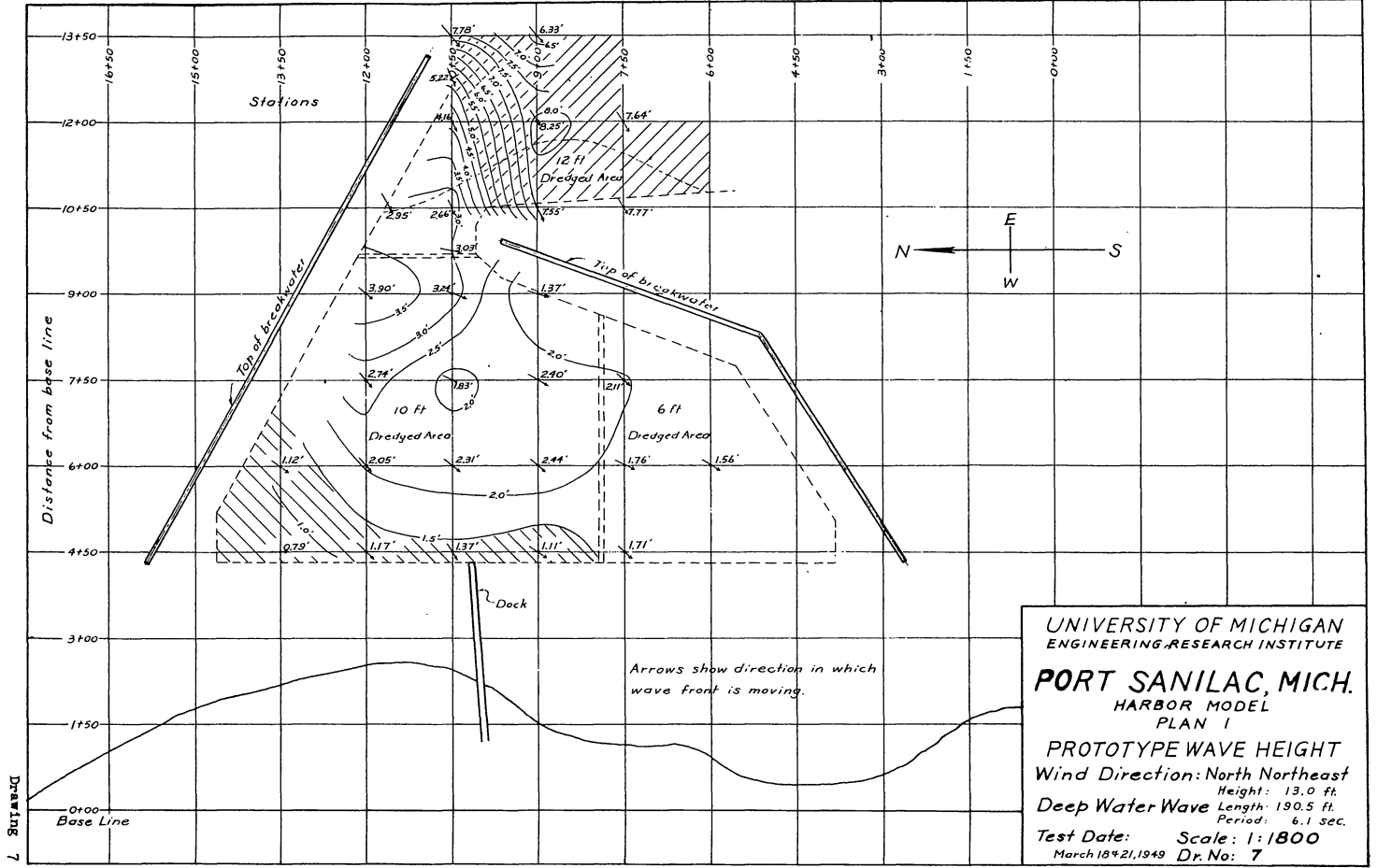






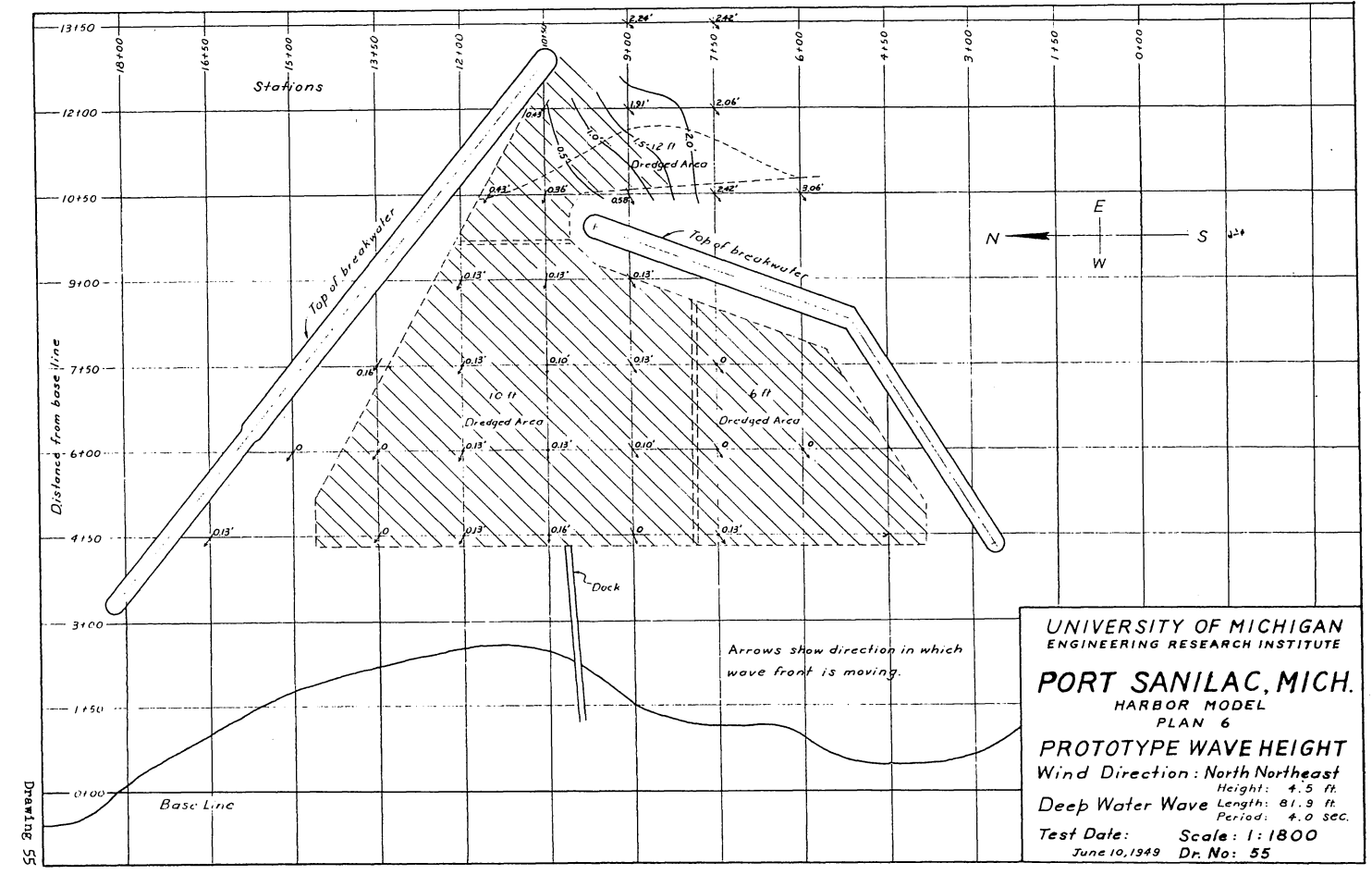
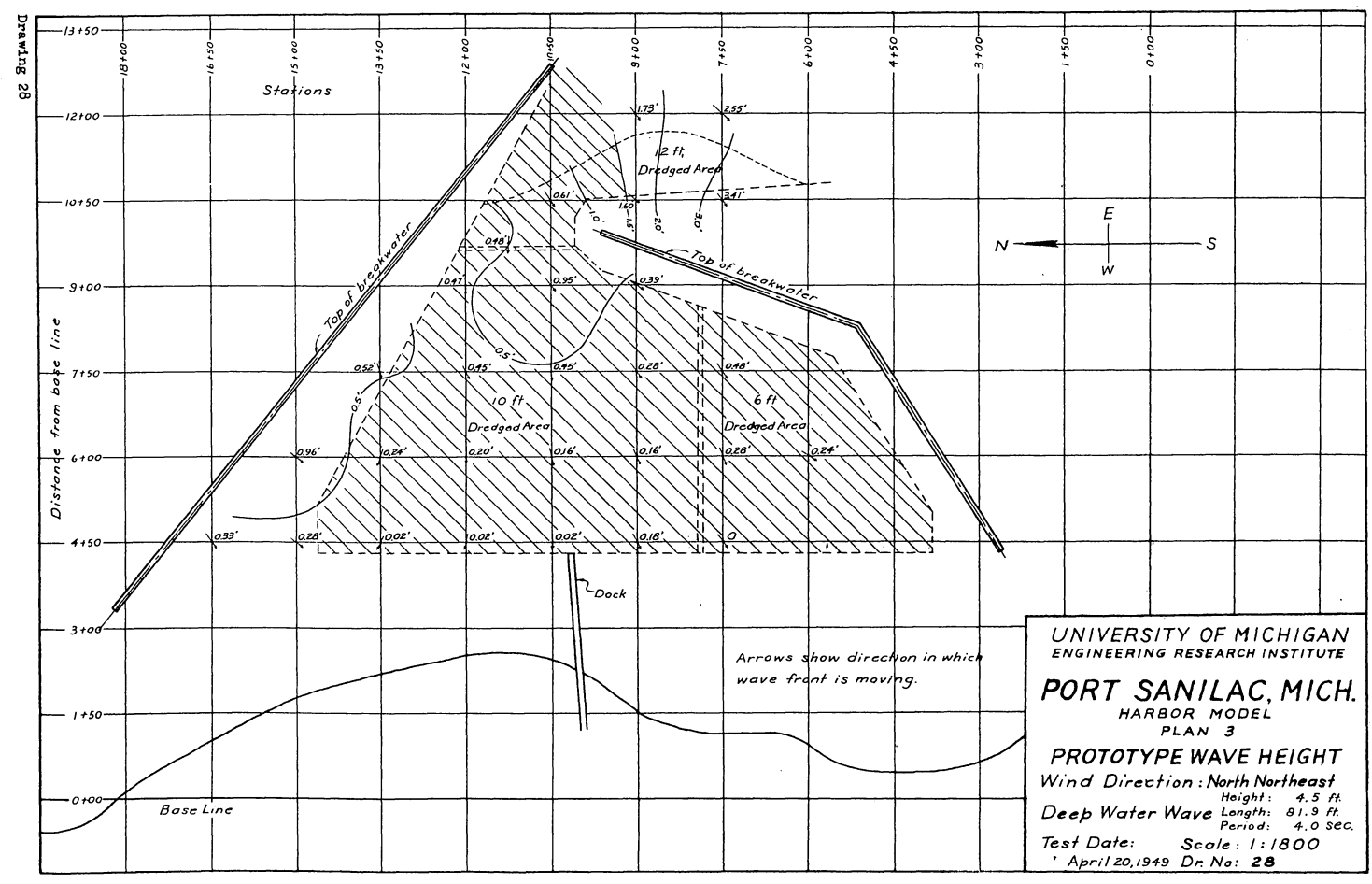
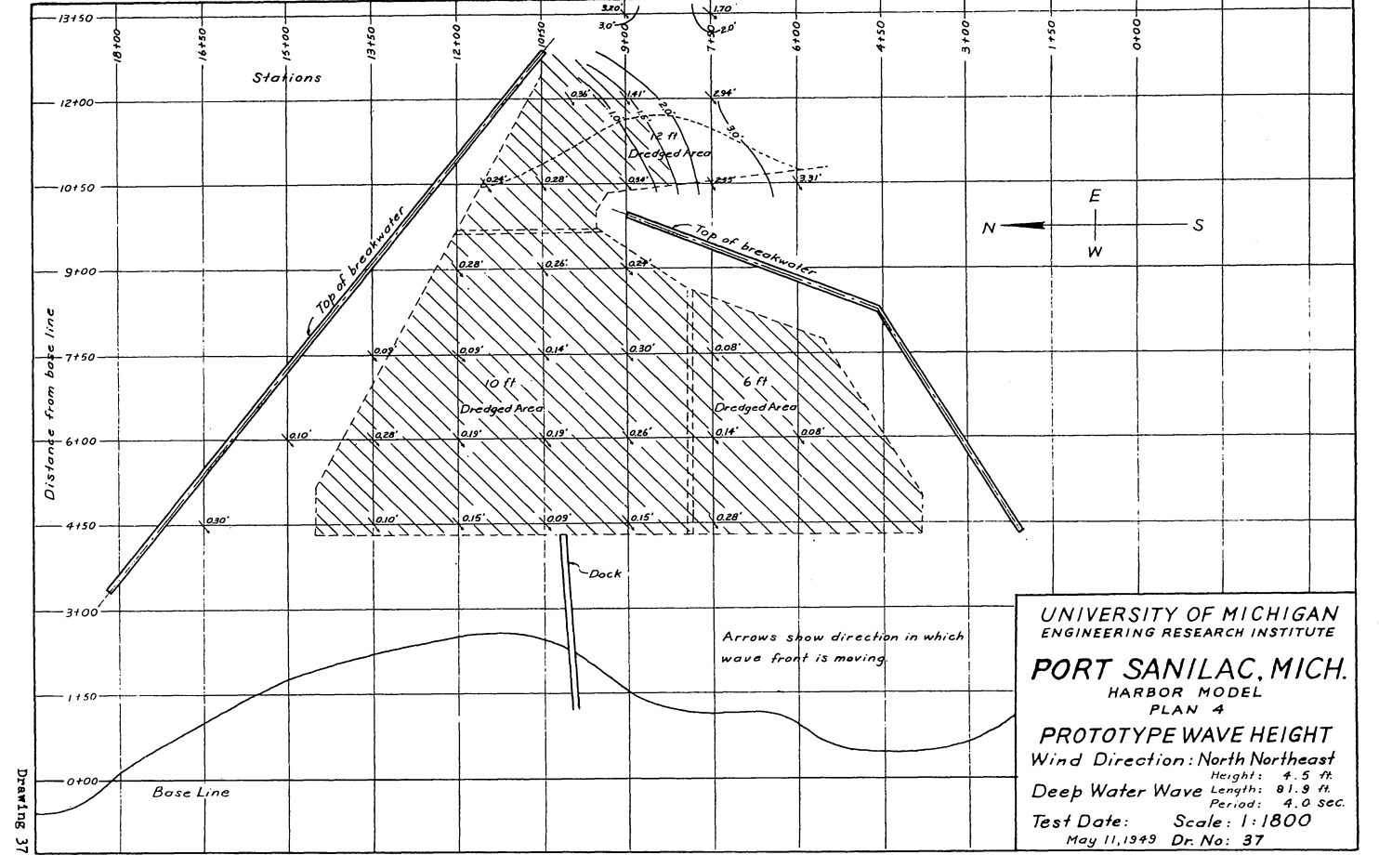
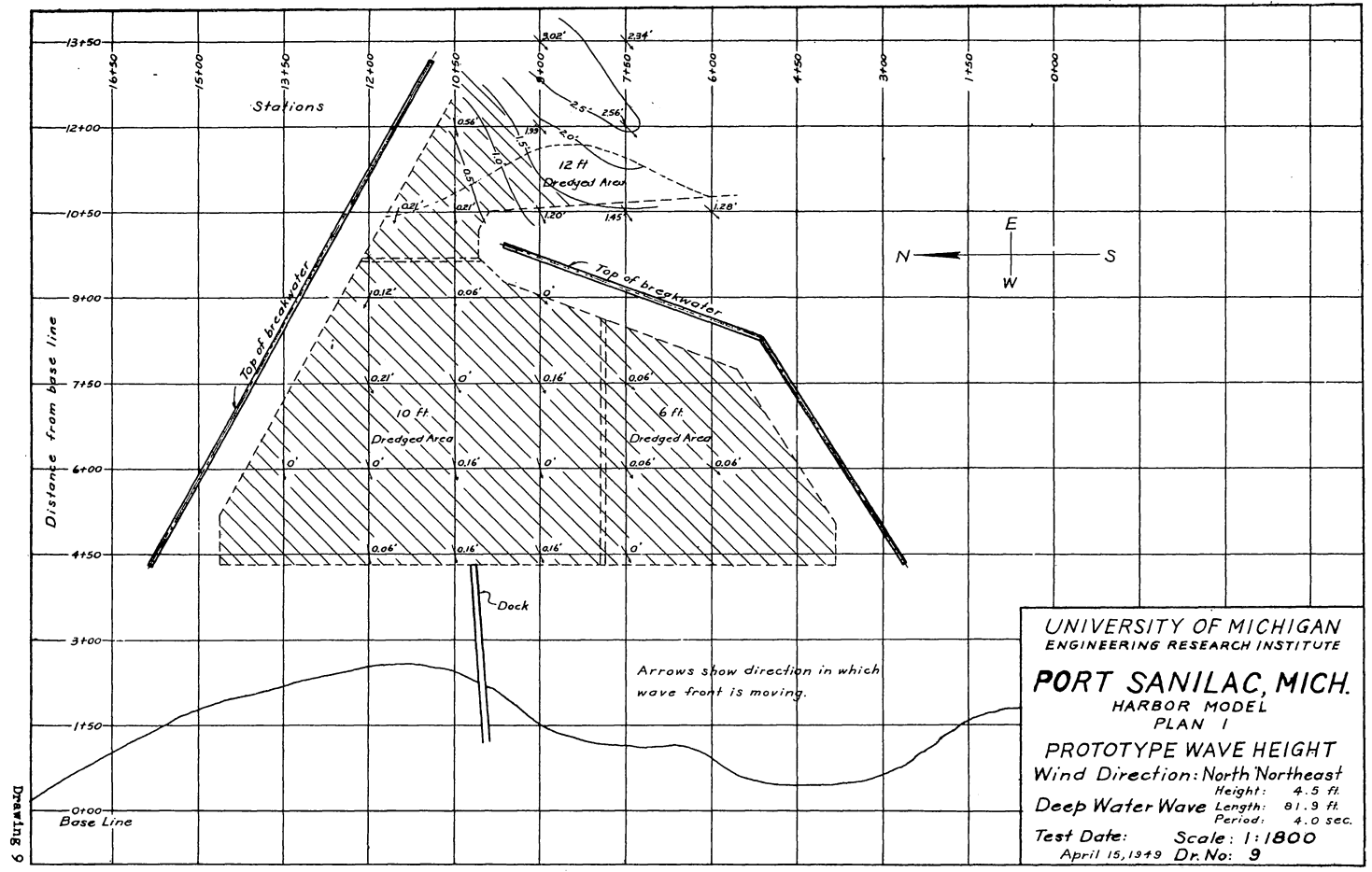










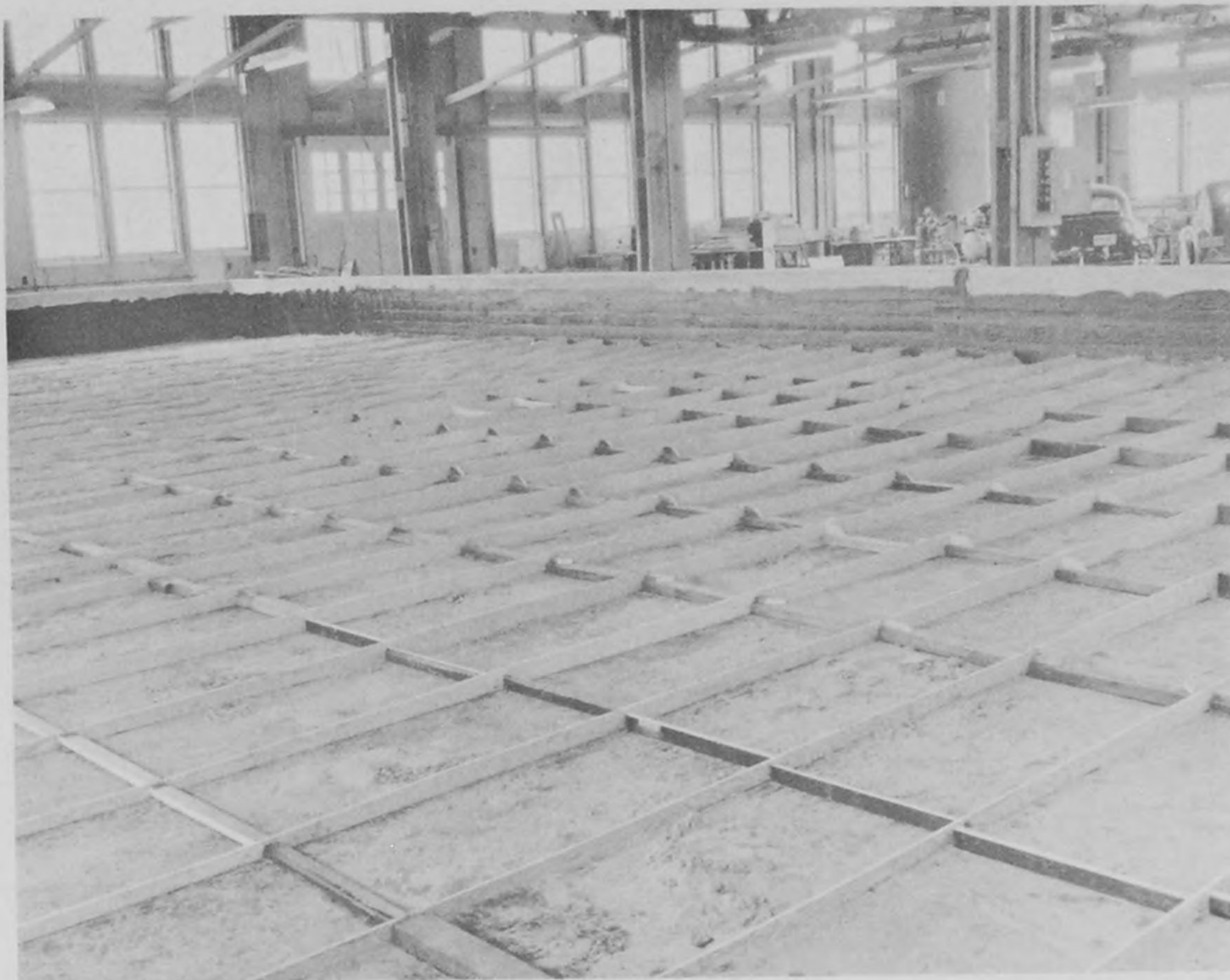




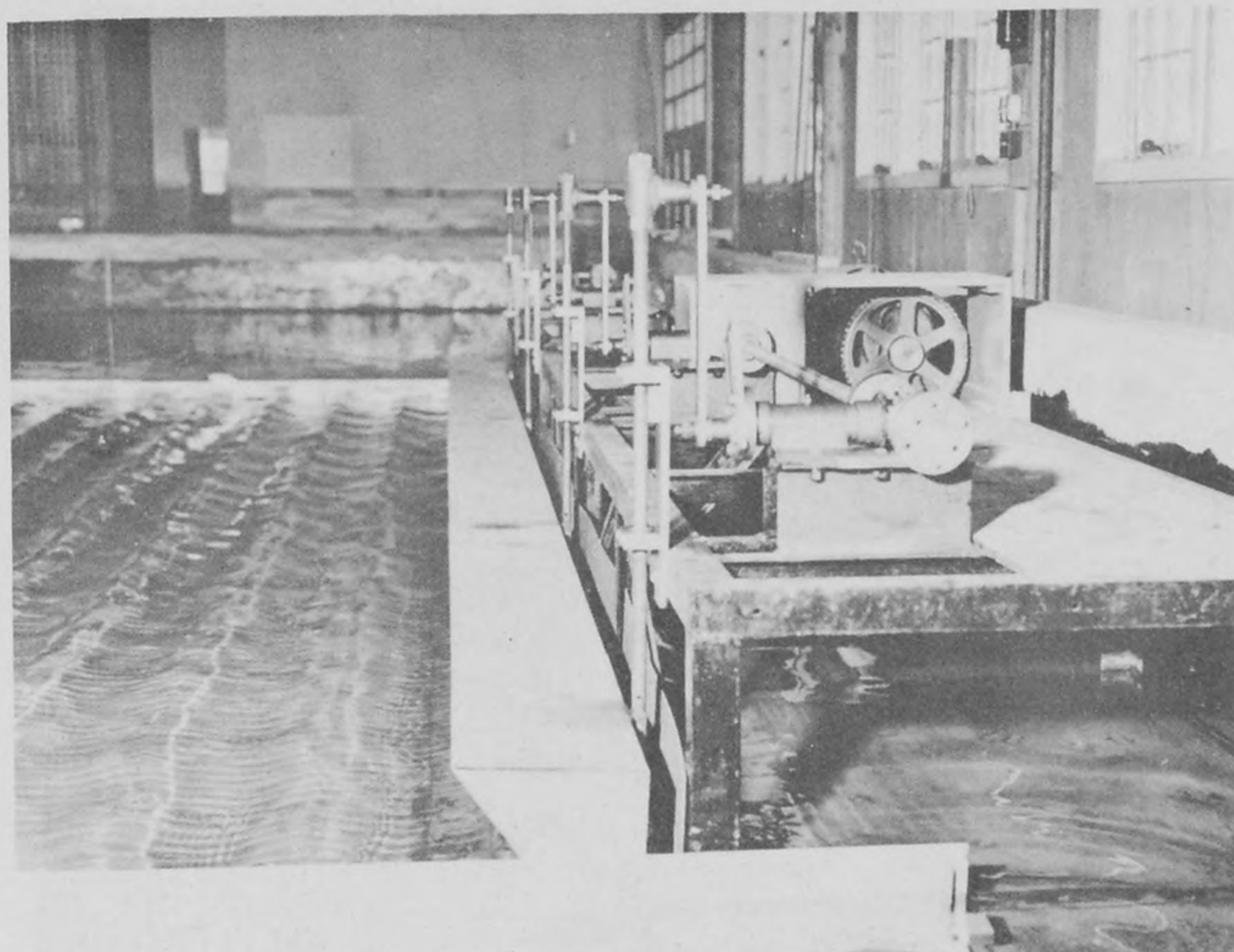
APPENDIX B

PHOTOGRAPHS





TEMPLATES

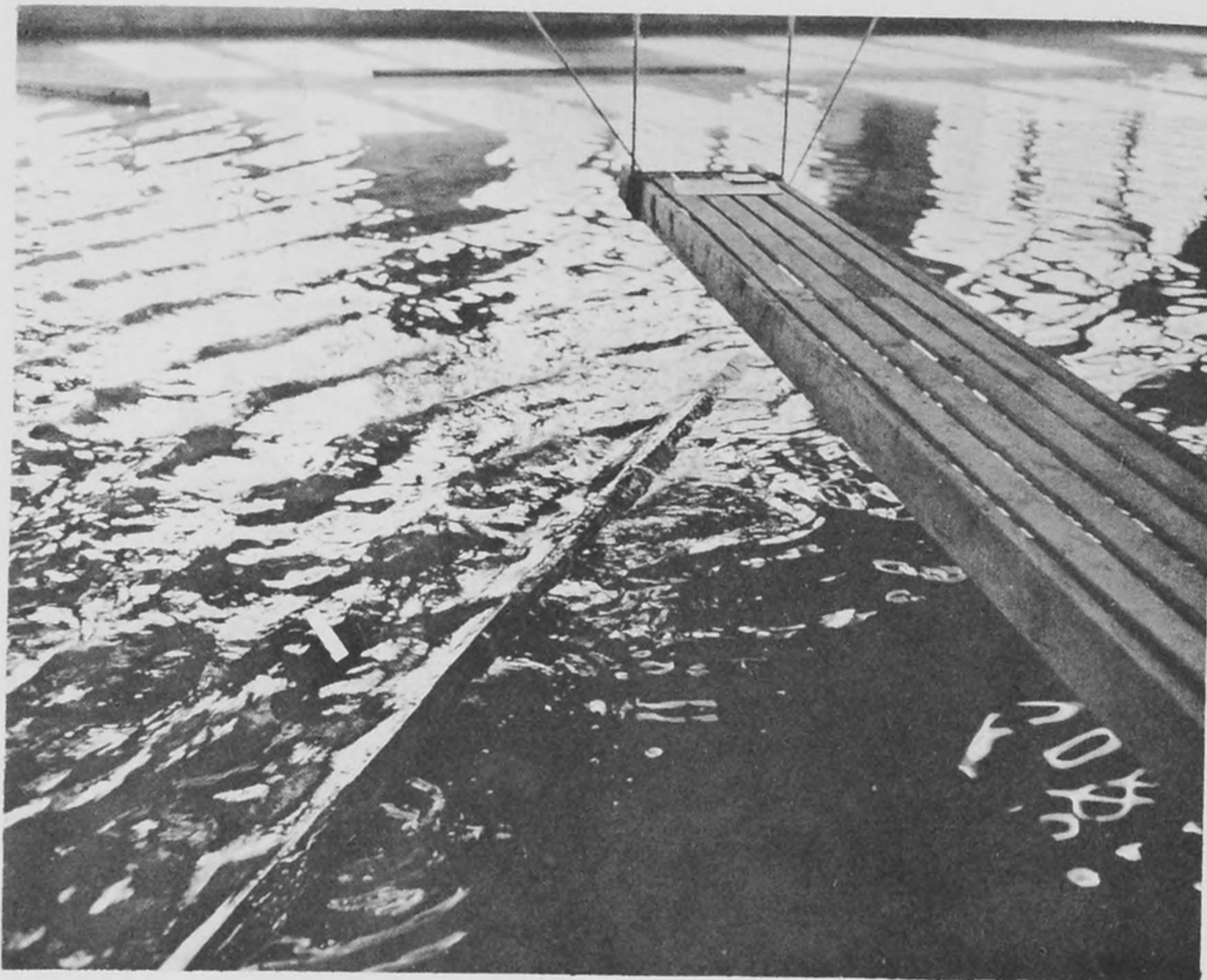


WAVE MACHINE

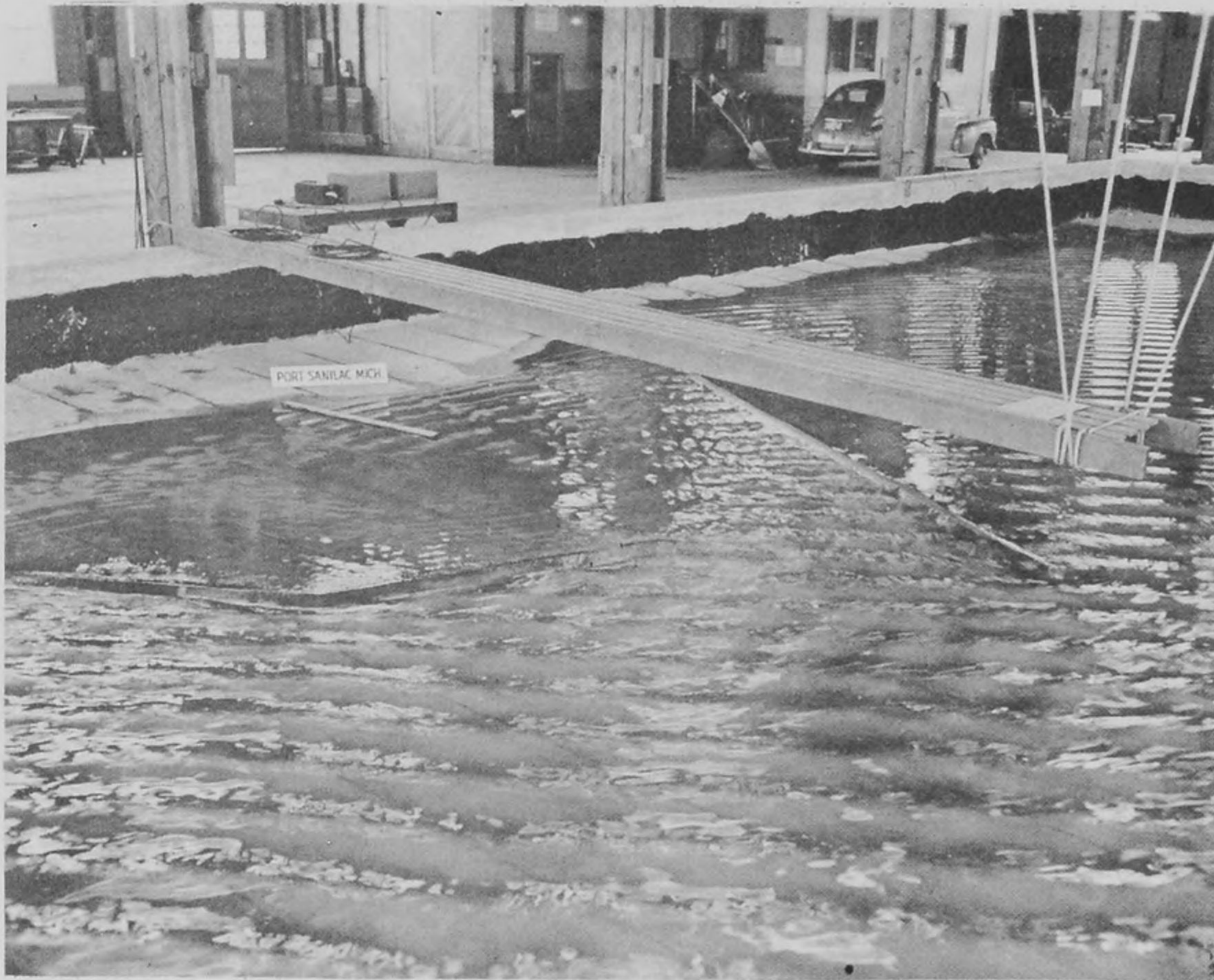
PORT SANILAC, MICHIGAN  
HARBOR MODEL  
PLATE 1.



PLAN 1 - E WIND



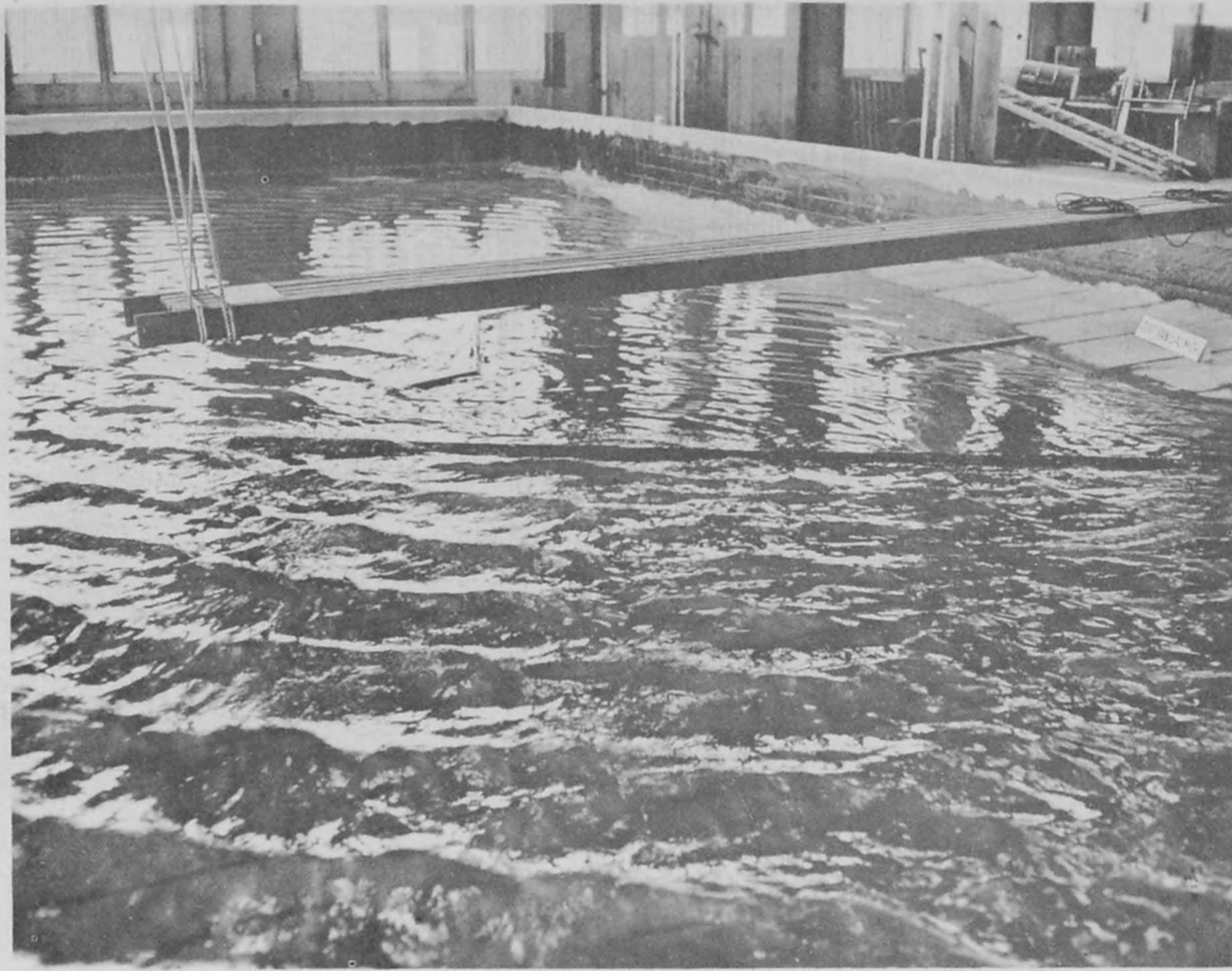
PLAN 1 - NNE WIND



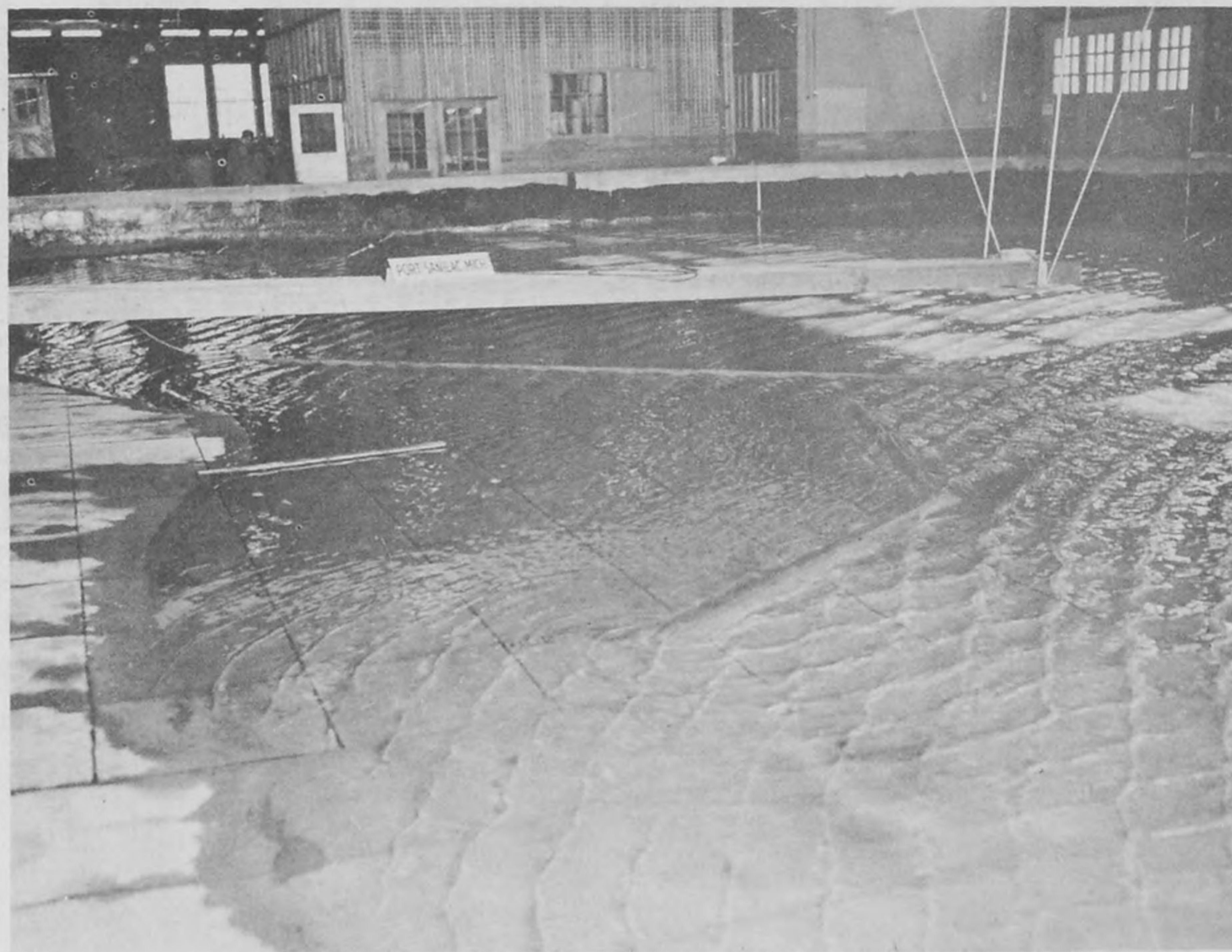
PLAN 2 - SSE WIND



PLAN 2 - SSE WIND



PLAN 2 - NNE WIND

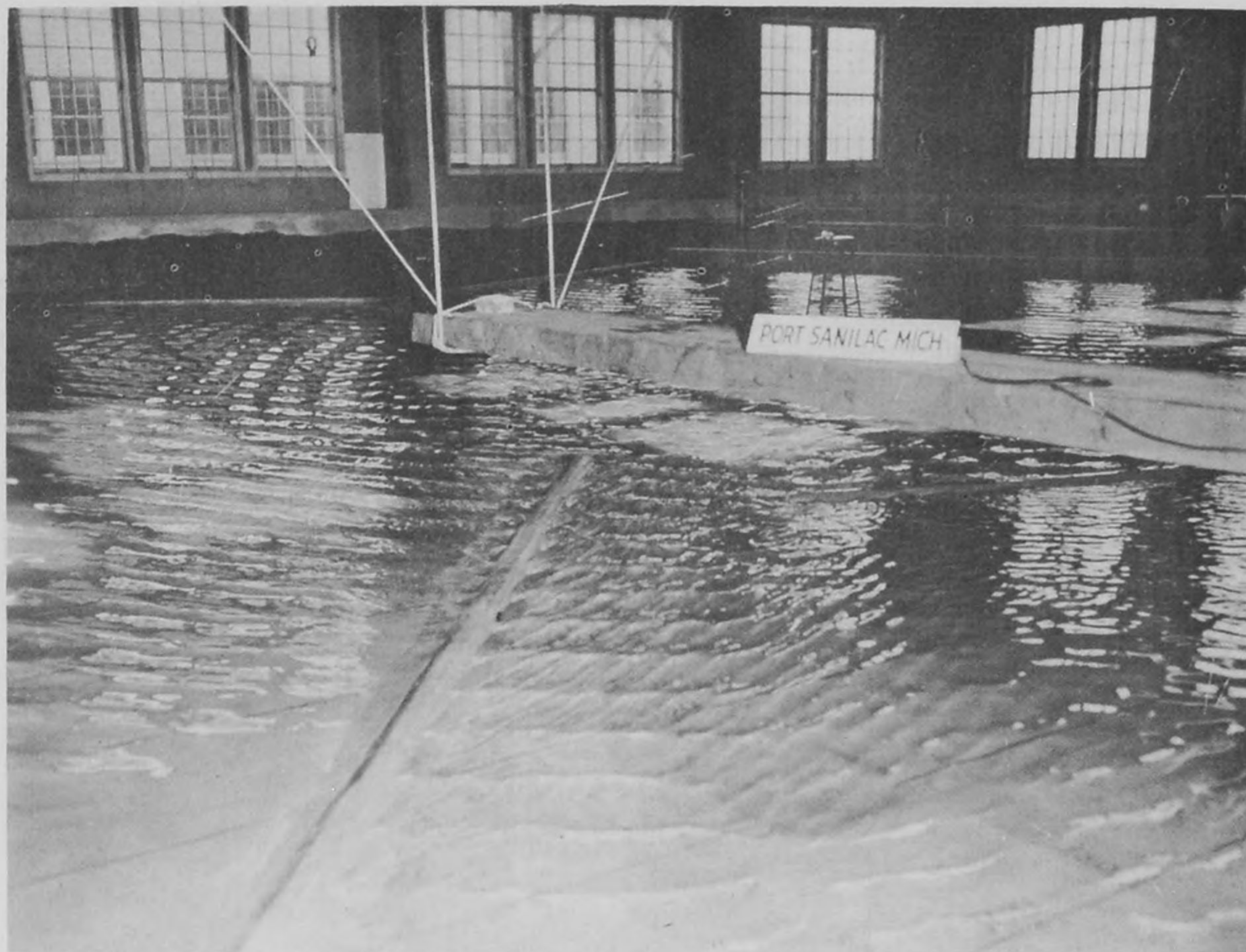


PLAN 3 - SSE WIND

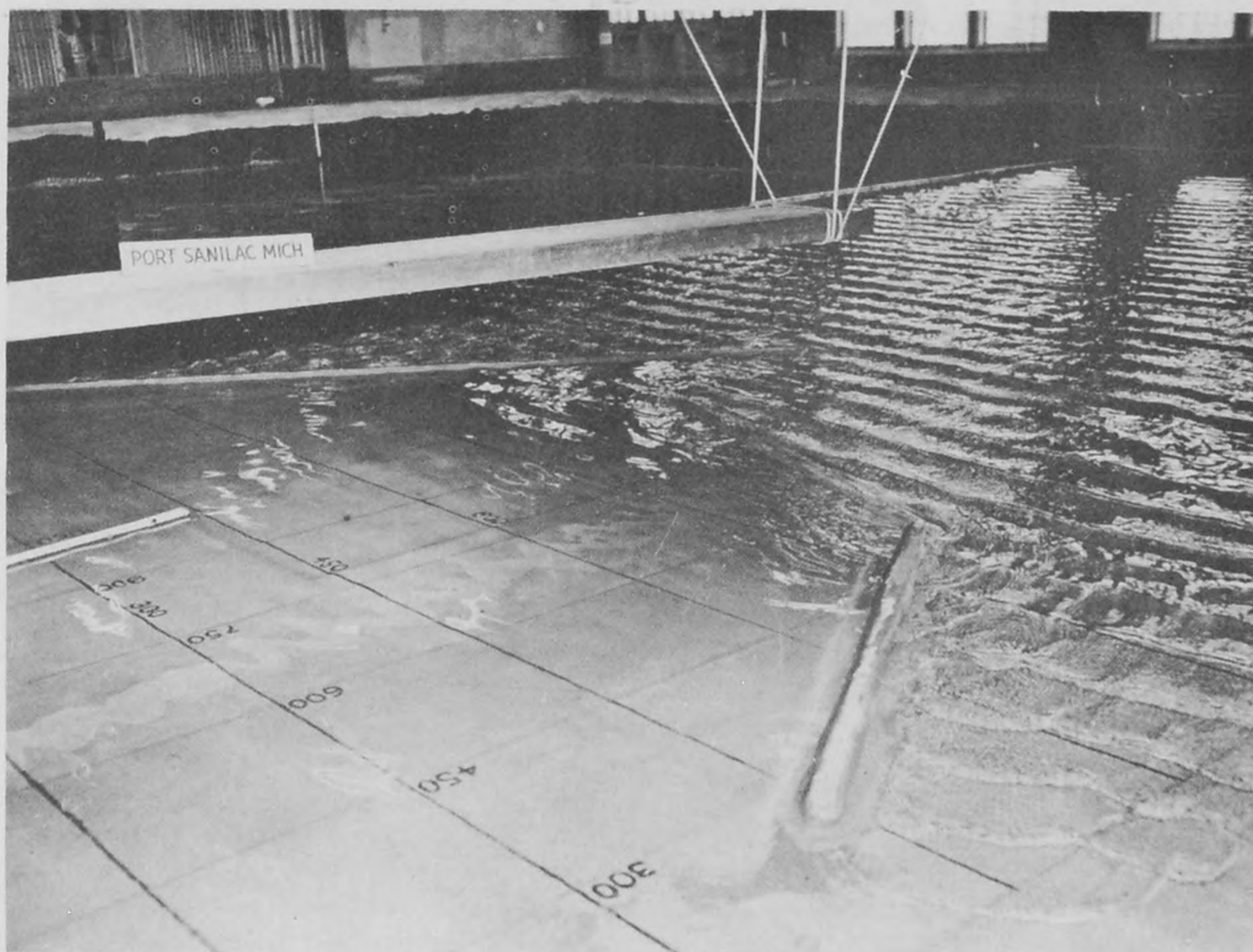
PORT SANILAC, MICHIGAN  
HARBOR MODEL  
PLATE 4.

PORT SANILAC, MICHIGAN  
HARBOR MODEL  
PLATE 4.





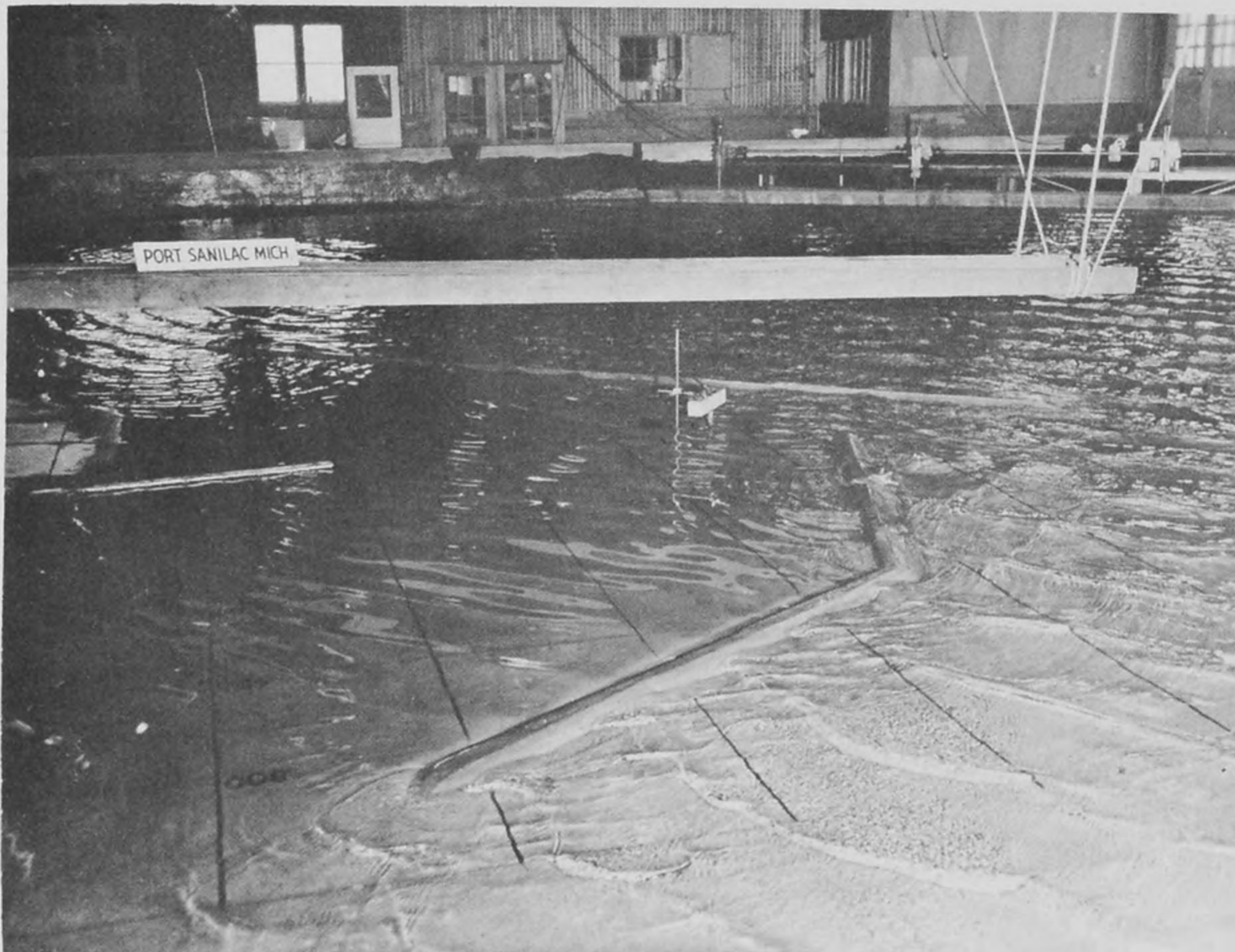
PLAN 3 - SSE WIND



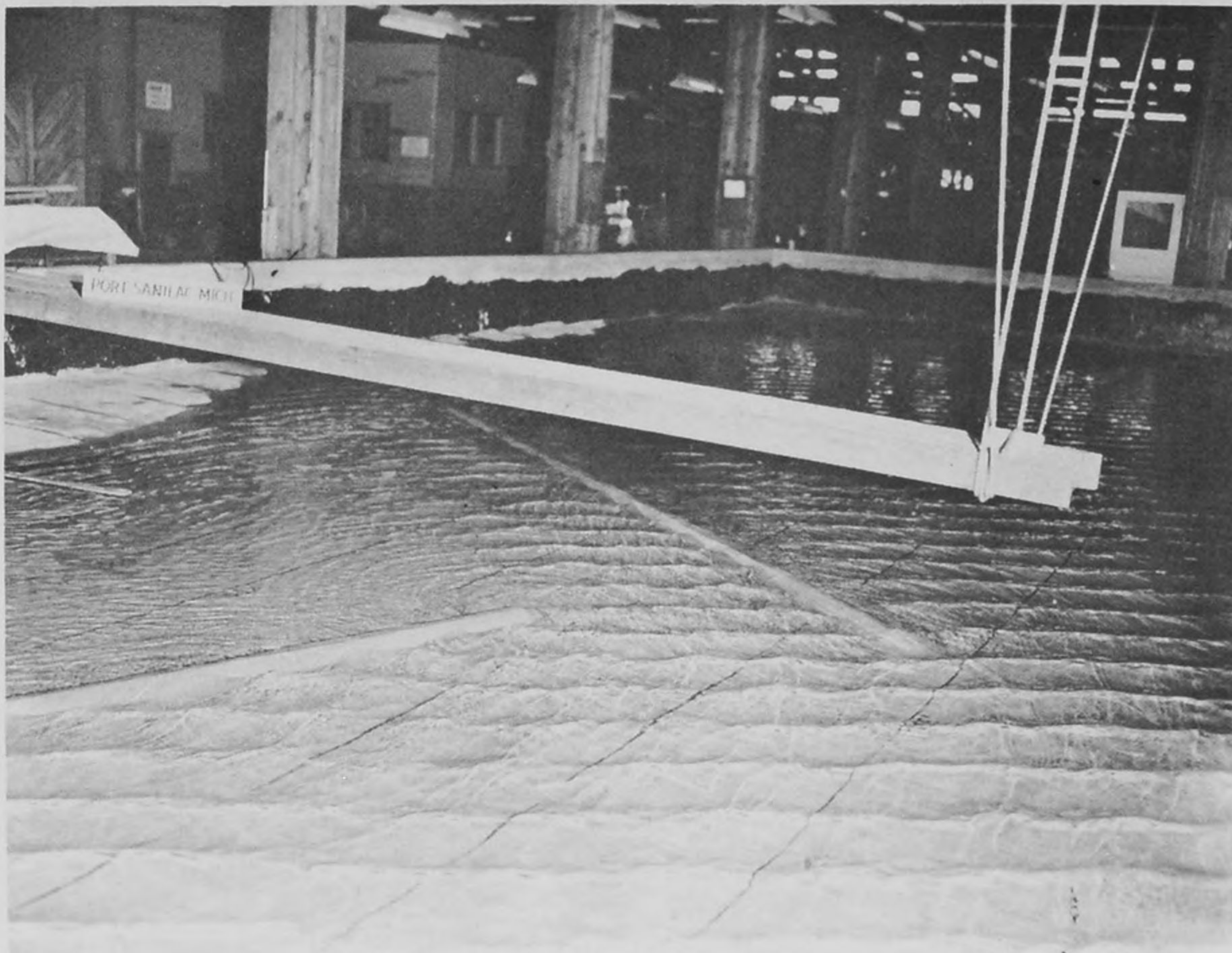
PLAN 3 - E WIND



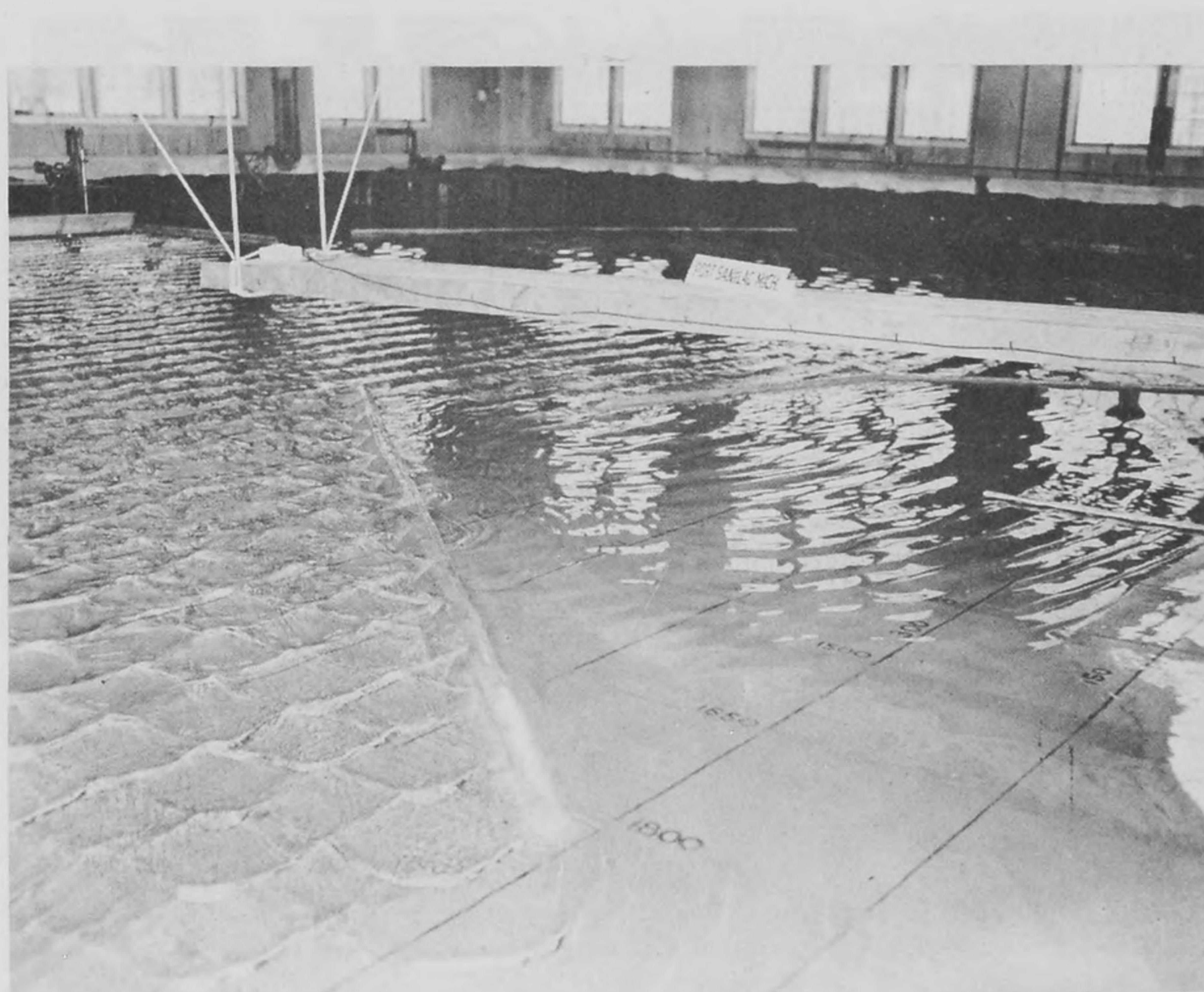
PLAN 3 - E WIND



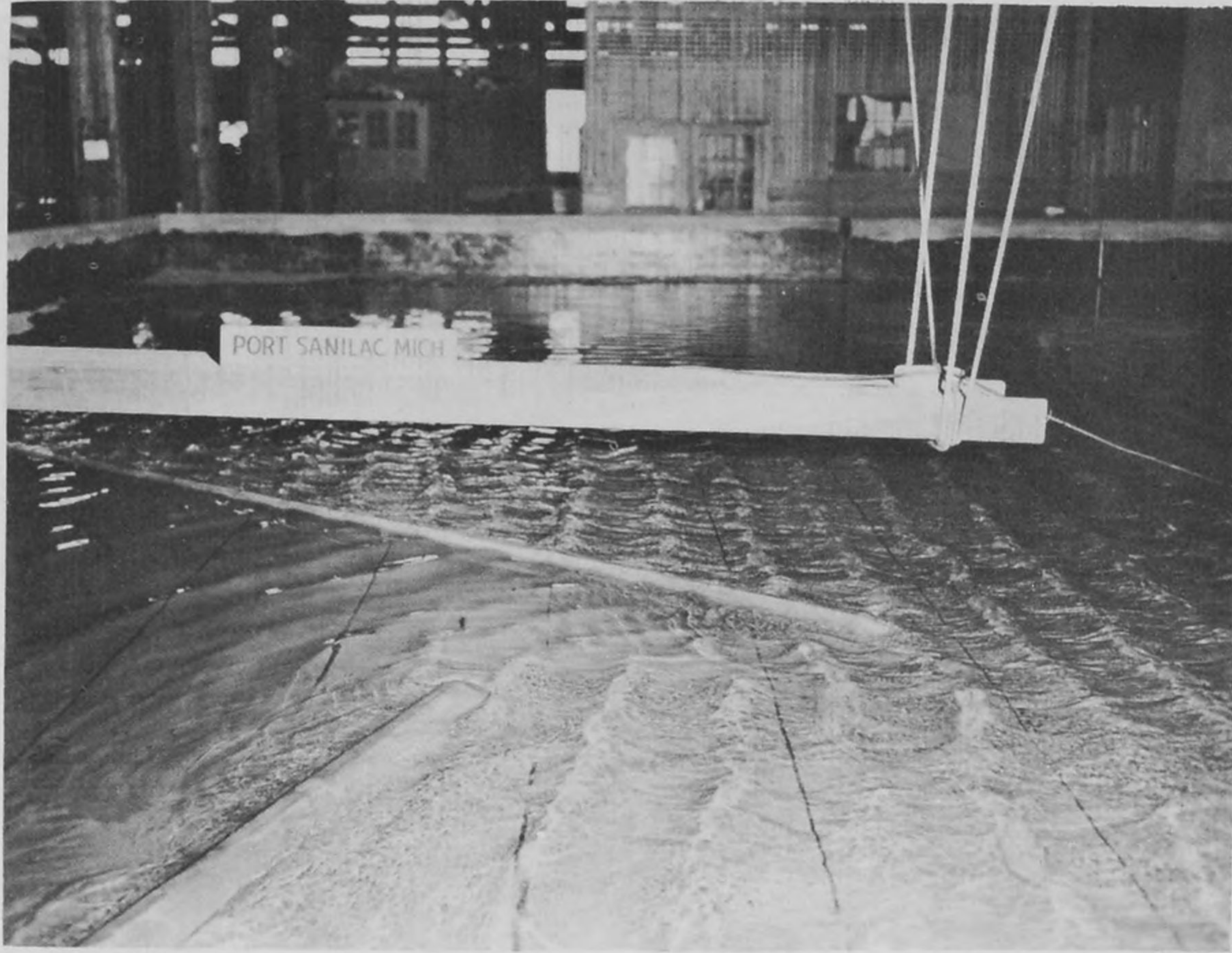
PLAN 3 - NNE WIND



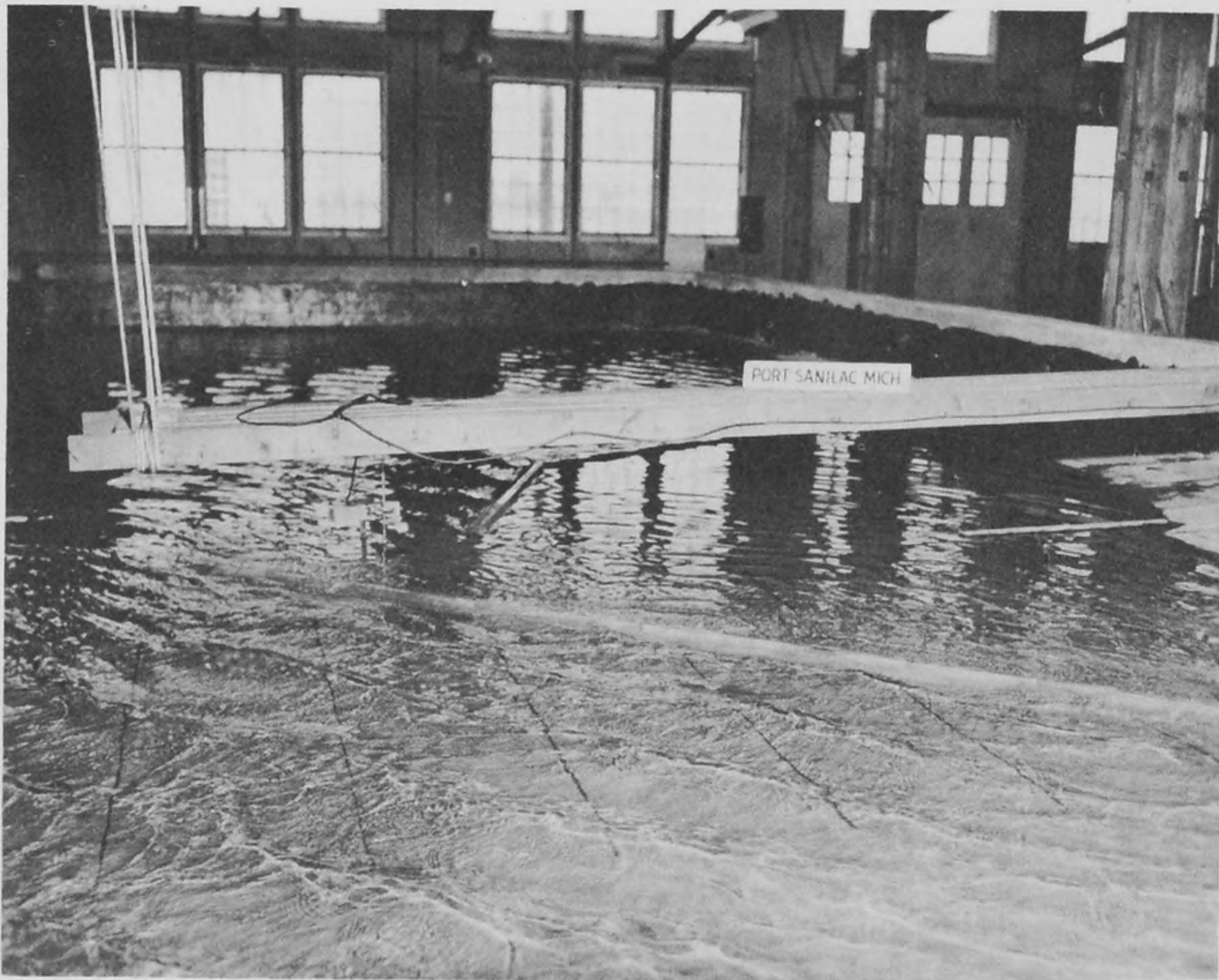
PLAN 4 - SSE WIND



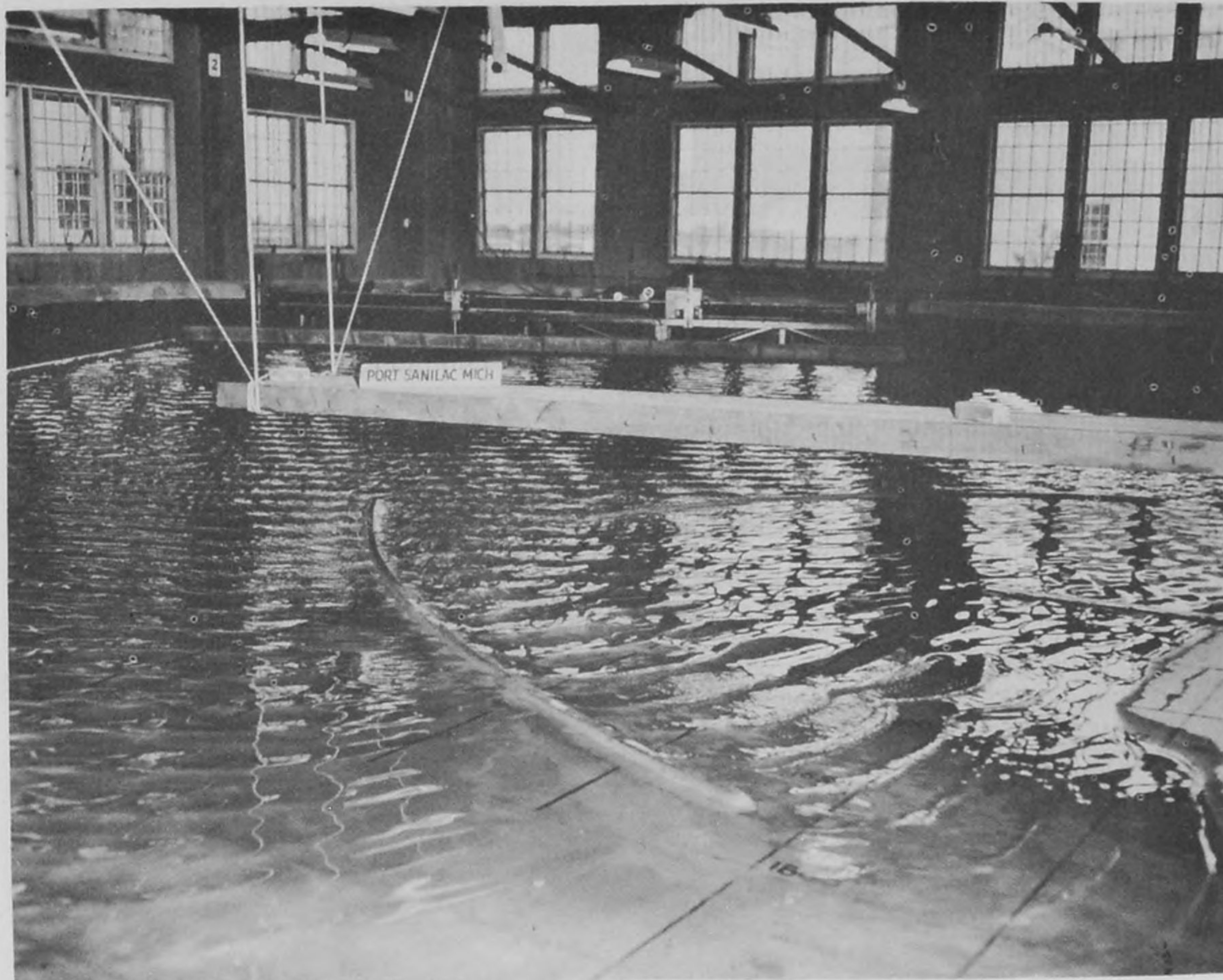
PLAN 4 - E WIND



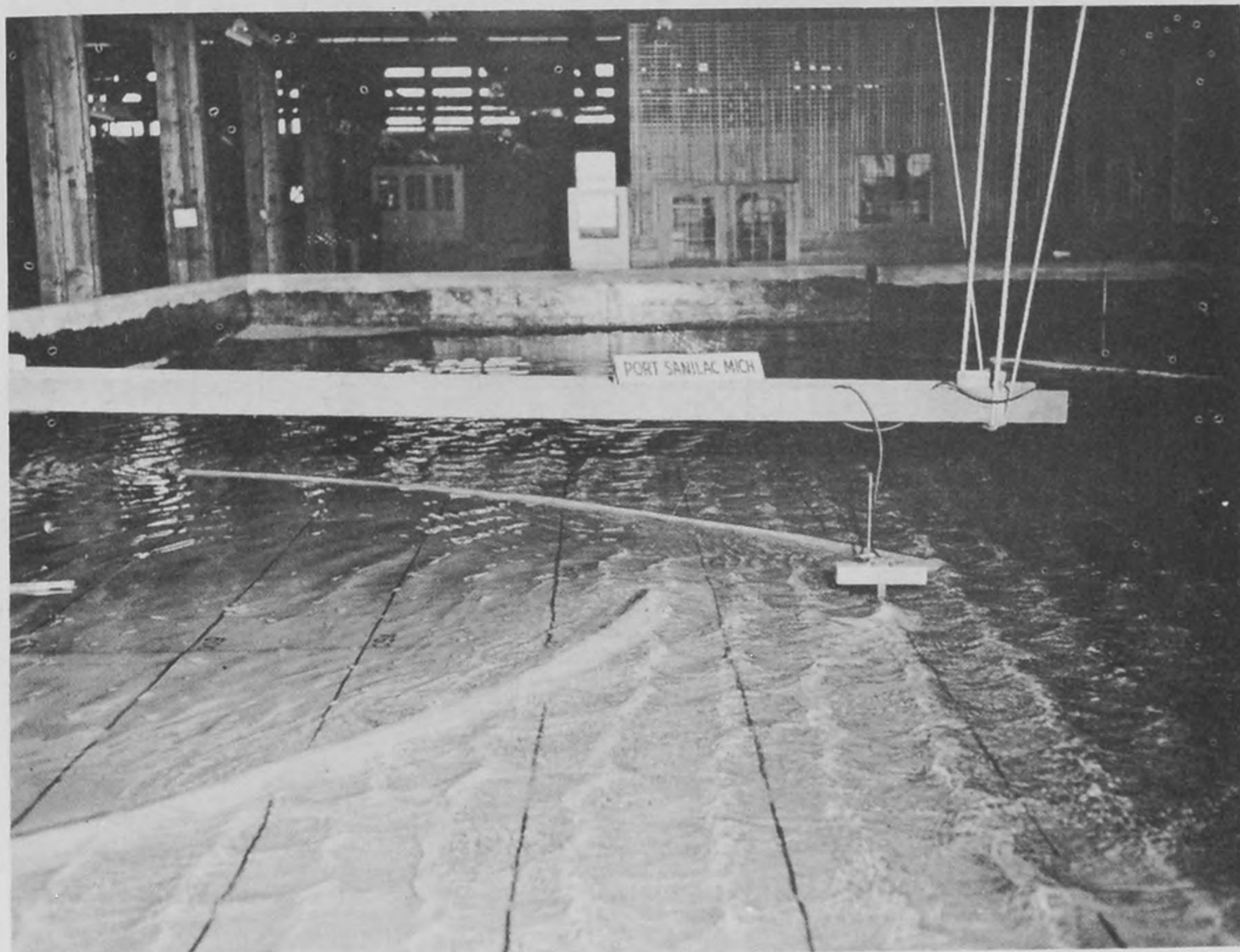
PLAN 4 - E WIND



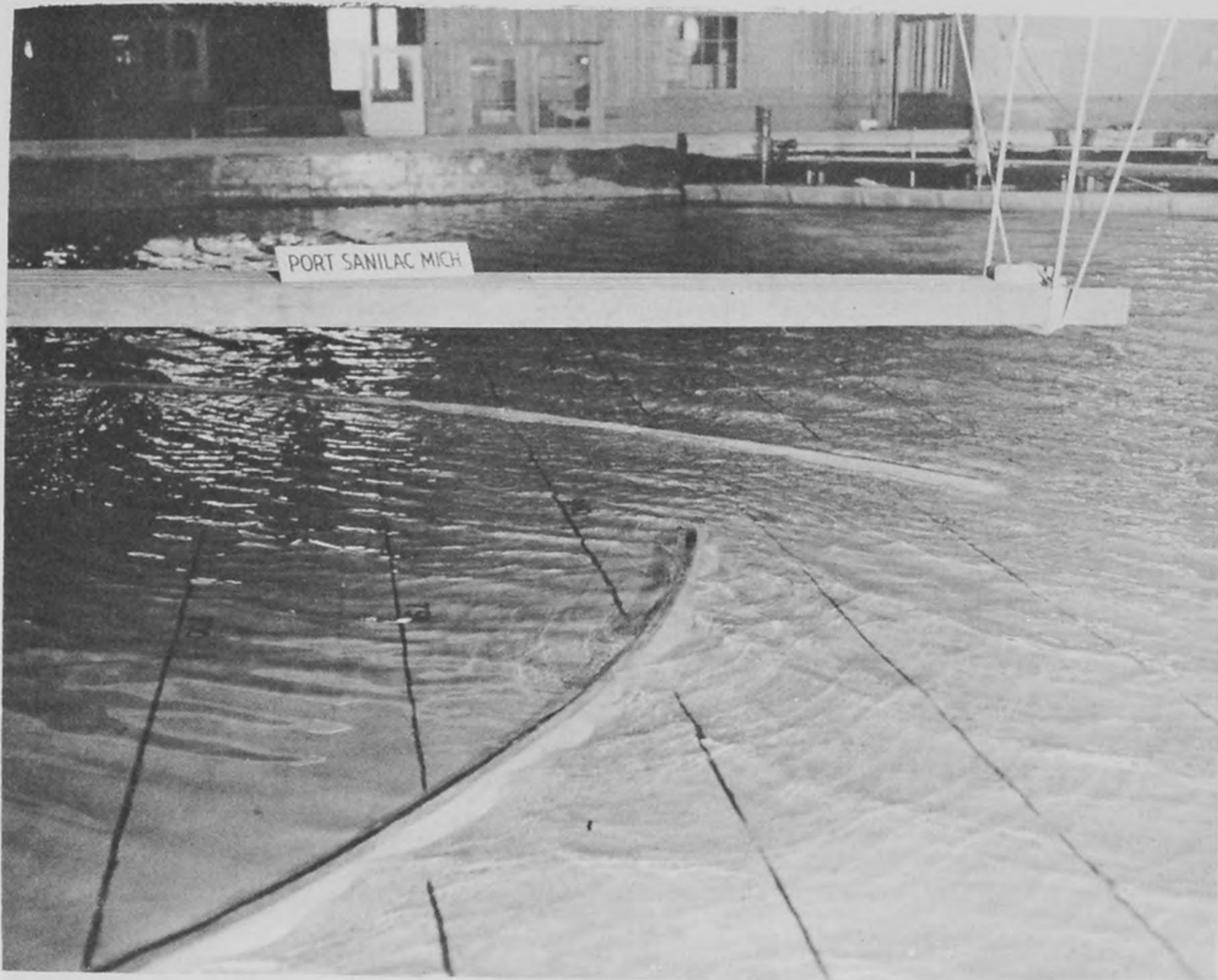
PLAN 4 - NNE WIND



PLAN 5 - SSE WIND



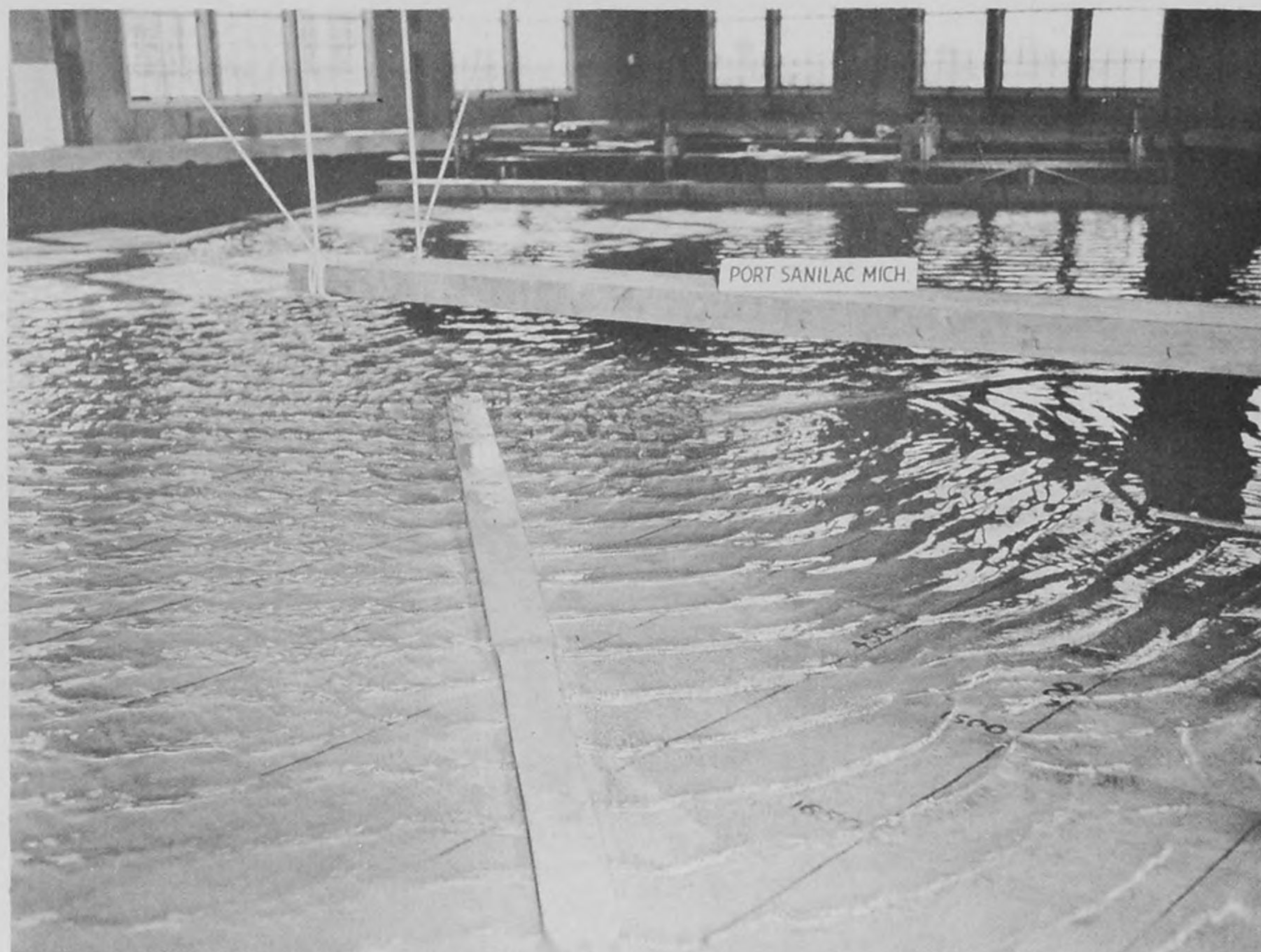
PLAN 5 - E WIND



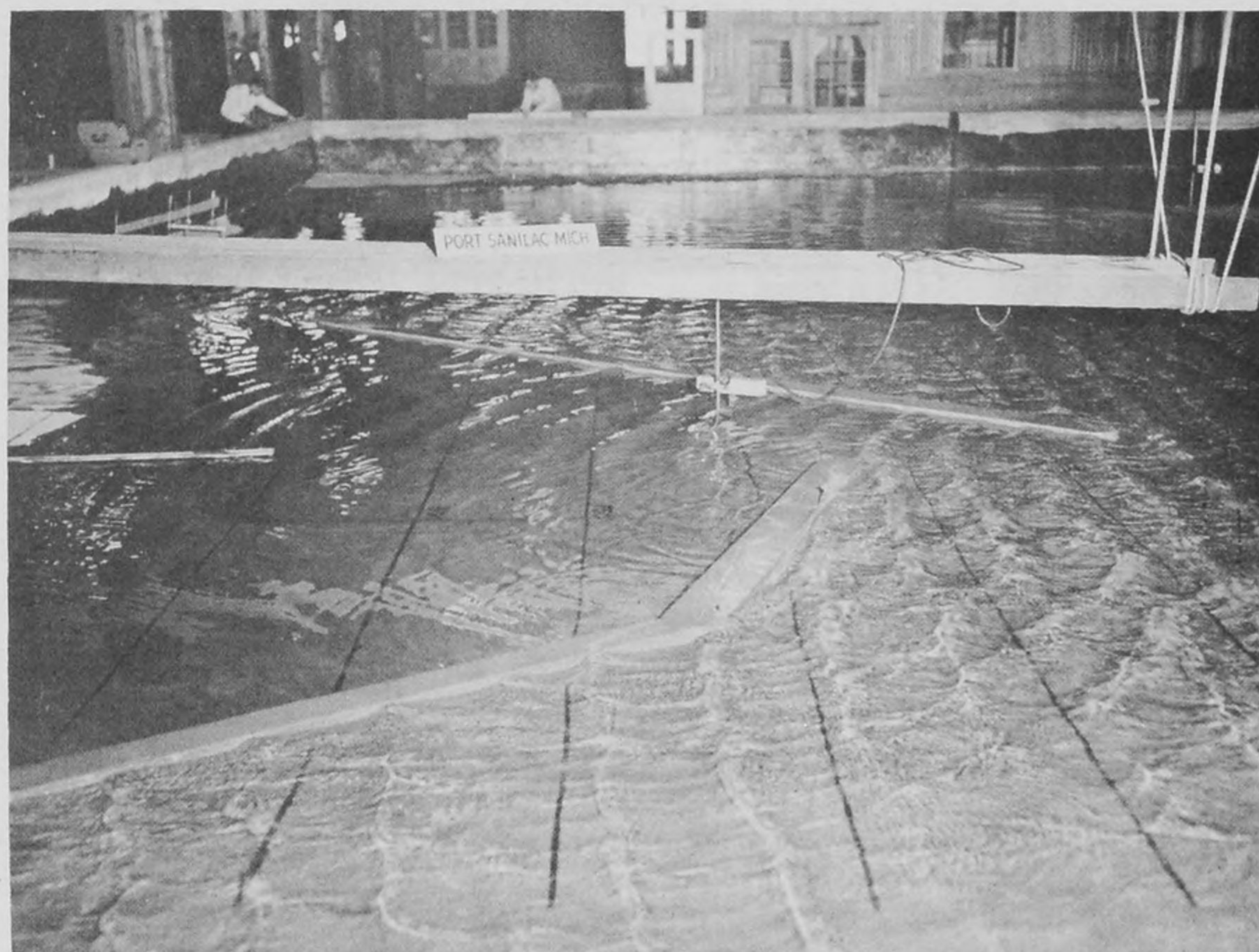
PLAN 5 - NNE WIND



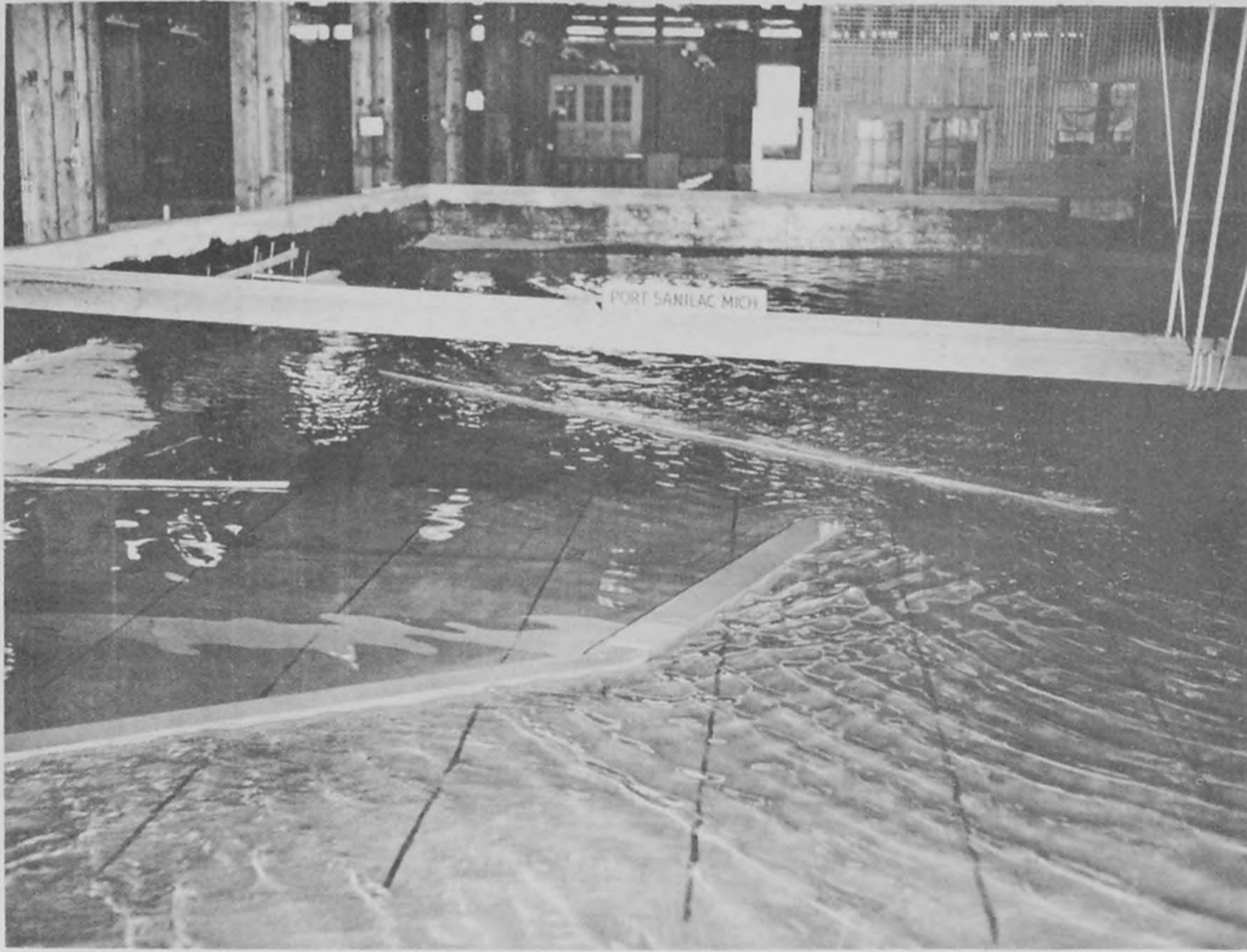
PLAN 6 - SSE WIND



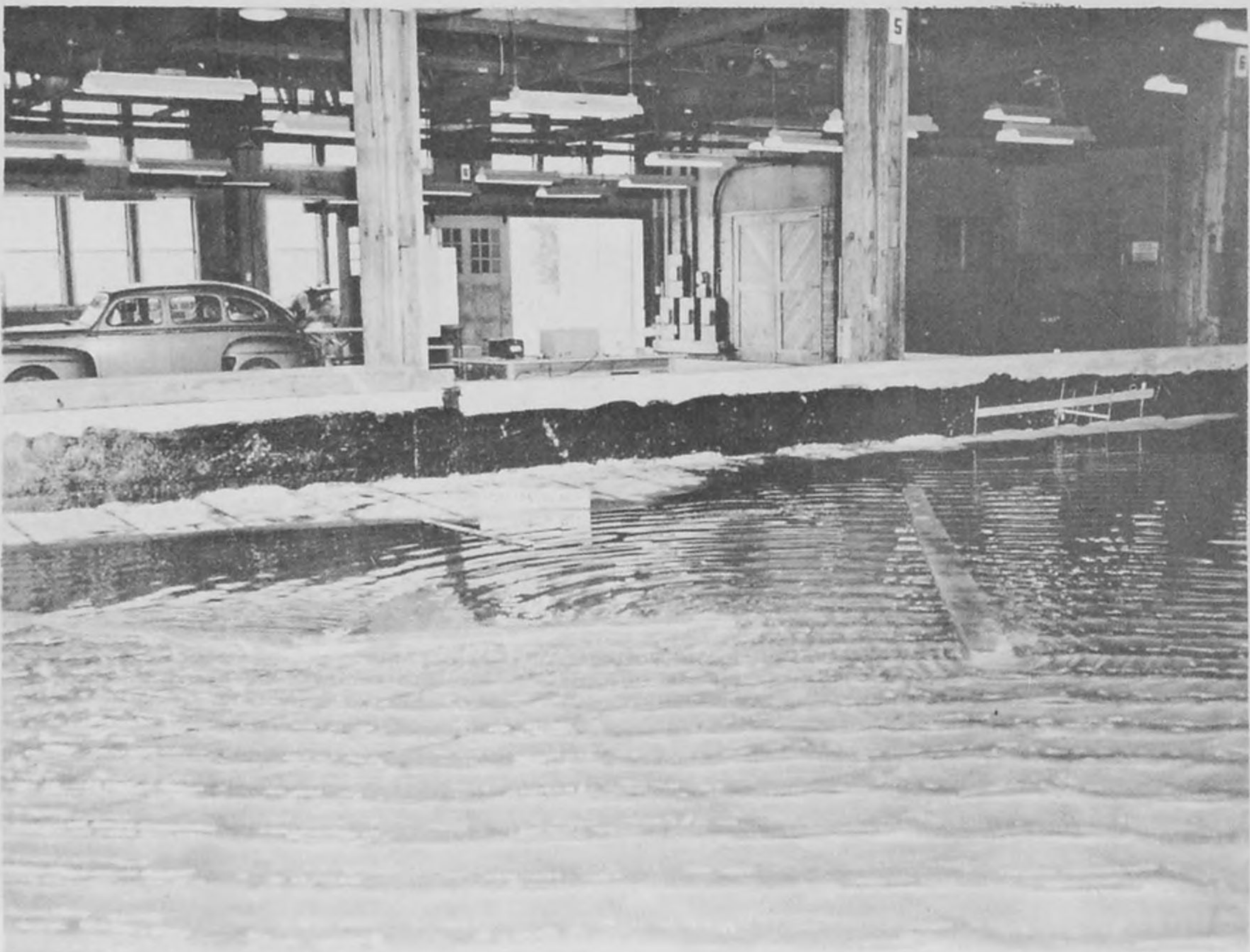
PLAN 6 - SSE WIND



PLAN 6 - E WIND



PLAN 6 - NNE WIND



PLAN 6 - SE WIND



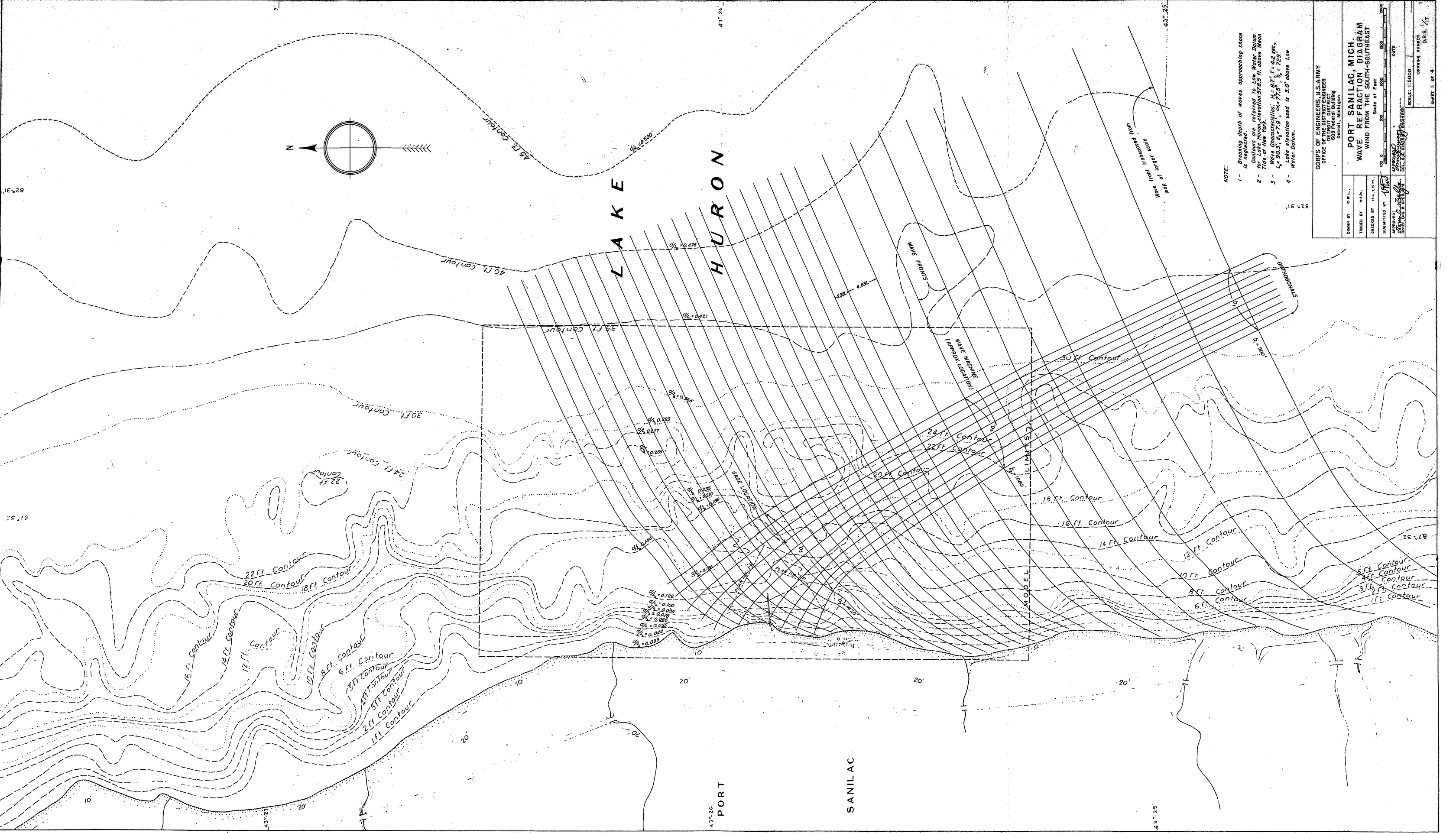
APPENDIX C

DRAWINGS SUPPLIED

BY

THE DETROIT DISTRICT,  
CORPS OF ENGINEERS, U. S. ARMY





NOTE:

- 1 - Breaking depth of waves approaching shore is neglected.
- 2 - Contours are referred to Low Water Datum.
- 3 - Tide of New York.
- 4 - Wave Characteristics:  $H_s = 8.7'$ ,  $T_s = 4.2$  sec.,  $L_s = 90.3'$ ,  $d_s = 7.9'$ ,  $c_s = 77.5'$ ,  $b_s = 72.9'$ .
- 5 - Lake elevation used is 3.0' above Low Water Datum.

CORPS OF ENGINEERS, U.S. ARMY  
 OFFICE OF THE DISTRICT ENGINEER  
 605 Independence Building  
 DETROIT, MICHIGAN

**PORT SANILAC, MICH.**  
**WAVE REFRACTION DIAGRAM**  
 WIND FROM THE SOUTH-SOUTHEAST

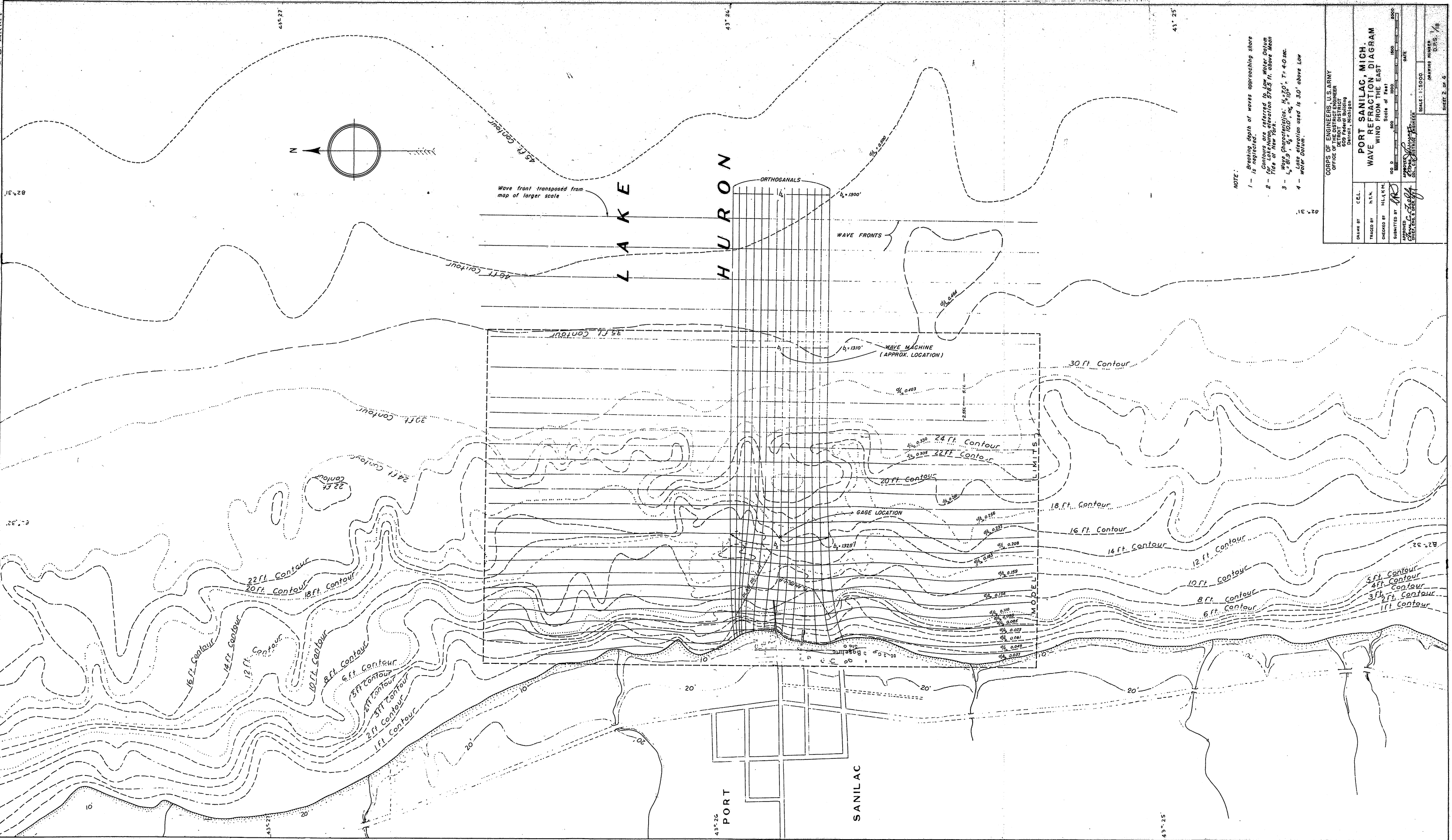
DRAWN BY	C.E.L.
CHECKED BY	H.H.H.
SUBMITTED BY	H.L.K.M.
APPROVED BY	[Signature]
DATE	
SCALE	1:5000
DRAWING NUMBER	
SHEET NUMBER	1 of 4
C.P.S.	1/7

Scale of Feet  
 0 100 200 300 400 500 600 700 800 900 1000 1500 2000



U.S. ARMY

CORPS OF ENGINEERS



NOTE:  
 1 - Breaking depth of waves approaching shore is neglected.  
 2 - Contours are referred to Low Water Datum Tide of New York Harbor.  
 3 - Wave Characteristics:  $H = 3.0'$ ,  $T = 4.0$  sec.  
 4 - Lake elevation used is 3.0' above Low Water Datum.

CORPS OF ENGINEERS U.S. ARMY  
 OFFICE OF THE DISTRICT ENGINEER  
 609 Federal Building  
 DETROIT, MICHIGAN

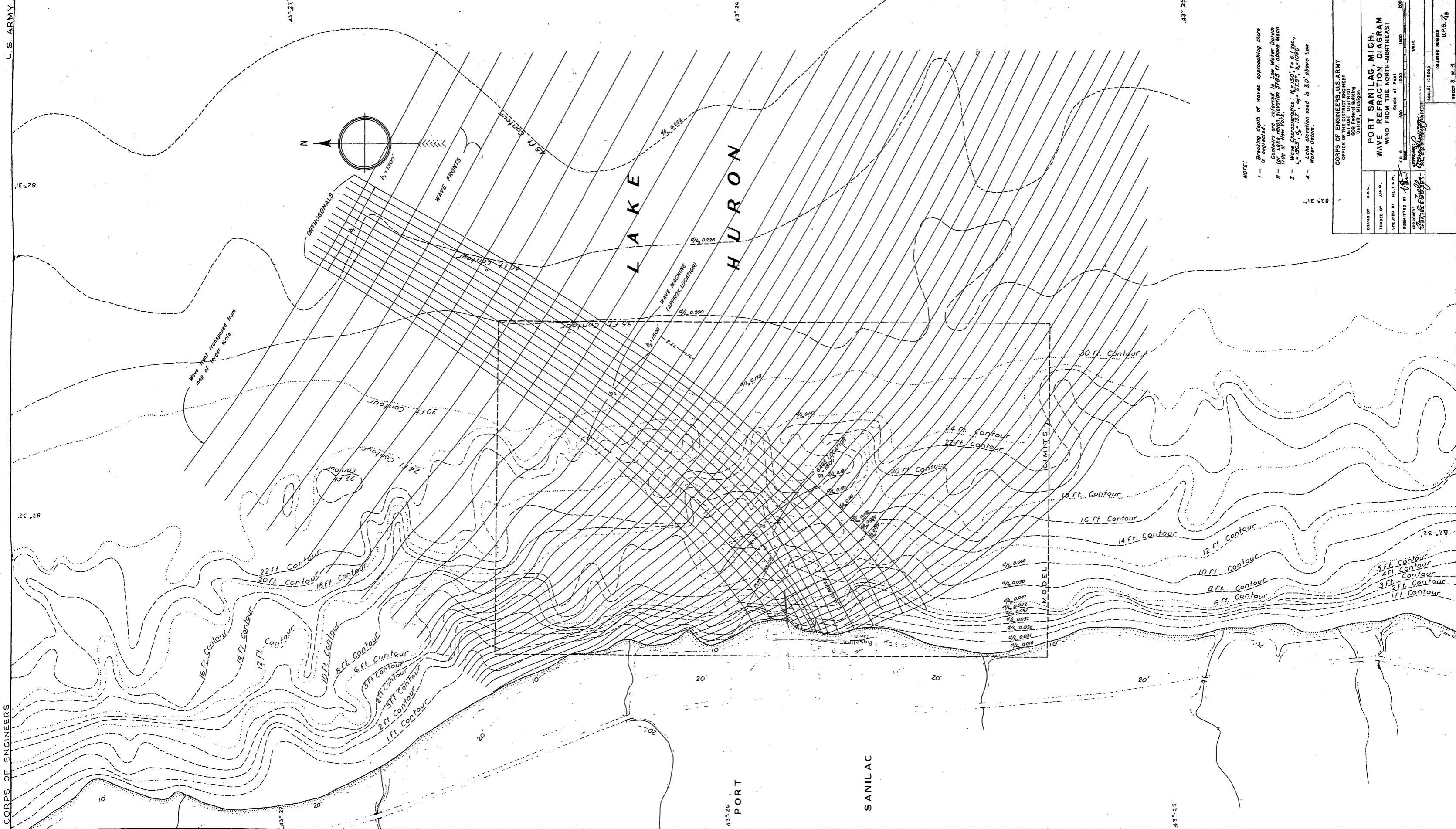
**PORT SANILAC, MICH.  
 WAVE REFRACTION DIAGRAM  
 WIND FROM THE EAST**

SCALE: 1:5000

DATE: \_\_\_\_\_  
 DRAWING NUMBER: O.P.S. 1/18  
 SHEET 2 OF 4

CELL	DATE
TRACED BY	H.L.K.
CHECKED BY	H.L.K.
SUBMITTED BY	[Signature]
APPROVED BY	[Signature]





CORPS OF ENGINEERS, U.S. ARMY  
 OFFICE OF THE DISTRICT ENGINEER  
 DETROIT DISTRICT  
 1000 W. WABASH AVENUE  
 DETROIT, MICHIGAN

**PORT SANILAC, MICH.**  
**WAVE REFRACTION DIAGRAM**  
 WIND FROM THE NORTH-NORTHEAST

SCALE: 1" = 500'

DATE: \_\_\_\_\_

SCALE: 1" = 500'

DRAWN BY: C.E.L.  
 TRACED BY: J.W.H.  
 CHECKED BY: A.L.A.K.H.  
 SUBMITTED BY: \_\_\_\_\_

APPROVED: \_\_\_\_\_

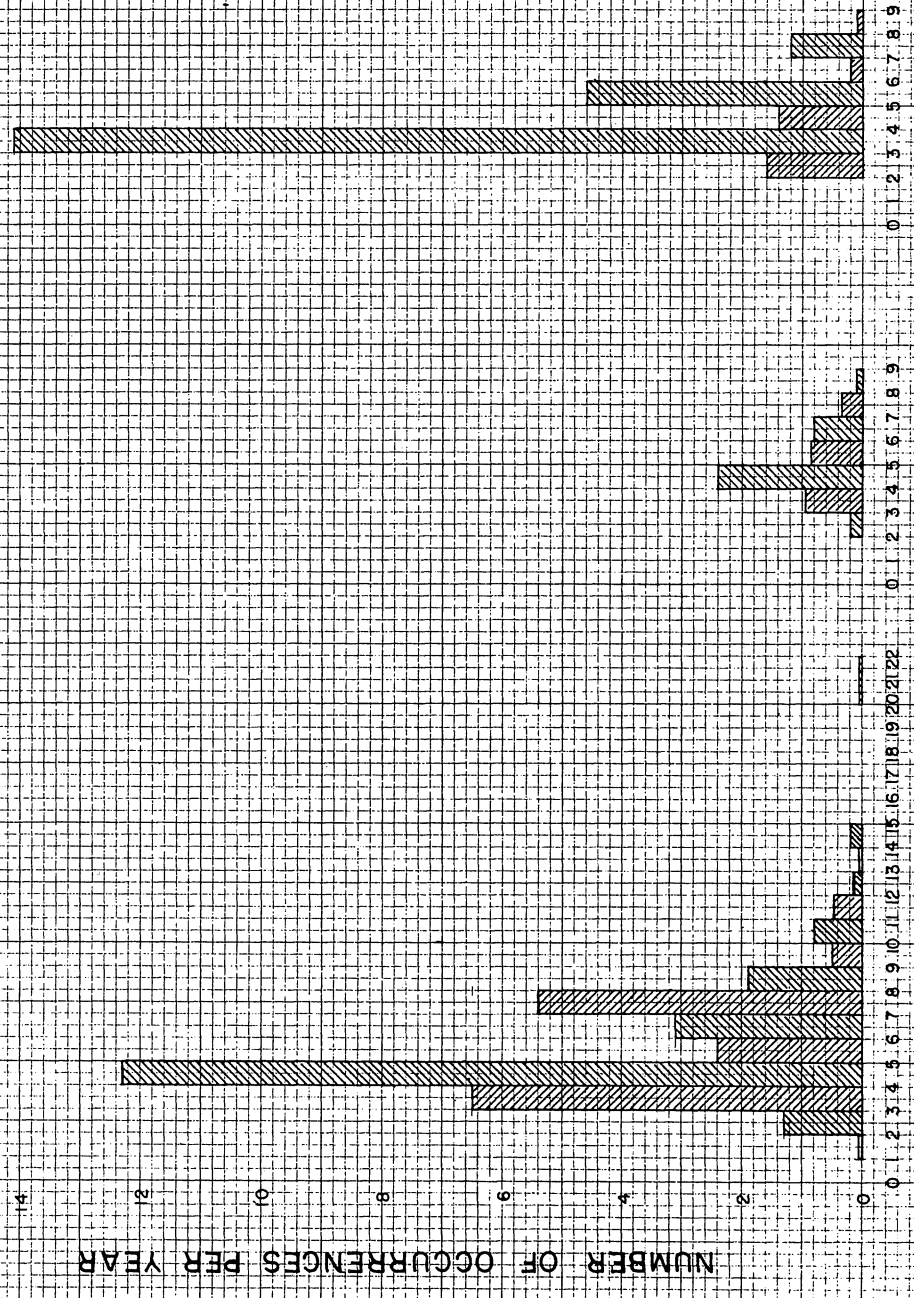
DRAWING NUMBER: D.P.S. 1/9

SHEET 3 OF 4





NORTH-NORTHEAST EAST SOUTH-SOUTHEAST



NOTE

- 1— The wave heights were computed from the winds in excess of 15 miles per hour that occurred during the 20 year period of 1905-1924, recorded at the U.S. Weather Bureau station at Port Huron, Mich.
- 2— The direction NNE contains winds from North to Northeast; East contains the winds from E.N.E. to E.S.E.; S.S.E. contains the winds from S.E. to South.
- 3— The graphs show the times per year that wind velocity and duration conditions occurred which would produce a maximum wave of the height shown.

PORT SANILAC, MICHIGAN  
WAVE FREQUENCY STUDY

UNIVERSITY OF MICHIGAN



3 9015 09911 4558





# AIIM SCANNER TEST CHART # 2

## Spectra

4 PT ABCDEFGHIJKLMNOPQRSTUVWXYZabcdefghijklmnopqrstuvwxyz;"/?0123456789  
 6 PT ABCDEFGHIJKLMNOPQRSTUVWXYZabcdefghijklmnopqrstuvwxyz;"/?0123456789  
 8 PT ABCDEFGHIJKLMNOPQRSTUVWXYZabcdefghijklmnopqrstuvwxyz;"/?0123456789  
 10 PT ABCDEFGHIJKLMNOPQRSTUVWXYZabcdefghijklmnopqrstuvwxyz;"/?0123456789

## Times Roman

4 PT ABCDEFGHIJKLMNOPQRSTUVWXYZabcdefghijklmnopqrstuvwxyz;"/?0123456789  
 6 PT ABCDEFGHIJKLMNOPQRSTUVWXYZabcdefghijklmnopqrstuvwxyz;"/?0123456789  
 8 PT ABCDEFGHIJKLMNOPQRSTUVWXYZabcdefghijklmnopqrstuvwxyz;"/?0123456789  
 10 PT ABCDEFGHIJKLMNOPQRSTUVWXYZabcdefghijklmnopqrstuvwxyz;"/?0123456789

## Century Schoolbook Bold

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## News Gothic Bold Reversed

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 6 PT ABCDEFGHIJKLMNOPQRSTUVWXYZabcdefghijklmnopqrstuvwxyz;"/?0123456789  
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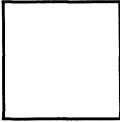
## Bodoni Italic

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## Greek and Math Symbols

4 PT ΑΒΓΔΕΕΘΗΙΚΑΜΝΟΠΦΡΣΤΥΩΧΨΖαβγδεξθηικλμνοπφρστνωχψζ≥≠",./≤±=≠' > < > < > < ≡  
 6 PT ΑΒΓΔΕΕΘΗΙΚΑΜΝΟΠΦΡΣΤΥΩΧΨΖαβγδεξθηικλμνοπφρστνωχψζ≥≠",./≤±=≠' > < > < > < ≡  
 8 PT ΑΒΓΔΕΕΘΗΙΚΑΜΝΟΠΦΡΣΤΥΩΧΨΖαβγδεξθηικλμνοπφρστνωχψζ≥≠",./≤±=≠' > < > < > < ≡  
 10 PT ΑΒΓΔΕΕΘΗΙΚΑΜΝΟΠΦΡΣΤΥΩΧΨΖαβγδεξθηικλμνοπφρστνωχψζ≥≠",./≤±=≠' > < > < > < ≡

White



Black



Isolated Characters

e	m	1	2	3	a
4	5	6	7	o	-
8	9	0	h	l	B

## MESH HALFTONE WEDGES

