

Computing stress intensity factors for curvilinear cracks

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SUMMARY

The use of the interaction integral to compute stress intensity factors around a crack tip requires selecting an auxiliary field and a material variation field. We formulate a family of these fields accounting for the curvilinear nature of cracks that, in conjunction with a discrete formulation of the interaction integral, yield optimally convergent stress intensity factors. In particular, we formulate three pairs of auxiliary and material variation fields chosen to yield a simple expression of the interaction integral for different classes of problems. The formulation accounts for crack face tractions and body forces. Distinct features of the fields are their ease of construction and implementation. The resulting stress intensity factors are observed converging at a rate that doubles that of the stress field. We provide a sketch of the theoretical justification for the observed convergence rates and discuss issues such as quadratures and domain approximations needed to attain such convergent behavior. Through two representative examples, a circular arc crack and a loaded power function crack, we illustrate the convergence rates of the computed stress intensity factors. The numerical results also show the independence of the method from the size of the domain of integration. Copyright © 2015 John Wiley & Sons, Ltd.

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1. INTRODUCTION

The stress field near the tip of a loaded crack is singular under the assumption of linear elastic fracture mechanics. The coefficients of the asymptotic stress field, known as the *stress intensity factors*, play a key role in characterizing the magnitude of the load applied to the crack and in predicting its propagation.

Given the stress singularity and the poor accuracy in pointwise evaluation of the stress field, it is often impossible to extract the stress intensity factors directly from numerical solutions, unless a higher order method to compute the elastic field is adopted such as those proposed in Liu *et al.* [1], Shen and Lew [2], and Chiaramonte *et al.* [3].

As a result, path and domain integral methods to extract the stress intensity factors have been created precisely to circumvent this limitation. A method of this kind typically formulates the expression of the stress intensity factors as functionals of the solution, thus enjoying a higher order of convergence than that of the elastic stress field itself. Predominant methods of this kind have been constructed based on the *J*-integral [4] and the interaction integral [5].

In the context of linear elastic fracture mechanics, the *J*-integral is identified with the system's elastic energy release rate, the elastic energy that would be released per unit length of crack

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extension in the tangential direction. This integral and related ones for the computation of fracture-mechanics-related quantities have been elaborated by Eshelby [6], Rice [4], Freund [7], and many others. Shih *et al.* [8] derived the expression for the energy release rate of a thermally stressed body in the presence of crack face traction and body force. A general treatment of such conservation integrals including those expressed in a form of domain integrals can be found in Moran and Shih [9, 10]. The domain form is better suited and more accurate for numerical computation. Nevertheless, in the case of mixed-mode loading, J is a quadratic function of all three stress intensity factors [11]; therefore, additional integrals are needed to determine the three quantities individually, for example, as performed by Chang and Wu [12] for non-planar curved cracks.

In contrast, the interaction integral or the interaction energy integral is able to yield the three stress intensity factors separately. This method is based on the J -integral by superposing the elastic field of the loaded body and an *auxiliary* field with known stress intensity factors. The auxiliary field does not need to satisfy the elasticity equations but must resemble the asymptotic solution of a cracked elastic body corresponding to one of the three loading modes (e.g., plane-strain mode I or mode II, or anti-plane mode III). Therefore, the auxiliary field, for straight cracks, is normally chosen to be the asymptotic solution, as found in [13, 14]. Doing so readily yields the stress intensity factor of the actual field for the chosen mode. Along with the auxiliary field, the interaction integral requires the construction of a vector field, named the *material variation* field, which indicates the velocity (variation) of points in the reference configuration as it is deformed into a domain with a longer crack. Under mild conditions, the value of the interaction integral does not depend on this choice, but a good choice of material variation can simplify computations. For example, the interaction integral is computed by integrating over the support of the material variation field, so it is convenient to choose material variation fields with small and compact support. While developing auxiliary and material variation fields for straight cracks (planar cracks in three dimensions) is an amenable task, doing so for curvilinear cracks (non-planar cracks in three dimensions) poses several challenges. In the following paragraphs, we provide a short review of the effort related to the computation of stress intensity factors with the use of the interaction integral.

Earlier methods to compute the stress intensity factors with the interaction integral involved path integrals such as those in Stern *et al.* [5] and Yau *et al.* [15]. The method was later generalized to a straight-front crack in three-dimensions by Nakamura and Parks [16] and Nakamura [17]. A curved crack front introduces additional terms, because the popular plane-strain modes I and II auxiliary fields no longer satisfy the compatibility and the equilibrium conditions. These additional terms were accounted for by Nahta and Moran [18] for the axisymmetric case and by Gosz *et al.* [19] for general planar cracks in three dimensions. Kim *et al.* [20] adopted auxiliary fields corresponding to penny-shaped cracks and conventional plane-strain and anti-plane ones. An alternative approach was given by Daimon and Okada [21] who adopted a compatible auxiliary field and accounted for its lack of equilibrium by superposing a numerically computed displacement field with the finite element method. A study of the effect of omitting some terms accounting for the curved front is given by Walters *et al.* [22].

More recently, the method of [19] was adapted for non-planar cracks in Gosz and Moran [23], the latter of which is arguably a milestone in the development of domain integral methods to extract stress intensity factors from curvilinear cracks in two dimensions and non-planar cracks in three dimensions. In [23], the method constructs the auxiliary fields through the use of curvilinear coordinates and their corresponding covariant basis to account for the crack curvature. This procedure constructs the auxiliary fields by juxtaposing the components of the stress fields of [13] with the described basis. In [24], Sukumar *et al.* implemented the method of [23] in combination with an extended finite element method in a three-dimensional setting [25] and a fast-marching method. The main drawback of the curvilinear coordinates of [23] is the need to perform boundary integrals over both the real crack surfaces and a pair of fictitious crack surfaces. In [26], González-Albuixech *et al.* proposed another curvilinear coordinate system that can eliminate the integration on the fictitious crack surfaces and in this way can facilitate the computation.

In [26, 27], González-Albuixech *et al.* studied the properties of the aforementioned methods in two and three dimensions, respectively, but not with all terms arising from the derivation of the interaction integral. Such omission of terms may have contributed to the observed slow convergence

and occasional divergence. This observation confirms the statement of [23] that all terms arising from the lack of compatibility and equilibrium of the auxiliary field have to be taken into account.

The domain version of the interaction integral has also been generalized to functionally graded materials [28, 29].

In this paper, we present a suite of auxiliary fields and material variations fields. By pairing two constructs of material variation fields and two constructs of auxiliary fields, we create two kinds of interaction integral suitable for curvilinear cracks and for situations in which body forces and crack face tractions are present. One kind of interaction integral is suited for applications where crack faces are loaded (e.g., hydraulic fracturing), and the other one is best suited for applications where body forces are non-zero (e.g., thermally loaded materials). Moreover, no fictitious crack face is needed, a major simplification to the predominant method in the literature.

One of the two choices of the material variation fields has a constant direction pointing to the direction of the crack growth, a straightforward choice adopted by most authors, and the other has a direction that is tangential to the crack near the crack tip, similar to that proposed in [25]. For a curved crack, this second choice necessarily coincides with the first one only at the crack tip. A key advantage of the material variation fields we introduce here is their ease of construction that is reflected in their straightforward implementation in computer codes. Moreover, in contrast to many of the existing constructions, the magnitude of the material variation fields is mesh independent. This mesh independence contributes to the observed optimal rate of convergence.

The two auxiliary fields are constructed from the well-known asymptotic solutions of a straight crack. Both fields respect the discontinuity introduced by the crack, thus avoiding the evaluation of integrals over fictitious crack faces as the method in [23] does. One of the auxiliary fields is obtained by 'extending' the asymptotic solutions past the range $[-\pi, \pi]$ and hence satisfies equilibrium, compatibility, and the constitutive relation. The resulting interaction integral expression then yields a term on the crack faces, even in the absence of crack face traction. The second auxiliary field is an incompatible strain field. It is obtained by first mapping a straight crack to the curved crack near the crack tip and by using this map to push forward the strain field of the straight-crack asymptotic solution. Then, by suitably rotating the strain tensor at each point, we obtain an auxiliary strain field that is traction-free at the curved crack faces. This is useful for problems in which crack faces are traction-free. In fact, if this auxiliary field is used in combination with the tangential material variation field, the crack face integral vanishes, resulting in a significantly simplified expression for the interaction integral.

We showcase the convergence of the stress intensity factors obtained with the proposed fields for a set of representative examples computed with two different finite element methods. In all cases, the stress intensity factors converge with a rate that doubles the rate of convergence of the strains. We also numerically demonstrate the independence of the computed stress intensity factors from the chosen support for the material variation field. Although the numerical examples adopt finite element methods to obtain an approximate solution to the elasticity problem, the numerical implementation of the interaction integral with the new fields is general and can be used in conjunction with any numerical method for the solution of the governing equations (e.g., finite difference, finite volume, boundary integral equations, isogeometric analysis, and meshless methods).

The paper is organized as follows. We first state the problem that we seek to solve in Section 2. We then proceed in Section 3 to present the interaction integral with the description of the new material variation and auxiliary fields. In the same section, we justify that the proposed forms of the interaction integral are well defined. A numerical approximation of the interaction integral is presented in Section 4 with remarks on its expected convergence. The last part of Section 4 provides a step-by-step recapitulation of the method suited for the reader interested in a concise presentation. In Section 5, we verify the computation of the stress intensity factors against analytical solutions for two problems: a circular arc crack and a power function crack. Throughout the paper, we included sections titled '*Justification*' that contain sketches of proofs for some of the assertions we make, and they are not essential for the description of the methods in this paper.

2. PROBLEM STATEMENT

We present next the problem statement that consists of the evaluation of the stress intensity factors following the solution of the elasticity fields for a cracked solid.

2.1. Elasticity problem

We consider a body $\mathcal{B} \subset \mathbb{R}^2$ undergoing a deformation defined by the displacement field \mathbf{u} . We assume \mathcal{B} to be an open connected domain with a (piecewise) smooth boundary $\partial\mathcal{B}$. We represent the with a twice differentiable, simple, and rectifiable curve $\mathcal{C} \subset \mathcal{B}$ and denote its faces with \mathcal{C}_\pm . The cracked domain is given by $\mathcal{B}_\mathcal{C} = \mathcal{B} \setminus \mathcal{C}$. The boundary of $\mathcal{B}_\mathcal{C}$ is the union of the crack faces and the boundary of \mathcal{B} , namely, $\partial\mathcal{B}_\mathcal{C} = \partial\mathcal{B} \cup \mathcal{C}_\pm$. Let $\partial\mathcal{B}_\mathcal{C}$ be decomposed into $\partial_\tau\mathcal{B}_\mathcal{C}$ and $\partial_d\mathcal{B}_\mathcal{C}$ such that $\partial_\tau\mathcal{B}_\mathcal{C} \supseteq \mathcal{C}_\pm$, $\partial_\tau\mathcal{B}_\mathcal{C} \cup \partial_d\mathcal{B}_\mathcal{C} = \partial\mathcal{B}_\mathcal{C}$, and $\partial_\tau\mathcal{B}_\mathcal{C} \cap \partial_d\mathcal{B}_\mathcal{C} = \emptyset$. Tractions $\bar{\mathbf{t}}$ and displacements $\bar{\mathbf{u}}$ are prescribed over $\partial_\tau\mathcal{B}_\mathcal{C}$ and $\partial_d\mathcal{B}_\mathcal{C}$, respectively, while a body force field \mathbf{b} is applied over $\mathcal{B}_\mathcal{C}$. Let \mathbf{x}_t denote any one of the two crack tips. We denote by \mathbf{n} the unit external normal to \mathcal{B} and the unit external normal to each one of the two faces of the crack. Figure 1 shows the schematic of the problem configuration.

We confine ourselves to planar linear elasticity in the context of infinitesimal deformations. The elasticity problem statement reads as follows: given \mathbf{b} , $\bar{\mathbf{u}}$, and $\bar{\mathbf{t}}$, find $\mathbf{u} : \mathcal{B}_\mathcal{C} \rightarrow \mathbb{R}^2$ such that

$$\nabla \cdot \boldsymbol{\sigma}(\nabla \mathbf{u}) + \mathbf{b} = 0, \quad \text{in } \mathcal{B}_\mathcal{C}, \tag{1a}$$

$$\mathbf{u} = \bar{\mathbf{u}}, \quad \text{on } \partial_d\mathcal{B}_\mathcal{C}, \tag{1b}$$

$$\boldsymbol{\sigma}(\nabla \mathbf{u})\mathbf{n} = \bar{\mathbf{t}}, \quad \text{on } \partial_\tau\mathcal{B}_\mathcal{C}, \tag{1c}$$

where $\boldsymbol{\sigma}$ is the stress tensor. This is given by

$$\boldsymbol{\sigma}(\nabla \mathbf{u}) = \mathbb{C} : \nabla \mathbf{u} \tag{2}$$

and

$$\mathbb{C} = \hat{\lambda} \mathbf{1} \otimes \mathbf{1} + 2\mu \mathbb{I}, \quad \hat{\lambda} = \begin{cases} \lambda, & \text{for plane strain,} \\ \frac{2\lambda\mu}{\lambda + 2\mu}, & \text{for plane stress.} \end{cases}$$

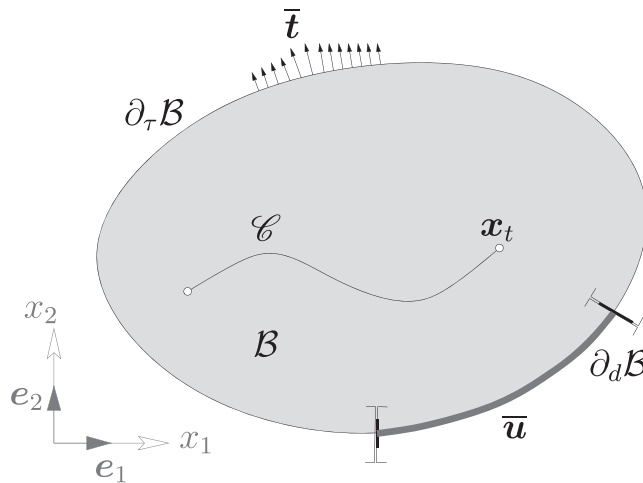


Figure 1. The configuration of the problem.

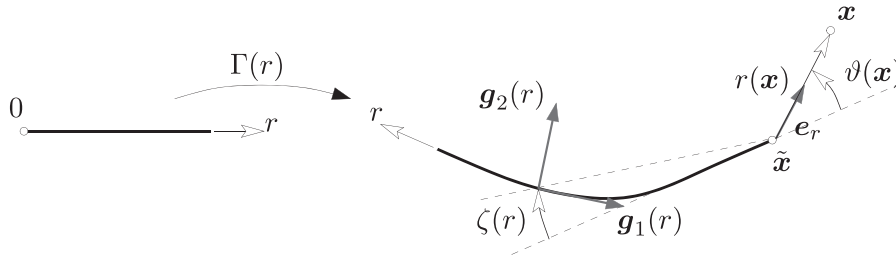


Figure 2. Description of local basis and coordinates.

The constants λ and μ are Lamé’s first and second parameters, respectively; $\mathbf{1}$ is the identity second order tensor, and \mathbb{I} is the fourth order symmetric identity operator given by

$$\mathbb{I} = \frac{1}{2}(\delta_{ik}\delta_{jl} + \delta_{il}\delta_{jk})\mathbf{e}_i \otimes \mathbf{e}_j \otimes \mathbf{e}_k \otimes \mathbf{e}_l,$$

where $\{\mathbf{e}_1, \mathbf{e}_2\}$ is a Cartesian basis and an index repeated twice in the same term indicates summation from 1 to 2 in such index.

2.2. Crack tip coordinates and stress intensity factors

To aid the definition of the stress intensity factors, we first introduce a system of coordinates and a family of vector bases.

Let (r, ϑ) be the crack tip polar coordinates as shown in Figure 2, and let $B_\rho(\hat{\mathbf{x}}) = \{\mathbf{x} \in \mathbb{R}^2 \mid |\mathbf{x} - \hat{\mathbf{x}}| < \rho\}$ be the open ball of radius $\rho > 0$ centered at $\hat{\mathbf{x}}$. The radial coordinate is defined as $r(\mathbf{x}) := |\mathbf{x} - \mathbf{x}_t|$. Let $\Gamma : r \mapsto \{\mathbf{x} \in \mathcal{C} \mid |\mathbf{x} - \mathbf{x}_t| = r\}$ be a description of the part of the crack parameterized by the distance to the crack tip. We set the domain of Γ to be $[0, \rho]$ with $\rho > 0$ such that $B_\rho(\mathbf{x}_t) \subset \mathcal{B}$ and that $\Gamma' \neq 0$ over its domain of definition. As a consequence, Γ is bijective for $r \in [0, \rho]$. We re-iterate that the crack is assumed to be a twice continuously differentiable curve such that $\Gamma \in C^2(\mathbb{R}_0^+; \mathbb{R}^2)$, where $\mathbb{R}_0^+ = \{0\} \cup \mathbb{R}^+$. By convention, the possible values of the (r, ϑ) coordinates for points in $B_\rho(\mathbf{x}_t)$ that we will use belong to

$$D_\rho = \{(r, \vartheta) \in \mathbb{R}_0^+ \times \mathbb{R} \mid -\pi - \zeta(r) \leq \vartheta \leq \pi - \zeta(r)\},$$

where $\zeta(r)$ is the angle between the vector $\Gamma(r) - \mathbf{x}_t$ and $\Gamma'(0)$. In other words, $\zeta(r)$ is the angle subtended by (1) the tangent at the crack tip and (2) the secant line passing through the crack tip and $\Gamma(r)$. Figure 3 shows the values of the coordinate ϑ for the particular case of a circular arc crack. Lastly, let $\mathbf{g}_i(r), r \in [0, \rho]$ be the right-handed orthonormal bases induced from the mapping Γ such that $\mathbf{g}_1(r) = -\Gamma'(r)/|\Gamma'(r)|$, see Figure 2.

Throughout this manuscript, we assume that there exist unique real numbers K_I, K_{II} such that

$$\mathbf{u} = K_I \mathbf{u}^I + K_{II} \mathbf{u}^{II} + \mathbf{u}_S, \tag{3}$$

where K_I and K_{II} are the modes I and II stress intensity factors, $\mathbf{u}^I, \mathbf{u}^{II} \in H^1(\mathcal{B}_\rho; \mathbb{R}^2) \setminus H^2(\mathcal{B}_\rho; \mathbb{R}^2)$ are the asymptotic displacement solutions of these modes, and $\mathbf{u}_S \in H^2(\mathcal{B}_\rho; \mathbb{R}^2)$ is a smoother displacement field than the asymptotic ones and its regularity is governed by the body forces and boundary conditions.

The problem of evaluating the stress intensity factors is as follows: given a solution \mathbf{u} of problem (1), compute K_I and K_{II} as defined in (3).

Remark (Explicit evaluation of the behavior at the crack tip)

Whenever $\boldsymbol{\beta} = \nabla \mathbf{u} \in C^0(\mathcal{B}_\rho; \mathbb{R}^{2 \times 2})$, the stress intensity factors can be equivalently defined as

$$\begin{aligned} K_I &= \lim_{r \rightarrow 0} \sqrt{2\pi r} \boldsymbol{\sigma}(\boldsymbol{\beta})|_{\vartheta=0} : \mathbf{g}_2 \otimes \mathbf{g}_2, \\ K_{II} &= \lim_{r \rightarrow 0} \sqrt{2\pi r} \boldsymbol{\sigma}(\boldsymbol{\beta})|_{\vartheta=0} : \mathbf{g}_1 \otimes \mathbf{g}_2. \end{aligned} \tag{4}$$

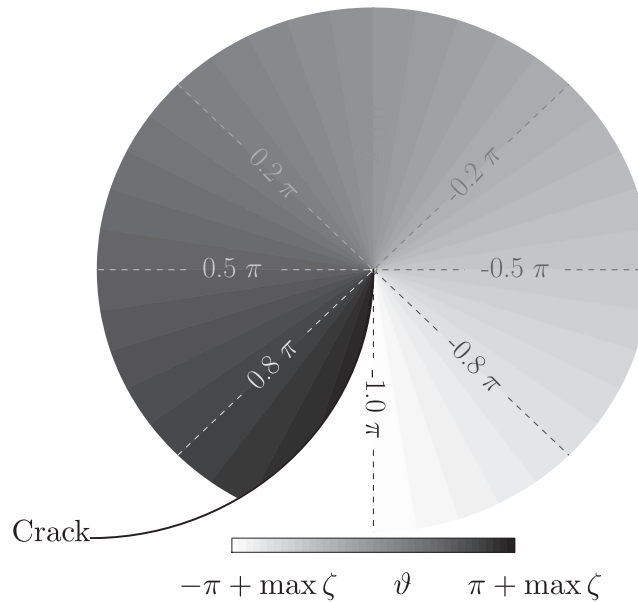


Figure 3. The ϑ coordinate with the branch cut along the crack.

The calculation of the stress intensity factors by evaluating the limits in (4) with a numerical solution often leads to poor results. In fact, few methods are capable of accurately resolving the singularity in the stress field. Therefore, the predominant methods to compute the stress intensity factors are based on the approximation of the *interaction integral*, a formulation that avoids point-wise evaluation of the stress field in the region with the singularity. We proceed to introduce the interaction integral in Section 3.

3. INTERACTION INTEGRAL

In the sequel, we first define the interaction integral functional between any two admissible fields alongside a concise justification of this definition (Section 3.1). In Section 3.2, we specialize it to the case in which one of the fields is the solution to the elasticity problem of interest. This specialization results in a formulation that possesses the following properties: (1) it does not involve second derivatives of the discrete approximation to the exact displacement field; (2) it is a problem-dependent functional because it uses the prescribed tractions and body forces; and (3) it can be further simplified, depending on the problem, by carefully choosing the so-called material variation and auxiliary fields, to be described later. We perform the simplification in Sections 3.3 and 3.4 and obtain three different formulas. In Section 3.5, we provide guidelines on which specific formula for the interaction integral to choose for a given application.

3.1. Definition of the interaction integral functional

The interaction integral involves two elasticity fields, one with known stress intensity factors and the other whose stress intensity factors we are interested in evaluating.

Let us introduce the *interaction energy momentum tensor* $\bar{\Sigma} : \mathbb{R}^{2 \times 2} \times \mathbb{R}^{2 \times 2} \rightarrow \mathbb{R}^{2 \times 2}$ defined as

$$\bar{\Sigma}(\beta^a, \beta^b) = \bar{w}(\beta^a, \beta^b) \mathbf{1} - \beta^a \top \sigma(\beta^b) - \beta^b \top \sigma(\beta^a),$$

where $\bar{w} : \mathbb{R}^{2 \times 2} \times \mathbb{R}^{2 \times 2} \rightarrow \mathbb{R}$ is given as

$$\bar{w}(\beta^a, \beta^b) = \frac{1}{2} [\sigma(\beta^a) : \beta^b + \sigma(\beta^b) : \beta^a]. \tag{5}$$

Assuming the same constitutive relation (2) for both fields, (5) simplifies to

$$\bar{w}(\boldsymbol{\beta}^a, \boldsymbol{\beta}^b) = \boldsymbol{\sigma}(\boldsymbol{\beta}^a) : \boldsymbol{\beta}^b = \boldsymbol{\sigma}(\boldsymbol{\beta}^b) : \boldsymbol{\beta}^a.$$

Additionally, let the set of *material variations* be defined as

$$\mathcal{M} = \{\delta\boldsymbol{\gamma} \in C^1(\mathcal{B}_\rho; \mathbb{R}^2) \mid |\delta\boldsymbol{\gamma} = \mathbf{0} \text{ in } \mathcal{B}_\rho \setminus B_\rho(\mathbf{x}_t), \delta\boldsymbol{\gamma}(\mathbf{x}_t) = \mathbf{g}_1(0)\}. \quad (6)$$

Finally, let

$$\mathcal{B}^b = \text{span}\{\nabla\mathbf{u}^I, \nabla\mathbf{u}^{II}\} \oplus H^1(\mathcal{B}_\rho; \mathbb{R}^{2 \times 2}),$$

and for any tensor field $\boldsymbol{\beta} \in \mathcal{B}^b$, define $K_I[\boldsymbol{\beta}]$ and $K_{II}[\boldsymbol{\beta}]$ such that

$$\boldsymbol{\beta} = K_I[\boldsymbol{\beta}]\nabla\mathbf{u}^I + K_{II}[\boldsymbol{\beta}]\nabla\mathbf{u}^{II} + \boldsymbol{\beta}_S \quad (7)$$

with $\boldsymbol{\beta}_S \in H^1(\mathcal{B}_\rho; \mathbb{R}^{2 \times 2})$. In particular, if $\boldsymbol{\beta} = \nabla\mathbf{u}$, for \mathbf{u} being a solution of problem (1), then $K_I[\boldsymbol{\beta}]$ and $K_{II}[\boldsymbol{\beta}]$ are the stress intensity factors of \mathbf{u} . However, $K_I[\boldsymbol{\beta}]$ and $K_{II}[\boldsymbol{\beta}]$ are also defined for any $\boldsymbol{\beta} \in \mathcal{B}^b$ that is not the gradient of a displacement field. For convenience, regardless of whether $\boldsymbol{\beta}$ is or is not the gradient of a displacement field, we will refer to $K_I[\boldsymbol{\beta}]$ and $K_{II}[\boldsymbol{\beta}]$ as the stress intensity factors of $\boldsymbol{\beta}$.

We define the *interaction integral functional* $\hat{\mathcal{I}} : \mathcal{B}^b \times \mathcal{B}^b \times \mathcal{M} \rightarrow \mathbb{R}$ as

$$\begin{aligned} \hat{\mathcal{I}}[\boldsymbol{\beta}^a, \boldsymbol{\beta}^b, \delta\boldsymbol{\gamma}] &= \int_{\mathcal{C}_\rho^\pm} \delta\boldsymbol{\gamma} \cdot \bar{\boldsymbol{\Sigma}}(\boldsymbol{\beta}^a, \boldsymbol{\beta}^b) \mathbf{n} \, dS \\ &\quad - \int_{B_\rho(\mathbf{x}_t) \setminus \mathcal{C}} \left[\bar{\boldsymbol{\Sigma}}(\boldsymbol{\beta}^a, \boldsymbol{\beta}^b) : \nabla\delta\boldsymbol{\gamma} + \nabla \cdot \bar{\boldsymbol{\Sigma}}(\boldsymbol{\beta}^a, \boldsymbol{\beta}^b) \cdot \delta\boldsymbol{\gamma} \right] \, dV, \end{aligned} \quad (8)$$

where, for convenience, we let \mathcal{C}_ρ^\pm denotes $\mathcal{C}_\pm \cap B_\rho(\mathbf{x}_t)$. The value $\hat{\mathcal{I}}[\boldsymbol{\beta}^a, \boldsymbol{\beta}^b, \delta\boldsymbol{\gamma}]$ is the *interaction integral* between $\boldsymbol{\beta}^a$ and $\boldsymbol{\beta}^b$. The relation between the interaction integral and the stress intensity factors of $\boldsymbol{\beta}^a, \boldsymbol{\beta}^b \in \mathcal{B}^b$ is

$$\hat{\mathcal{I}}[\boldsymbol{\beta}^a, \boldsymbol{\beta}^b, \delta\boldsymbol{\gamma}] = \eta \left(K_I[\boldsymbol{\beta}^a] K_I[\boldsymbol{\beta}^b] + K_{II}[\boldsymbol{\beta}^a] K_{II}[\boldsymbol{\beta}^b] \right) \quad (9)$$

for any $\delta\boldsymbol{\gamma} \in \mathcal{M}$, where η is a material constant defined as

$$\eta = \begin{cases} \frac{\lambda + 2\mu}{2\mu(\lambda + \mu)}, & \text{for plane strain,} \\ \frac{2(\lambda + \mu)}{\mu(3\lambda + 2\mu)}, & \text{for plane stress.} \end{cases}$$

It follows from (9) that, if we are interested in finding $K_I[\boldsymbol{\beta}]$ (or $K_{II}[\boldsymbol{\beta}]$), we must generate an *auxiliary tensor field* $\boldsymbol{\beta}_I^{\text{aux}}$ (or $\boldsymbol{\beta}_{II}^{\text{aux}} \in \mathcal{B}^b$, satisfying $K_I[\boldsymbol{\beta}_I^{\text{aux}}] = 1$, $K_{II}[\boldsymbol{\beta}_I^{\text{aux}}] = 0$ (or $K_I[\boldsymbol{\beta}_{II}^{\text{aux}}] = 0$, $K_{II}[\boldsymbol{\beta}_{II}^{\text{aux}}] = 1$). In this case, (9) implies that

$$K_{I,II}[\boldsymbol{\beta}] = \frac{\hat{\mathcal{I}}[\boldsymbol{\beta}, \boldsymbol{\beta}_{I,II}^{\text{aux}}, \delta\boldsymbol{\gamma}]}{\eta}.$$

Notice that the interaction integral can be regarded as a tool to *extract* the singular parts of fields $\boldsymbol{\beta}^a, \boldsymbol{\beta}^b$, as it follows from (8). The regular part $\boldsymbol{\beta}_S$ of either field (cf. (7)) does not contribute to the value of the interaction integral.

Justification (Equation (9))

Consider $\boldsymbol{\beta}^a, \boldsymbol{\beta}^b \in \mathcal{B}^b$, and $r > 0$. Notice that the two terms in the volume integral of (8) form an exact divergence. Applying the divergence theorem on $(B_\rho(\mathbf{x}_t) \setminus \mathcal{C}) \setminus B_r(\mathbf{x}_t)$ reveals that the

integration in (8) over $(B_\rho(\mathbf{x}_t) \setminus \mathcal{C}) \setminus B_r(\mathbf{x}_t)$ and $\mathcal{C}_\rho^\pm \setminus B_r(\mathbf{x}_t)$ adds up to an integral over $\partial B_r(\mathbf{x}_t)$. It then follows that

$$\hat{\mathcal{I}}[\boldsymbol{\beta}^a, \boldsymbol{\beta}^b, \delta\boldsymbol{\gamma}] = \lim_{r \rightarrow 0} \int_{\partial B_r(\mathbf{x}_t)} \delta\boldsymbol{\gamma} \cdot \overline{\boldsymbol{\Sigma}}(\boldsymbol{\beta}^a, \boldsymbol{\beta}^b) \mathbf{n} \, dS, \tag{10}$$

with \mathbf{n} here also used to denote the outward unit normal to $\partial B_r(\mathbf{x}_t)$, because the rest of the terms vanish as $r \rightarrow 0$.

To proceed in showing that (10) implies (9), we write $\boldsymbol{\beta}^a = \boldsymbol{\beta}_T^a + \boldsymbol{\beta}_S^a$ and $\boldsymbol{\beta}^b = \boldsymbol{\beta}_T^b + \boldsymbol{\beta}_S^b$, where $\boldsymbol{\beta}_T^a, \boldsymbol{\beta}_T^b \in \text{span}\{\nabla \mathbf{u}^I, \nabla \mathbf{u}^{II}\}$ and $\boldsymbol{\beta}_S^a, \boldsymbol{\beta}_S^b \in H^1(\mathcal{B}_\rho; \mathbb{R}^{2 \times 2})$. It is straightforward to show that

$$\hat{\mathcal{I}}[\boldsymbol{\beta}_T^a, \boldsymbol{\beta}_T^b, \delta\boldsymbol{\gamma}] = \lim_{r \rightarrow 0} \int_{\partial B_r(\mathbf{x}_t)} \delta\boldsymbol{\gamma} \cdot \overline{\boldsymbol{\Sigma}}(\boldsymbol{\beta}_T^a, \boldsymbol{\beta}_T^b) \mathbf{n} \, dS = \eta \left(K_I[\boldsymbol{\beta}^a] K_I[\boldsymbol{\beta}^b] + K_{II}[\boldsymbol{\beta}^a] K_{II}[\boldsymbol{\beta}^b] \right).$$

Therefore, it remains to show that $\hat{\mathcal{I}}[\boldsymbol{\beta}_S^a, \boldsymbol{\beta}_T^b, \delta\boldsymbol{\gamma}] = \hat{\mathcal{I}}[\boldsymbol{\beta}_T^a, \boldsymbol{\beta}_S^b, \delta\boldsymbol{\gamma}] = \hat{\mathcal{I}}[\boldsymbol{\beta}_S^a, \boldsymbol{\beta}_S^b, \delta\boldsymbol{\gamma}] = 0$. To this end, we first define

$$\hat{\mathcal{I}}_r[\boldsymbol{\beta}^a, \boldsymbol{\beta}^b, \delta\boldsymbol{\gamma}] := \int_{\partial B_r(\mathbf{x}_t)} \delta\boldsymbol{\gamma} \cdot \overline{\boldsymbol{\Sigma}}(\boldsymbol{\beta}^a, \boldsymbol{\beta}^b) \mathbf{n} \, dS.$$

Then, we invoke the Cauchy–Schwarz inequality and the explicit expression of $\boldsymbol{\beta}_T^b$ to obtain

$$\begin{aligned} \left| \hat{\mathcal{I}}_r[\boldsymbol{\beta}_S^a, \boldsymbol{\beta}_T^b, \delta\boldsymbol{\gamma}] \right| &\leq C \|\boldsymbol{\beta}_S^a\|_{L^2[\partial B_r(\mathbf{x}_t)]} \left\| \boldsymbol{\beta}_T^b \right\|_{L^2[\partial B_r(\mathbf{x}_t)]} \leq C \|\boldsymbol{\beta}_S^a\|_{L^2[\partial B_r(\mathbf{x}_t)]}, \\ \left| \hat{\mathcal{I}}_r[\boldsymbol{\beta}_S^a, \boldsymbol{\beta}_S^b, \delta\boldsymbol{\gamma}] \right| &\leq C \|\boldsymbol{\beta}_S^a\|_{L^2[\partial B_r(\mathbf{x}_t)]} \left\| \boldsymbol{\beta}_S^b \right\|_{L^2[\partial B_r(\mathbf{x}_t)]}, \end{aligned} \tag{11}$$

where $C > 0$ is independent of r .

To continue, we need to invoke a trace inequality with a scaling of r for any $f \in H^1[B_\rho(\mathbf{x}_t) \setminus \mathcal{C}]$ and $r \in (0, \rho)$,

$$\|f\|_{L^2[\partial B_r(\mathbf{x}_t)]} \leq C r^{1/2} \|f\|_{H^1[B_\rho(\mathbf{x}_t) \setminus \mathcal{C}]}, \tag{12}$$

where $C > 0$ is independent of f and r^\ddagger .

With (12), we then proceed to simplify (11):

$$\begin{aligned} \left| \hat{\mathcal{I}}_r[\boldsymbol{\beta}_S^a, \boldsymbol{\beta}_T^b, \delta\boldsymbol{\gamma}] \right| &\leq C r^{1/2} \|\boldsymbol{\beta}_S^a\|_{H^1[\partial B_\rho(\mathbf{x}_t) \setminus \mathcal{C}]}, \\ \left| \hat{\mathcal{I}}_r[\boldsymbol{\beta}_S^a, \boldsymbol{\beta}_S^b, \delta\boldsymbol{\gamma}] \right| &\leq C r \|\boldsymbol{\beta}_S^a\|_{H^1[\partial B_\rho(\mathbf{x}_t) \setminus \mathcal{C}]} \left\| \boldsymbol{\beta}_S^b \right\|_{H^1[\partial B_\rho(\mathbf{x}_t) \setminus \mathcal{C}]}. \end{aligned}$$

Thus, as $r \rightarrow 0$, both $\hat{\mathcal{I}}_r[\boldsymbol{\beta}_S^a, \boldsymbol{\beta}_T^b, \delta\boldsymbol{\gamma}]$ and $\hat{\mathcal{I}}_r[\boldsymbol{\beta}_S^a, \boldsymbol{\beta}_S^b, \delta\boldsymbol{\gamma}]$ tend to zero. From a symmetry in the first two slots of $\hat{\mathcal{I}}_r$, we also have $\lim_{r \rightarrow 0} \hat{\mathcal{I}}[\boldsymbol{\beta}_T^a, \boldsymbol{\beta}_S^b, \delta\boldsymbol{\gamma}] = 0$.

[‡] To prove (12), we first write

$$\bar{f} := \frac{1}{\pi \rho^2} \int_{B_\rho(\mathbf{x}_t)} f \, d\Omega, \quad \hat{f} := f - \bar{f}.$$

Then, with a form of Poincaré’s inequality and a scaling argument,

$$\|\hat{f}\|_{L^2[\partial B_r(\mathbf{x}_t)]} \leq C r^{1/2} \|f\|_{H^1[B_r(\mathbf{x}_t) \setminus \mathcal{C}]} \leq C r^{1/2} \|f\|_{H^1[B_\rho(\mathbf{x}_t) \setminus \mathcal{C}]},$$

On the other hand, because $\|\bar{f}\|_{L^2[\partial B_r(\mathbf{x}_t)]} = (\bar{f}^2 2\pi r)^{1/2}$ and $\|\bar{f}\|_{L^2[B_\rho(\mathbf{x}_t)]} = (\bar{f}^2 \pi \rho^2)^{1/2}$, we have

$$\|\bar{f}\|_{L^2[\partial B_r(\mathbf{x}_t)]} = C r^{1/2} \|\bar{f}\|_{L^2[B_\rho(\mathbf{x}_t)]} \leq C r^{1/2} \|f\|_{L^2[B_\rho(\mathbf{x}_t)]}.$$

Adding these two inequalities yields (12).

Remark (Relation to the energy release rate)

The interaction integral functional is directly related to the energy release rate $\mathcal{G} : \mathcal{B}^b \times \mathcal{M} \rightarrow \mathbb{R}$ [30], which can be defined as

$$\mathcal{G}[\boldsymbol{\beta}, \delta\boldsymbol{\gamma}] = \lim_{r \rightarrow 0} \int_{\partial B_r(\mathbf{x}_t)} \delta\boldsymbol{\gamma} \cdot \boldsymbol{\Sigma}(\boldsymbol{\beta}) \mathbf{n} \, dS, \tag{13}$$

where $\boldsymbol{\Sigma} : \mathbb{R}^{2 \times 2} \rightarrow \mathbb{R}^{2 \times 2}$ is Eshelby's energy momentum tensor [31] and \mathbf{n} is used to denote the outward unit normal to $\partial B_r(\mathbf{x}_t)$. For linear elastic materials, Eshelby's energy momentum tensor takes the form

$$\boldsymbol{\Sigma}(\boldsymbol{\beta}) = \frac{1}{2} \boldsymbol{\sigma}(\boldsymbol{\beta}) : \boldsymbol{\beta} \mathbf{1} - \boldsymbol{\beta}^\top \boldsymbol{\sigma}(\boldsymbol{\beta}).$$

The aforementioned equation is related to the interaction energy momentum tensor by the following relation

$$\overline{\boldsymbol{\Sigma}}(\boldsymbol{\beta}^a, \boldsymbol{\beta}^b) = \boldsymbol{\Sigma}(\boldsymbol{\beta}^a + \boldsymbol{\beta}^b) - \boldsymbol{\Sigma}(\boldsymbol{\beta}^a) - \boldsymbol{\Sigma}(\boldsymbol{\beta}^b) \Rightarrow \boldsymbol{\Sigma}(\boldsymbol{\beta}^b) = \frac{1}{2} \overline{\boldsymbol{\Sigma}}(\boldsymbol{\beta}^b, \boldsymbol{\beta}^b).$$

Comparing (13) and (10) and exploiting the linearity of the constitutive relation, we have the following relation between the interaction integral functional and the energy release rate

$$\hat{\mathcal{I}}[\boldsymbol{\beta}^a, \boldsymbol{\beta}^b, \delta\boldsymbol{\gamma}] = \mathcal{G}[\boldsymbol{\beta}^a + \boldsymbol{\beta}^b, \delta\boldsymbol{\gamma}] - \mathcal{G}[\boldsymbol{\beta}^a, \delta\boldsymbol{\gamma}] - \mathcal{G}[\boldsymbol{\beta}^b, \delta\boldsymbol{\gamma}]. \tag{14}$$

After replacing with (7) and evaluating, the limit in (13) gives the widely known relation

$$\mathcal{G}[\boldsymbol{\beta}, \delta\boldsymbol{\gamma}] = \eta (K_I [\boldsymbol{\beta}]^2 + K_{II} [\boldsymbol{\beta}]^2).$$

We can alternatively recover (9) by replacing this relation into (14). Thus, (9) can be justified by the direct evaluation of the limit in (10) or by its relation to the energy release rate. It is worth noting that while \mathcal{G} is a non-linear functional in $\boldsymbol{\beta}$, $\hat{\mathcal{I}}$ is linear in both $\boldsymbol{\beta}^a$ and $\boldsymbol{\beta}^b$.

Remark (Constraints on \mathcal{M})

In (13), $\delta\boldsymbol{\gamma}$ is understood as a variation of material points that represents unit crack advancement. Thus, in order to compute the energy release rate, we must enforce $\delta\boldsymbol{\gamma} = \mathbf{g}_1(0)$ at \mathbf{x}_t . This constraint is the justification behind the definition of \mathcal{M} in (6).

3.2. Problem-dependent interaction integral functional

We present here a functional \mathcal{I} that takes the same value as $\hat{\mathcal{I}}$ when $\boldsymbol{\beta}^a$ coincides with the solution of problem (1). Namely, if $\boldsymbol{\beta}^a = \nabla \mathbf{u}$, where \mathbf{u} satisfies (1a) and (1c), then $\mathcal{I}[\boldsymbol{\beta}^a, \boldsymbol{\beta}^b, \delta\boldsymbol{\gamma}] = \hat{\mathcal{I}}[\boldsymbol{\beta}^a, \boldsymbol{\beta}^b, \delta\boldsymbol{\gamma}]$ for any $\boldsymbol{\beta}^b$. Therefore, in this case, $\mathcal{I}[\boldsymbol{\beta}^a, \boldsymbol{\beta}^b, \delta\boldsymbol{\gamma}]$ is the interaction integral between $\boldsymbol{\beta}^a$ and $\boldsymbol{\beta}^b$.

The motivation behind introducing \mathcal{I} is to formulate a functional defined over gradients of displacement fields that belong to classical finite elements spaces, namely, a functional for which no second derivatives of a numerical solution are needed. This is possible because, in contrast to $\hat{\mathcal{I}}$, \mathcal{I} does not involve derivatives of $\boldsymbol{\beta}^a$.

Expanding the divergence in (8) and substituting with (1a) and (1c) for $\boldsymbol{\beta}^a = \nabla \mathbf{u}$ yields

$$\begin{aligned} \hat{\mathcal{I}}[\boldsymbol{\beta}^a, \boldsymbol{\beta}^b, \delta\boldsymbol{\gamma}] &= \int_{\mathcal{C}_\rho^\pm} \delta\boldsymbol{\gamma} \cdot \bar{\boldsymbol{\tau}}(\boldsymbol{\beta}^a, \boldsymbol{\beta}^b) \, dS \\ &\quad - \int_{B_\rho(\mathbf{x}_t) \setminus \mathcal{C}} \left[\overline{\boldsymbol{\Sigma}}(\boldsymbol{\beta}^a, \boldsymbol{\beta}^b) : \nabla \delta\boldsymbol{\gamma} + \delta\boldsymbol{\gamma} \cdot \bar{\boldsymbol{\lambda}}(\boldsymbol{\beta}^a, \boldsymbol{\beta}^b) \right] \, dV, \end{aligned}$$

where

$$\bar{\tau}(\boldsymbol{\beta}^a, \boldsymbol{\beta}^b) = \bar{w}(\boldsymbol{\beta}^a, \boldsymbol{\beta}^b) \mathbf{n} - \boldsymbol{\beta}^a \top \boldsymbol{\sigma}(\boldsymbol{\beta}^b) \mathbf{n} - \boldsymbol{\beta}^b \top \bar{\mathbf{t}}, \tag{15}$$

and

$$\bar{\lambda}(\boldsymbol{\beta}^a, \boldsymbol{\beta}^b) = \boldsymbol{\beta}^a : \nabla \boldsymbol{\sigma}(\boldsymbol{\beta}^b) - \boldsymbol{\sigma}(\boldsymbol{\beta}^a) : (\nabla \boldsymbol{\beta}^b)^\top - \boldsymbol{\beta}^a \top \nabla \cdot \boldsymbol{\sigma}(\boldsymbol{\beta}^b) + \boldsymbol{\beta}^b \top \mathbf{b}. \tag{16}$$

Remark (Indicial expression of relevant quantities)

For ease of implementation, we provide here the indicial representation of $\bar{\Sigma}$, $\bar{\tau}$, and $\bar{\lambda}$ (making use of Einstein's repeated indexes convention), namely,

$$\begin{aligned} \bar{\Sigma}_{ij}(\boldsymbol{\beta}^a, \boldsymbol{\beta}^b) &= \sigma_{kl}^a \beta_{kl}^b \delta_{ij} - \beta_{ki}^a \sigma_{kj}^b - \beta_{ki}^b \sigma_{kj}^a, \\ \bar{\tau}_i(\boldsymbol{\beta}^a, \boldsymbol{\beta}^b) &= \bar{w} n_i - \beta_{ji}^a \sigma_{jk}^b n_k - \beta_{ji}^b \bar{t}_j, \\ \bar{\lambda}_i(\boldsymbol{\beta}^a, \boldsymbol{\beta}^b) &= \beta_{mn}^a \sigma_{mn,i}^b - \sigma_{kj}^a \beta_{ki,j}^b - \beta_{ki}^a \sigma_{kj,j}^b + \beta_{ki}^b b_k, \end{aligned}$$

where $\sigma_{ij}^{a,b}$ is understood as $\sigma(\boldsymbol{\beta}^{a,b})_{ij}$.

Notice that for each pair $(\boldsymbol{\beta}^a, \boldsymbol{\beta}^b)$, $\bar{\tau}(\boldsymbol{\beta}^a, \boldsymbol{\beta}^b)$, and $\bar{\lambda}(\boldsymbol{\beta}^a, \boldsymbol{\beta}^b)$ are functions over \mathcal{C}_ρ^\pm and $B_\rho(\mathbf{x}_t) \setminus \mathcal{C}$, respectively.

To reflect the lower regularity needed for $\boldsymbol{\beta}^a$, we first define a finite partition of $\mathcal{B}_\mathcal{C}$ as a set $\{T_1, \dots, T_N\}$ for some $N \in \mathbb{N}$ such that T_i is open for any i , $T_i \cap T_j = \emptyset$, for any $i \neq j$, and $\bigcup_i T_i = \overline{\mathcal{B}_\mathcal{C}}$. An example of such partition is a finite element mesh for $\mathcal{B}_\mathcal{C}$. Then, we can set

$$\begin{aligned} \mathcal{B}^a &= \{ \boldsymbol{\beta} \in L^2(\mathcal{B}_\mathcal{C}, \mathbb{R}^{2 \times 2}) \mid \boldsymbol{\beta}|_{T_i} \in H^1(T_i; \mathbb{R}^{2 \times 2}) \text{ for any } T_i \text{ in some finite partition of } \mathcal{B}_\mathcal{C} \} \\ &\quad \oplus \text{span} \{ \nabla \mathbf{u}^I, \nabla \mathbf{u}^{II} \} \end{aligned}$$

and define $\mathcal{I} : \mathcal{B}^a \times \mathcal{B}^b \times \mathcal{M} \rightarrow \mathbb{R}$ as

$$\begin{aligned} \mathcal{I}[\boldsymbol{\beta}^a, \boldsymbol{\beta}^b, \delta \boldsymbol{\gamma}] &= \int_{\mathcal{C}_\rho^\pm} \overbrace{\delta \boldsymbol{\gamma} \cdot \bar{\tau}(\boldsymbol{\beta}^a, \boldsymbol{\beta}^b)}^{\mathcal{I}_1} dS \\ &\quad - \int_{B_\rho(\mathbf{x}_t) \setminus \mathcal{C}} \left[\underbrace{\bar{\Sigma}(\boldsymbol{\beta}^a, \boldsymbol{\beta}^b) : \nabla \delta \boldsymbol{\gamma}}_{\mathcal{I}_2} + \underbrace{\delta \boldsymbol{\gamma} \cdot \bar{\lambda}(\boldsymbol{\beta}^a, \boldsymbol{\beta}^b)}_{\mathcal{I}_3} \right] dV. \end{aligned} \tag{17}$$

In this way, $\boldsymbol{\beta}^a$ can have discontinuities across a finite number of interfaces, as in finite element solutions and still have well-defined values at the crack faces (which a function in L^2 may not have). Note that \mathcal{I} is linear in $\boldsymbol{\beta}^b$ but affine in $\boldsymbol{\beta}^a$, and therefore, not symmetric with respect to them. The way \mathcal{I} will be used is by setting $\boldsymbol{\beta}^a$ to be either the exact solution or a numerical approximation to it and $\boldsymbol{\beta}^b$ to be an auxiliary field.

3.3. The fields

We next proceed to construct the material variation and auxiliary fields that will enable the extraction of the stress intensity factors for curvilinear crack geometries.

3.3.1. Material variation fields. The objective of this section is to construct vector fields $\delta \boldsymbol{\gamma}$ that belong to the set of material variations \mathcal{M} , see (6). We provide two constructs, but any $\delta \boldsymbol{\gamma} \in \mathcal{M}$ could be used. We start from a general form $\delta \boldsymbol{\gamma}(\mathbf{x}_t + r \mathbf{e}_r) = q(r) \mathbf{t}(r)$, where $q : \mathbb{R}_0^+ \rightarrow \mathbb{R}_0^+$

and $\mathbf{t} : \mathbb{R}_0^+ \rightarrow \mathbb{R}^2$. The function $q(r)$ represents the magnitude of the material variation field and is constructed to have support within $B_\rho(\mathbf{x}_t)$. The function $\mathbf{t}(r)$ embodies the direction of the material variation field and is taken to satisfy $|\mathbf{t}(r)| = 1, \forall r \in \mathbb{R}_0^+$.

The magnitude and direction of the two material variation fields that we propose only depend on r . We will thus abuse notation writing $\delta\boldsymbol{\gamma}(r)$ in place of $\delta\boldsymbol{\gamma}(\mathbf{x}_t + r\mathbf{e}_r)$.

The scalar function $q(r) \in C^2(\mathbb{R}_0^+)$ is defined as

$$q(r) = \begin{cases} 1, & \text{if } r \leq \rho_I, \\ f(r), & \text{if } \rho_I < r < \rho, \\ 0, & \text{otherwise,} \end{cases} \tag{18}$$

with $f(r)$ being a fifth order polynomial and $\rho_I = \rho/4$. Note that to construct higher order methods, the regularity of $q(r)$ and thus the polynomial order of $f(r)$ will have to be suitably adapted.

In the sequel, we list the two material variation fields:

- (1) *Unidirectional material variation fields.* The first field is designed to be constant within a distance ρ_I from the crack tip. The field is then constructed as

$$\delta\boldsymbol{\gamma}^{\text{UNI}}(r) = q(r)\mathbf{g}_1(0). \tag{19}$$

This field satisfies

$$\nabla\delta\boldsymbol{\gamma}^{\text{UNI}} = \mathbf{0}, \text{ for } r < \rho_I. \tag{20}$$

Figure 4a shows its stream traces alongside a circular arc crack.

- (2) *Tangential material variation fields.* The second field is designed to be tangential to the crack and is given by

$$\delta\boldsymbol{\gamma}^{\text{TAN}}(r) = q(r)\mathbf{g}_1(r). \tag{21}$$

This field satisfies

$$\delta\boldsymbol{\gamma}^{\text{TAN}} \cdot \mathbf{n} = 0 \text{ on } \mathcal{C}_\rho^\pm. \tag{22}$$

The stream traces of $\delta\boldsymbol{\gamma}^{\text{TAN}}$, for the particular case of a circular arc crack, are shown in Figure 4b.

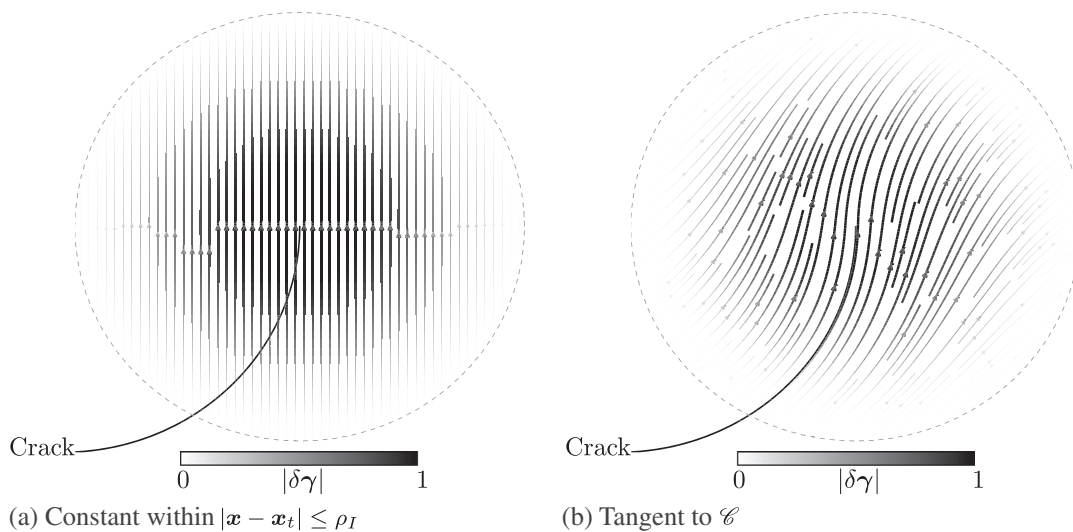


Figure 4. Stream traces for the material variation fields: (a) $\delta\boldsymbol{\gamma}^{\text{UNI}}$ and (b) $\delta\boldsymbol{\gamma}^{\text{TAN}}$. (a) Constant within $|\mathbf{x} - \mathbf{x}_t| \leq \rho_I$ and (b) tangent to \mathcal{C} .

Remark (Regularity of $\delta\mathbf{y}$)

Note that because $\Gamma \in C^2(\mathbb{R}_0^+)$ and $q \in C^2(\mathbb{R}_0^+)$, both $\delta\mathbf{y}^{\text{UNI}}$ and $\delta\mathbf{y}^{\text{TAN}}$ satisfy the continuity requirement of \mathcal{M} , namely, both are in $C^1(\mathcal{B}_{\mathcal{C}}; \mathbb{R}^2)$.

3.3.2. *Auxiliary fields.* As discussed in Section 3.1, the objective is to construct tensor fields

$$\boldsymbol{\beta}_{I,II}^{\text{aux}} \in \mathcal{B}^b \tag{23a}$$

such that

$$\begin{aligned} K_I[\boldsymbol{\beta}_I^{\text{aux}}] &= 1, K_{II}[\boldsymbol{\beta}_I^{\text{aux}}] = 0, \\ K_I[\boldsymbol{\beta}_{II}^{\text{aux}}] &= 0, K_{II}[\boldsymbol{\beta}_{II}^{\text{aux}}] = 1. \end{aligned} \tag{23b}$$

For a crack that is straight near the tip, namely, $\mathcal{C} \cap B_\rho(\mathbf{x}_t)$ is straight, a natural choice is the strain fields of the solutions to pure modes I and II loading [13]. In fact, these solutions, appropriately scaled, satisfy (23b) and the regularity requirement (23a). Furthermore, the stress field $\boldsymbol{\sigma}(\boldsymbol{\beta}_{I,II}^{\text{aux}})$ is divergence-free, and the fields $\boldsymbol{\beta}_{I,II}^{\text{aux}}$ are compatible, that is,

$$\nabla \cdot \boldsymbol{\sigma}(\boldsymbol{\beta}_{I,II}^{\text{aux}}) = \mathbf{0} \quad \text{in } B_\rho(\mathbf{x}_t) \setminus \mathcal{C}, \tag{23c}$$

$$\exists \boldsymbol{\Phi}: B_\rho(\mathbf{x}_t) \setminus \mathcal{C} \rightarrow \mathbb{R}^2 \text{ such that } \boldsymbol{\beta}_{I,II}^{\text{aux}} = \nabla \boldsymbol{\Phi} \quad \text{in } B_\rho(\mathbf{x}_t) \setminus \mathcal{C}, \tag{23d}$$

as they are indeed derived from gradients of vector fields. Additionally, the stress field is traction-free on the crack faces:

$$\boldsymbol{\sigma}(\boldsymbol{\beta}_{I,II}^{\text{aux}}) \mathbf{n} = \mathbf{0} \quad \text{on } \mathcal{C}_\rho^\pm. \tag{23e}$$

These features allow for significant simplifications of the interaction integral functional in (17).

For curvilinear cracks, however, analytically obtaining auxiliary fields with the same features is not generally possible, because a field that satisfies all conditions (23) is the solution of problem (1) in the neighborhood of \mathbf{x}_t for the given curvilinear crack geometry \mathcal{C} . Instead, we will construct auxiliary fields that, although sufficiently regular and satisfying (23b), may violate (23c), (23d), or (23e). Needless to say that doing so hinders the simplification of the interaction integral functional, as discussed in Section 3.4.

In the following, we discuss two constructs of the auxiliary fields that satisfy (23a) and (23b): (1) we present a compatible $\boldsymbol{\beta}_{I,II}^{\text{aux}}$ with divergence-free stress field $\boldsymbol{\sigma}(\boldsymbol{\beta}_{I,II}^{\text{aux}})$, but for which $\boldsymbol{\sigma}(\boldsymbol{\beta}_{I,II}^{\text{aux}})$ is not traction-free on the crack faces and (2) then we introduce a variant of $\boldsymbol{\beta}_{I,II}^{\text{aux}}$ that is incompatible and whose stress field is not divergence-free, but its stress field is traction-free on the crack faces.

- (1) *Divergence-free and compatible (DFC) fields.* We first construct an auxiliary field that satisfies conditions (23a), (23b), (23c), and (23d).

To this extent, consider the displacement fields obtained for a straight crack in pure modes I and II loading given by $\mathbf{u}^{I,II} = \sum_{i,j} u_{ij}^{I,II} \mathbf{g}_i(0) \otimes \mathbf{g}_j(0)$, where, for completeness, the components $u_{ij}^{I,II}$ are recapitulated in Appendix A. The auxiliary fields are then taken as

$$\boldsymbol{\beta}_{I,II}^{\text{aux}}(r, \vartheta) := \boldsymbol{\beta}_{I,II}^{\text{DFC}}(r, \vartheta) = \nabla \mathbf{u}^{I,II}(r, \vartheta), \tag{24}$$

where for each r , the domain of definition of ϑ is $[-\pi - \zeta(r), \pi - \zeta(r)]$ as introduced in Section 2.2, rather than $[-\pi, \pi]$.

- (2) *Traction-free fields.* We now construct auxiliary fields such that (23a), (23b), and (23e) are satisfied.

Consider the mapping $\varphi: D_\rho \rightarrow [-\pi, \pi]$ of the angular component of the polar coordinate system introduced in Section 2.2. This mapping is designed to take a value of $\pm\pi$ on the crack faces and can be constructed as

$$\varphi(r, \vartheta) = \vartheta + \zeta(r).$$

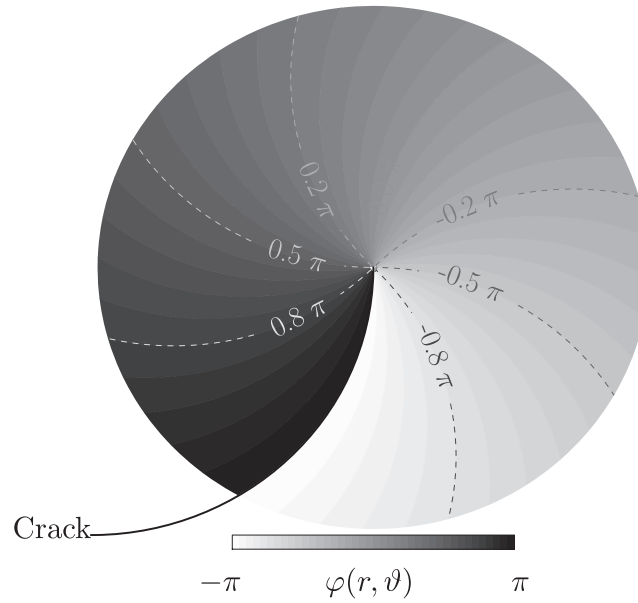


Figure 5. The mapping $\varphi(r, \vartheta)$.

Values of φ are plotted for a circular arc crack geometry in Figure 5.

We then construct $\beta_{I,II}^{\text{aux}}$ as

$$\beta_{I,II}^{\text{aux}}(r, \vartheta) := \beta_{I,II}^{\text{TF}}(r, \vartheta) = \sum_{i,j} \left[\nabla \mathbf{u}^{I,II} \Big|_{(r, \varphi(r, \vartheta))} : \mathbf{g}_i(0) \otimes \mathbf{g}_j(0) \right] \mathbf{g}_i(r) \otimes \mathbf{g}_j(r). \quad (25)$$

This auxiliary field is well defined for $r \in [0, \rho)$, where $\zeta(r)$ is also well defined. Its values for $r > \rho$ do not participate in the interaction integral, because of the support of $\delta\gamma$, and hence are immaterial. In the succeeding texts, we show that $\beta_{I,II}^{\text{TF}} \in \text{span} \{ \nabla \mathbf{u}^I, \nabla \mathbf{u}^{II} \} \oplus H^1(B_\rho(\mathbf{x}_t); \mathbb{R}^{2 \times 2})$ and hence that it can be extended to a function $\beta_{I,II}^{\text{TF}} \in \text{span} \{ \nabla \mathbf{u}^I, \nabla \mathbf{u}^{II} \} \oplus H^1(\mathcal{B}_\rho; \mathbb{R}^{2 \times 2}) = \mathcal{B}^b$.

The inspiration behind this construct is to transport $\nabla \mathbf{u}^{I,II}$ from the straight crack faces, on which $\sigma(\nabla \mathbf{u}^{I,II})$ is traction-free, to the faces of the curvilinear crack, rotating $\nabla \mathbf{u}^{I,II}$, and hence $\sigma(\nabla \mathbf{u}^{I,II})$, precisely by the angle between $\mathbf{g}_1(r)$ and $\mathbf{g}_1(0)$. This is generally an incompatible field with non-divergence-free stresses but traction-free crack faces.

Justification (traction-free property)

We begin by computing the stresses from the constitutive relation (2) on both sides of (25). Let then $\sigma^{I,II} : \mathbb{R}^+ \times (-\pi, \pi) \rightarrow \mathbb{R}^{2 \times 2}$ denote $\sigma(\nabla \mathbf{u}^{I,II})$, which are precisely the stress fields of a straight crack (Appendix A) parallel to the local crack tip basis vector $\mathbf{g}_1(0)$. These stress fields are traction-free along these straight faces, so $\sigma^{I,II}(r, \pm\pi)\mathbf{g}_2(0) = \mathbf{0}$. Then, on \mathcal{C}_ρ^\pm , we have

$$\begin{aligned} \sigma(\beta_{I,II}^{\text{TF}}) \mathbf{n} &= \sum_{i,j} [\sigma^{I,II}(r, \varphi(r, \vartheta)) : \mathbf{g}_i(0) \otimes \mathbf{g}_j(0)] \mathbf{g}_i(r) \otimes \mathbf{g}_j(r) \mathbf{n} \\ &= \mp \sum_{i,j} [\mathbf{g}_i(0) \cdot \sigma^{I,II}(r, \pm\pi) \mathbf{g}_j(0)] \mathbf{g}_i(r) \delta_{j2} = \mathbf{0}, \end{aligned} \quad (26)$$

where we used that $\varphi = \pm\pi$ on \mathcal{C}_ρ^\pm .

Justification (Regularity of β^{TF})

It is not *a priori* apparent that $\beta^{\text{TF}} \in \mathcal{B}^b$, but it does. To prove $\beta^{\text{TF}} \in \mathcal{B}^b$, first, note that $\beta_{I,II}^{\text{DFC}} \in \text{span} \{ \nabla \mathbf{u}^I, \nabla \mathbf{u}^{II} \}$. It is then enough to show that $\beta_S := \beta_{I,II}^{\text{TF}} - \beta_{I,II}^{\text{DFC}} \in H^1(B_\rho(\mathbf{x}_t); \mathbb{R}^{2 \times 2})$ and hence that it can be extended to a function $\beta_S \in H^1(\mathcal{B}_\rho; \mathbb{R}^{2 \times 2})$. To this end, we write

$$\beta_S = \sum_{i,j} [\nabla \mathbf{u}^{I,II}(r, \vartheta + \zeta(r)) : \mathbf{g}_i(0) \otimes \mathbf{g}_j(0) - \nabla \mathbf{u}^{I,II}(r, \vartheta) : \mathbf{g}_i(r) \otimes \mathbf{g}_j(r)] \mathbf{g}_i(r) \otimes \mathbf{g}_j(r), \tag{27}$$

where $\zeta(r)$ for $r > 0$ satisfies that

$$\cos \zeta(r) = s(r) := -\frac{\Gamma'(0)}{|\Gamma'(0)|} \cdot \frac{\Gamma(0) - \Gamma(r)}{|\Gamma(0) - \Gamma(r)|},$$

and $\zeta(0) = 0 = \lim_{r \rightarrow 0^+} \zeta(r)$. Hence,

$$\begin{aligned} \zeta'(r) &= \pm \frac{d}{dr} \arccos s(r) = \mp \frac{s'(r)}{\sqrt{1-s(r)^2}} \\ &= \mp \frac{1}{|\Gamma'(0)|} \left[\frac{\Gamma'(0) \cdot \Gamma'(r)}{|\Gamma(r) - \Gamma(0)|} - \frac{\Gamma'(0) \cdot [\Gamma(r) - \Gamma(0)] \Gamma'(r) \cdot [\Gamma(r) - \Gamma(0)]}{|\Gamma(r) - \Gamma(0)|^3} \right] \\ &\quad \times \left[1 - \left(-\frac{\Gamma'(0)}{|\Gamma'(0)|} \cdot \frac{\Gamma(0) - \Gamma(r)}{|\Gamma(0) - \Gamma(r)|} \right)^2 \right]^{-1/2}, \end{aligned}$$

which is well defined and continuous for any $r > 0$. A tedious calculation shows that $\zeta'(0) = \lim_{r \rightarrow 0^+} \zeta'(r)$ is also well defined and given by

$$|\zeta'(0)| = \left| \sqrt{-s''(0)} \right| = \frac{1}{2|\Gamma'(0)|^2} \left\{ |\Gamma'(0)|^2 |\Gamma''(0)|^2 - [\Gamma'(0) \cdot \Gamma''(0)]^2 \right\}^{1/2} < \infty.$$

Hence, there exists $C > 0$ such that for all $r \in [0, \rho]$,

$$|\zeta'(r)| \leq C, \tag{28a}$$

and thus

$$|\zeta(r)| \leq Cr. \tag{28b}$$

Here and henceforth, C indicates a positive constant independent of $r \in [0, \rho]$, whose value may change from line to line.

Next, as shown in Appendix A, $\nabla \mathbf{u}^{I,II}(r, \vartheta) = r^{-1/2} f^{I,II}(\theta)$, where $f^{I,II} \in C^\infty(\mathbb{R}; \mathbb{R}^{2 \times 2})$. From (27), we can write

$$\beta_S = r^{-1/2} \sum_{i,j} [f(\vartheta + \zeta(r)) : \mathbf{g}_i(0) \otimes \mathbf{g}_j(0) - f(\vartheta) : \mathbf{g}_i(r) \otimes \mathbf{g}_j(r)] \mathbf{g}_i(r) \otimes \mathbf{g}_j(r),$$

where we have omitted the superscript I, II of f , as we shall do hereafter. It is then straightforward to show that $\beta_S \in L^2(B_\rho(\mathbf{x}_t); \mathbb{R}^{2 \times 2})$.

It remains to show that

$$\frac{\partial \beta_S}{\partial r}, \frac{1}{r} \frac{\partial \beta_S}{\partial \vartheta} \in L^2(B_\rho(\mathbf{x}_t); \mathbb{R}^{2 \times 2}).$$

We first examine

$$\begin{aligned} \frac{1}{r} \frac{\partial \beta_S}{\partial \vartheta} &= r^{-3/2} \sum_{i,j} [f'(\vartheta + \zeta(r)) : \mathbf{g}_i(0) \otimes \mathbf{g}_j(0) - f'(\vartheta) : \mathbf{g}_i(r) \otimes \mathbf{g}_j(r)] \mathbf{g}_i(r) \otimes \mathbf{g}_j(r) \\ &= r^{-3/2} \sum_{i,j} \{ [f'(\vartheta + \zeta(r)) - f'(\vartheta)] : \mathbf{g}_i(0) \otimes \mathbf{g}_j(0) \\ &\quad + f'(\vartheta) : [\mathbf{g}_i(0) \otimes \mathbf{g}_j(0) - \mathbf{g}_i(r) \otimes \mathbf{g}_j(r)] \} \mathbf{g}_i(r) \otimes \mathbf{g}_j(r). \end{aligned} \tag{29}$$

Because f is C^∞ , we apply (28b) and write

$$\|f'(\vartheta + \zeta(r)) - f'(\vartheta)\|_\infty \leq \|f\|_{W^{2,\infty}} |\zeta(r)| \leq Cr. \tag{30}$$

On the other hand, we note that $\mathbf{g}_1(r) = -\Gamma'(r)/|\Gamma'(r)|$ and $\mathbf{g}_2(r) = \omega \cdot \mathbf{g}_1(r)$, where $\omega := -\mathbf{e}_1 \otimes \mathbf{e}_2 + \mathbf{e}_2 \otimes \mathbf{e}_1$, differentiating with respect to r yields

$$\mathbf{g}'_1(r) = -\frac{\Gamma''(r)}{|\Gamma'(r)|} + \frac{\Gamma'(r) \cdot \Gamma''(r)}{|\Gamma'(r)|^3} \Gamma'(r), \quad \mathbf{g}'_2(r) = \omega \cdot \mathbf{g}'_1(r),$$

for $r \in [0, \rho]$. Thus, $\mathbf{g}'_1(r)$ and $\mathbf{g}'_2(r)$ are bounded, and hence

$$\|\mathbf{g}_i(0) \otimes \mathbf{g}_j(0) - \mathbf{g}_i(r) \otimes \mathbf{g}_j(r)\|_\infty \leq Cr. \tag{31}$$

It follows from (29), (30), and (31) that $\frac{1}{r} \frac{\partial \beta_S}{\partial \vartheta} \in L^2(B_\rho(\mathbf{x}_t); \mathbb{R}^{2 \times 2})$.

Now, we compute

$$\begin{aligned} \frac{\partial \beta_S}{\partial r} &= -\frac{\beta_S}{2r} + r^{-1/2} \sum_{i,j} \{ \zeta'(r) f'(\vartheta + \zeta(r)) : \mathbf{g}_i(0) \otimes \mathbf{g}_j(0) \\ &\quad - f(\vartheta) : [\mathbf{g}'_i(r) \otimes \mathbf{g}_j(r) + \mathbf{g}_i(r) \otimes \mathbf{g}'_j(r)] \} \mathbf{g}_i(r) \otimes \mathbf{g}_j(r) \\ &\quad + r^{-1/2} \sum_{i,j} [f(\vartheta + \zeta(r)) : \mathbf{g}_i(0) \otimes \mathbf{g}_j(0) - f(\vartheta) : \mathbf{g}_i(r) \otimes \mathbf{g}_j(r)] \\ &\quad \cdot [\mathbf{g}'_i(r) \otimes \mathbf{g}_j(r) + \mathbf{g}_i(r) \otimes \mathbf{g}'_j(r)]. \end{aligned}$$

The analysis of the term β_S/r is similar to the one performed in (29), and thus $\partial \beta_S / \partial r \in L^2(B_\rho(\mathbf{x}_t); \mathbb{R}^{2 \times 2})$ follows from the boundedness of \mathbf{g}'_1 and \mathbf{g}'_2 and (28a).

3.4. Simplified expressions for the interaction integral functionals

We describe three pairs of material variation fields $\delta \boldsymbol{\gamma}$ and auxiliary fields $\boldsymbol{\beta}^b = \boldsymbol{\beta}^{\text{aux}}$, and for each pair, we provide the simplified expressions of the interaction integral functional $\mathcal{I}[\boldsymbol{\beta}, \boldsymbol{\beta}^{\text{aux}}, \delta \boldsymbol{\gamma}]$ that result from substituting the two fields. In this section, we have removed subscripts I, II from the auxiliary fields, as the following results are independent of the choice of the mode of interest and doing so clarifies the presentation.

We begin by stating two results used in obtaining the simplified expressions: (1) for traction-free auxiliary stress fields $\boldsymbol{\sigma}(\boldsymbol{\beta}^{\text{aux}})$ such as $\boldsymbol{\sigma}(\boldsymbol{\beta}^{\text{TF}})$ and tangential material variation fields such as $\delta \boldsymbol{\gamma}^{\text{TAN}}$, we have

$$\delta \boldsymbol{\gamma}^{\text{TAN}} \cdot \bar{\boldsymbol{\tau}}(\boldsymbol{\beta}, \boldsymbol{\beta}^{\text{TF}}) = -\delta \boldsymbol{\gamma}^{\text{TAN}} \cdot \boldsymbol{\beta}^{\text{TF} \top} \bar{\boldsymbol{\tau}}, \tag{32}$$

and (2) for compatible and divergence-free auxiliary fields such as $\boldsymbol{\beta}^{\text{DFC}}$, we have

$$\bar{\boldsymbol{\lambda}}(\boldsymbol{\beta}, \boldsymbol{\beta}^{\text{DFC}}) = \boldsymbol{\beta}^{\text{DFC} \top} \mathbf{b}. \tag{33}$$

Justification (Equations (32) and (33))

We begin with (32). Recalling expression (15) for $\bar{\boldsymbol{\tau}}(\boldsymbol{\beta}, \boldsymbol{\beta}^{\text{aux}})$, we have, over \mathcal{C}_ρ^\pm ,

$$\delta \boldsymbol{\gamma} \cdot \bar{\boldsymbol{\tau}}(\boldsymbol{\beta}, \boldsymbol{\beta}^{\text{aux}}) = \bar{w}(\boldsymbol{\beta}, \boldsymbol{\beta}^{\text{aux}}) \delta \boldsymbol{\gamma} \cdot \mathbf{n} - \delta \boldsymbol{\gamma} \cdot \boldsymbol{\beta}^\top \boldsymbol{\sigma}(\boldsymbol{\beta}^{\text{aux}}) \mathbf{n} - \delta \boldsymbol{\gamma} \cdot \boldsymbol{\beta}^{\text{aux} \top} \bar{\boldsymbol{\tau}}. \tag{34}$$

Because we assumed that $\delta \boldsymbol{\gamma}$ is a tangential material variation field ($\delta \boldsymbol{\gamma} \cdot \mathbf{n} = 0$) and because $\boldsymbol{\sigma}(\boldsymbol{\beta}^{\text{aux}})$ is traction-free ($\boldsymbol{\sigma}(\boldsymbol{\beta}^{\text{aux}}) \mathbf{n} = 0$ on \mathcal{C}_ρ^\pm), then (32) holds.

Next, we look at (33). Recall that, from (16),

$$\bar{\boldsymbol{\lambda}}(\boldsymbol{\beta}, \boldsymbol{\beta}^{\text{aux}}) = \boldsymbol{\beta} : \nabla \boldsymbol{\sigma}(\boldsymbol{\beta}^{\text{aux}}) - \boldsymbol{\sigma}(\boldsymbol{\beta}) : (\nabla \boldsymbol{\beta}^{\text{aux}})^\top - \boldsymbol{\beta}^\top \nabla \cdot \boldsymbol{\sigma}(\boldsymbol{\beta}^{\text{aux}}) + \boldsymbol{\beta}^{\text{aux} \top} \mathbf{b}.$$

Because we assumed that $\sigma(\beta^{\text{aux}})$ is divergence-free, the third term of the preceding equation vanishes. Furthermore, because we assumed that β^{aux} is compatible, there exists $\Phi: B_\rho(\mathbf{x}_t) \setminus \mathcal{C} \rightarrow \mathbb{R}^2$ such that $\beta^{\text{aux}} = \nabla\Phi$. Exploiting the major and minor symmetries of the constitutive tensor \mathbb{C} , we have

$$\beta : \nabla\sigma(\beta^{\text{aux}}) - \sigma(\beta) : \nabla\beta^{\text{aux}\top} = \beta : \mathbb{C} : \nabla\nabla\Phi - \beta : \mathbb{C} : \nabla\nabla\Phi = \mathbf{0}.$$

Thus, (33) holds.

We now present the simplified expressions for the functional obtained for each pair and for the particular case of rectilinear cracks, in order to re-connect these results with what is commonly found in the literature.

- (1) *Unidirectional material variation with divergence-free auxiliary fields.* We set $\delta\boldsymbol{\gamma} = \delta\boldsymbol{\gamma}^{\text{UNI}}$ and $\beta^{\text{aux}} = \beta^{\text{DFC}}$. Then, (20) implies that the domain of integration of \mathcal{I}_2 reduces to $[B_\rho(\mathbf{x}_t) \setminus B_{\rho_I}(\mathbf{x}_t)] \setminus \mathcal{C}$. Substituting (33) in (17) then simplifies to

$$\begin{aligned} \mathcal{I}[\beta, \beta^{\text{DFC}}, \delta\boldsymbol{\gamma}^{\text{UNI}}] &= \int_{\mathcal{C}_\rho^\pm} \delta\boldsymbol{\gamma}^{\text{UNI}} \cdot \bar{\boldsymbol{\tau}}(\beta, \beta^{\text{DFC}}) dS - \int_{B_\rho(\mathbf{x}_t) \setminus \mathcal{C}} \beta^{\text{DFC}\top} \mathbf{b} \cdot \delta\boldsymbol{\gamma}^{\text{UNI}} dV \\ &\quad - \int_{[B_\rho(\mathbf{x}_t) \setminus B_{\rho_I}(\mathbf{x}_t)] \setminus \mathcal{C}} \bar{\boldsymbol{\Sigma}}(\beta, \beta^{\text{DFC}}) : \nabla\delta\boldsymbol{\gamma}^{\text{UNI}} dV. \end{aligned} \tag{35}$$

- (2) *Tangential material variation with divergence-free auxiliary fields.* A slight variation of the previous pairing is the combination $\delta\boldsymbol{\gamma} = \delta\boldsymbol{\gamma}^{\text{TAN}}$ and $\beta^{\text{aux}} = \beta^{\text{DFC}}$. Applying (34), (22), and (33) yields

$$\begin{aligned} \mathcal{I}[\beta, \beta^{\text{DFC}}, \delta\boldsymbol{\gamma}^{\text{TAN}}] &= - \int_{\mathcal{C}_\rho^\pm} [\delta\boldsymbol{\gamma}^{\text{TAN}} \cdot \beta^\top \sigma(\beta^{\text{DFC}}) \mathbf{n} + \delta\boldsymbol{\gamma}^{\text{TAN}} \cdot \beta^{\text{DFC}\top} \bar{\boldsymbol{\tau}}] dS \\ &\quad - \int_{B_\rho(\mathbf{x}_t) \setminus \mathcal{C}} [\bar{\boldsymbol{\Sigma}}(\beta, \beta^{\text{DFC}}) : \nabla\delta\boldsymbol{\gamma}^{\text{TAN}} + \beta^{\text{DFC}\top} \mathbf{b} \cdot \delta\boldsymbol{\gamma}^{\text{TAN}}] dV. \end{aligned} \tag{36}$$

- (3) *Tangential material variation with traction-free auxiliary fields.* Here, we employ $\delta\boldsymbol{\gamma} = \delta\boldsymbol{\gamma}^{\text{TAN}}$ and $\beta^{\text{aux}} = \beta^{\text{TF}}$. Applying (34), (22), and (26) leads to

$$\begin{aligned} \mathcal{I}[\beta, \beta^{\text{TF}}, \delta\boldsymbol{\gamma}^{\text{TAN}}] &= - \int_{\mathcal{C}_\rho^\pm} \delta\boldsymbol{\gamma}^{\text{TAN}} \cdot \beta^{\text{TF}\top} \bar{\boldsymbol{\tau}} dS \\ &\quad - \int_{B_\rho(\mathbf{x}_t) \setminus \mathcal{C}} [\bar{\boldsymbol{\Sigma}}(\beta, \beta^{\text{TF}}) : \nabla\delta\boldsymbol{\gamma}^{\text{TAN}} + \bar{\boldsymbol{\lambda}}(\beta, \beta^{\text{TF}}) \cdot \delta\boldsymbol{\gamma}^{\text{TAN}}] dV. \end{aligned} \tag{37}$$

- (4) *Locally rectilinear cracks.* Finally, it is worth noting that in the particular case of a locally linear crack geometry, that is, $\Gamma''(r) = 0, \forall r \in [0, \rho]$, $\delta\boldsymbol{\gamma} = \delta\boldsymbol{\gamma}^{\text{TAN}} = \delta\boldsymbol{\gamma}^{\text{UNI}}$ and $\beta^{\text{aux}} = \beta^{\text{DFC}} = \beta^{\text{TF}}$, the interaction integrals of (35), (36), and (37) all simplify to

$$\begin{aligned} \mathcal{I}[\beta, \beta^{\text{aux}}, \delta\boldsymbol{\gamma}] &= - \int_{\mathcal{C}_\rho^\pm} \delta\boldsymbol{\gamma} \cdot \beta^{\text{aux}\top} \bar{\boldsymbol{\tau}} dS - \int_{[B_\rho(\mathbf{x}_t) \setminus B_{\rho_I}(\mathbf{x}_t)] \setminus \mathcal{C}} \bar{\boldsymbol{\Sigma}}(\beta, \beta^{\text{aux}}) : \nabla\delta\boldsymbol{\gamma} dV \\ &\quad - \int_{B_\rho(\mathbf{x}_t) \setminus \mathcal{C}} \beta^{\text{aux}\top} \mathbf{b} \cdot \delta\boldsymbol{\gamma} dV, \end{aligned}$$

which is the traditional expression of the interaction integral for a straight crack first introduced in [15] and commonly found in the literature.

The presence of singularities in some of the factors in each one of the terms in (17) raises the question of whether the integrals therein are well defined. For the choices of material variation and auxiliary fields previously, the three terms are in fact integrable. It is straightforward to see that \mathcal{I}_2 is integrable, and the integrability of \mathcal{I}_1 and \mathcal{I}_3 is discussed in the succeeding texts.

Justification (Integrability of \mathcal{I}_1)

We show the integrability of \mathcal{I}_1 by considering each term of $\bar{\tau}$ (cf. 15). For the first term of $\bar{\tau}$, notice that $\bar{w}(\boldsymbol{\beta}, \boldsymbol{\beta}^{\text{aux}}) = O(r^{-1})$ and $\delta\boldsymbol{\gamma}^{\text{UNI}} \cdot \mathbf{n} = O(r)$ as $r \rightarrow 0$, and hence the first term in $\delta\boldsymbol{\gamma} \cdot \bar{\tau}$ asymptotically behaves as a constant near the crack tip. For the second term of $\bar{\tau}$, we only need to consider the case $\boldsymbol{\beta}^{\text{aux}} = \boldsymbol{\beta}^{\text{DFC}}$. Notice that on the crack faces, $\vartheta = \pm\pi - \zeta(r)$, and therefore $\cos(\vartheta/2), \cos(3\vartheta/2) = O(r)$. Using the expressions for $\boldsymbol{\sigma}^{I,II}$ in Appendix A, this implies that

$$\boldsymbol{\sigma}(\boldsymbol{\beta}^{\text{DFC}}) : \mathbf{g}_1(0) \otimes \mathbf{g}_2(0), \boldsymbol{\sigma}(\boldsymbol{\beta}^{\text{DFC}}) : \mathbf{g}_2(0) \otimes \mathbf{g}_2(0) = O(r)$$

on the crack faces near the crack tip. Moreover, because $\mathbf{n} = \mathbf{g}_2(r) = \mathbf{g}_2(0) + O(r)$ and $\mathbf{g}_1(0) \cdot \mathbf{g}_2(0) = 0$, we have $\boldsymbol{\sigma}(\boldsymbol{\beta}^{\text{DFC}}) \mathbf{n} = O(r)$. Finally, because $\boldsymbol{\beta} = O(r^{-1/2})$ close to the crack tip, we can conclude that $\boldsymbol{\beta}^\top \boldsymbol{\sigma}(\boldsymbol{\beta}^{\text{DFC}}) \mathbf{n} \sim r^{1/2}$ as $r \rightarrow 0$, and hence it is integrable. For the third term in $\bar{\tau}$, if $\bar{\mathbf{t}} \in L^\infty(\mathcal{C}_\pm)$, then $\delta\boldsymbol{\gamma} \cdot \boldsymbol{\beta}^{\text{aux}} \top \bar{\mathbf{t}} = O(r^{-1/2})$ as $r \rightarrow 0$, which is integrable as well.

Justification (Integrability of \mathcal{I}_3)

As discussed in Section 3.4, we know $\bar{\boldsymbol{\lambda}}(\boldsymbol{\beta}, \boldsymbol{\beta}^{\text{DFC}}) = \boldsymbol{\beta}^{\text{DFC}} \top \mathbf{b}$. If $\mathbf{b} \in L^\infty(\mathcal{B}_\mathcal{C})$, then $\boldsymbol{\beta}^{\text{DFC}} \top \mathbf{b} = O(r^{-1/2})$ as $r \rightarrow 0$. Therefore, for $\boldsymbol{\beta}^{\text{aux}} = \boldsymbol{\beta}^{\text{DFC}}$, \mathcal{I}_3 is integrable.

For $\boldsymbol{\beta}^{\text{aux}} = \boldsymbol{\beta}^{\text{TF}}$, we begin by taking advantage of (27) and the linearity of $\bar{\boldsymbol{\lambda}}$ in the second argument to write

$$\bar{\boldsymbol{\lambda}}(\boldsymbol{\beta}, \boldsymbol{\beta}^{\text{TF}}) = \bar{\boldsymbol{\lambda}}(\boldsymbol{\beta}, \boldsymbol{\beta}^{\text{DFC}}) + \bar{\boldsymbol{\lambda}}(\boldsymbol{\beta}, \boldsymbol{\beta}_S).$$

But from earlier discussion about the regularity of $\boldsymbol{\beta}^{\text{TF}}, \nabla \boldsymbol{\beta}_S = O(r^{-1/2})$ as $r \rightarrow 0$. Thus, it is straightforward to show that $\bar{\boldsymbol{\lambda}}(\boldsymbol{\beta}, \boldsymbol{\beta}_S) = O(r^{-1})$, and hence \mathcal{I}_3 is integrable.

3.5. Choosing the interaction integral functional to use

Before introducing the numerical approximation of the aforementioned integrals, it is worth making some remarks on which functional is best suited for a specific application.

When the crack faces are loaded, a boundary integral over the faces has to be carried out irrespective of the auxiliary fields. For this particular problem, it may be appealing to choose a pairing with $\boldsymbol{\beta}^{\text{DFC}}$ such as (35) and (36). Doing so reduces the numerical complexity of the interaction integral as $\bar{\boldsymbol{\lambda}}$ greatly simplifies (and vanishes identically in the absence of body forces).

If the crack faces are traction-free, it can be appealing to compute the value of the interaction integral merely as a domain integral, as in (37). This eliminates the need to construct quadrature rules over the crack faces. Furthermore, in the presence of body forces, the integrand $\bar{\boldsymbol{\lambda}}$ will be non-zero even with $\boldsymbol{\beta}^{\text{DFC}}$, thus requiring the computation of the domain integral. For this particular case, using $\boldsymbol{\beta}^{\text{TF}}$ will result in a computationally more efficient technique.

Remark (Omission of unidirectional material variation with traction-free auxiliary fields)

The pairing $\delta\boldsymbol{\gamma}^{\text{UNI}}$ with $\boldsymbol{\beta}^{\text{TF}}$ is omitted because it provides no advantage over other pairings. In fact, because of $\delta\boldsymbol{\gamma}^{\text{UNI}}$, we have to compute the boundary integral \mathcal{I}_1 regardless of the loads on the body. Similarly, because of $\boldsymbol{\beta}^{\text{TF}}$, we have to perform the domain integral associated with the divergence of the reciprocal energy momentum tensor regardless of the loads on the body. It is thus apparent that for this particular pairing, we do not eliminate neither \mathcal{I}_1 nor \mathcal{I}_3 , unlike for other pairings (when traction and body forces are zero). Therefore, this pairing would result in a computationally inefficient formulation, with no apparent advantage over other pairings.

4. NUMERICAL COMPUTATION OF THE INTERACTION INTEGRAL

In this section, we are concerned with the computation of the interaction integral between any of the auxiliary fields and the solution \mathbf{u} of problem (1). The solution \mathbf{u} and its gradient $\boldsymbol{\beta} = \nabla \mathbf{u}$ are going to be approximated by a convergent sequence of displacement fields $\{\mathbf{u}^h\}_h$ and strain fields $\{\boldsymbol{\beta}^h = \nabla \mathbf{u}^h\}_h$, respectively, or discrete solutions. We first give some general considerations on the

expected conditions for convergence of the computed stress intensity factors that are independent of the method adopted to compute β^h . Then, we particularize some of these results to a $\{\beta^h\}$ that stems from a sequence of finite element approximations. Additionally, we discuss some minor changes needed when the exact domain needs to be approximated as well because of the presence of curvilinear cracks. The result of this section is an algorithm to compute \mathcal{I} , summarized at the end of this section for readers interested in its implementation.

4.1. Approximation of the interaction integral

Given a sequence of discrete solutions $\beta^h \rightarrow \beta$ in a sense to be specified later, it defines a sequence of values for the interaction integral $\mathcal{I}[\beta^h, \beta^{\text{aux}}, \delta\gamma]$ and hence a sequence of approximate stress intensity factors

$$K_{I,II}^h(\beta^h) = \frac{\mathcal{I}[\beta^h, \beta_{I,II}^{\text{aux}}, \delta\gamma]}{\eta}. \tag{38}$$

For the approximate stress intensity factors to converge to the exact ones $K_{I,II}[\beta]$ as $h \searrow 0$, it is enough for \mathcal{I} to be continuous with respect to its first argument in the topology in which β^h converges to β . It is simple to see then that for the stress intensity factors computed with (37), it is enough to have $\beta^h \rightarrow \beta$ in $L^2(\mathcal{B}_\rho; \mathbb{R}^{2 \times 2})$, because these functionals do not involve integration of β over \mathcal{C}_ρ^\pm . In contrast, for the stress intensity factors computed with (35) and (36), we additionally need to request $\beta^h \rightarrow \beta$ in $L^2(\mathcal{C}_\pm; \mathbb{R}^{2 \times 2})$.

4.1.1. Finite-element-based approximations. For sequences $\{u^h\}_h$ constructed with some finite element spaces, there is an important advantage of having a functional \mathcal{I} continuous in its first argument. That is, the order of convergence of the stress intensity factors doubles the order of convergence of β^h to β [32, 33, contain related results, and see Appendix B], so the values of the stress intensity factors are a lot more accurate than the discrete solution itself. It is not difficult to check that \mathcal{I} in (37) is continuous in its first argument in $L^2(\mathcal{B}_\rho; \mathbb{R}^{2 \times 2})$. Therefore, we can conclude that if $\|\beta^h \rightarrow \beta\|_{L^2(\mathcal{B}_\rho; \mathbb{R}^{2 \times 2})} \leq Ch^k$, then $|K_{I,II}^h(\beta^h) - K_{I,II}(\beta)| \leq Ch^{2k}$, for some $C > 0, k \in \mathbb{N}$ independent of h .

The functional \mathcal{I} given by (35) or (36) is not continuous in its first argument in $L^2(\mathcal{B}_\rho; \mathbb{R}^{2 \times 2})$, because of the boundary integrals. As described in Appendix B, the result that states that the order of convergence of $K_{I,II}^h$ should double that of β^h does not apply in this case. Nevertheless, as shown later in the numerical examples, the rates of convergence seem to double as well for these two functionals.

The values of k of the numerical methods used for the numerical examples in Section 5 are 0.5 and 1, and thus these methods converge at the rates of $\mathcal{O}(h)$ and $\mathcal{O}(h^2)$, respectively. In order to achieve higher order of accuracy within the context of finite element methods, it is necessary to make use of alternative methods that can accurately resolve the stress singularity, such as [1–3]. Furthermore, for curvilinear cracks, high order approximations of the crack faces are needed to attain a corresponding order of accuracy of the method.

4.2. Discrete interaction integral functional

One of the delicate issues to be addressed in this section is the fact that for curvilinear cracks, each discrete solution is computed on an approximation of the exact domain. The precise steps to handle the difference between exact and approximate domains in finite element methods are fairly standard and hence are often skipped in the description of new methods. We decided to discuss this part with some additional detail here because of the presence of the boundary integrals. The uninterested reader could simply skip to the next section.

For each h , the discrete solution \mathbf{u}^h is computed on a domain \mathcal{B}_ℓ^h with crack faces \mathcal{C}_\pm^h . We assume that as $h \searrow 0$, the approximate domain and the approximate crack faces and their normal vectors converge to the exact ones[§]. For example, a standard isoparametric mapping will suffice.

Because the discrete solutions are defined over different domains, the interaction integral functional \mathcal{I} needs to be approximated as well (the integrals over \mathcal{B}_ℓ and \mathcal{C}_\pm could not be computed for the discrete solutions otherwise). Thus, for each h , we construct a discrete interaction integral functional $\mathcal{I}^h : \mathcal{B}_h^a \times \mathcal{B}^b \times \mathcal{M} \rightarrow \mathbb{R}$, where \mathcal{B}_h^a is defined analogously to \mathcal{B}^a , but considering \mathcal{B}_ℓ^h as the domain of the problem. Then, given a sequence of solutions \mathbf{u}^h converging to the exact solution \mathbf{u} in $H^{1\mathbb{Q}}$, we expect $\lim_{h \searrow 0} \mathcal{I}^h[\boldsymbol{\beta}^h, \boldsymbol{\beta}^{\text{aux}}, \delta\boldsymbol{\gamma}] = \mathcal{I}[\boldsymbol{\beta}^e, \boldsymbol{\beta}^{\text{aux}}, \delta\boldsymbol{\gamma}]$, for any of the $\boldsymbol{\beta}^{\text{aux}} \in \mathcal{B}^b$ and any $\delta\boldsymbol{\gamma} \in \mathcal{M}$, where $\boldsymbol{\beta}^h = \nabla \mathbf{u}^h$ and $\boldsymbol{\beta}^e = \nabla \mathbf{u}$. Equivalently, letting the approximate stress intensity factors $K_{I,II}^h : \mathcal{B}_h^a \rightarrow \mathbb{R}$ be

$$K_{I,II}^h(\boldsymbol{\beta}^h) = \frac{\mathcal{I}^h[\boldsymbol{\beta}^h, \boldsymbol{\beta}_{I,II}^{\text{aux}}, \delta\boldsymbol{\gamma}]}{\eta}, \tag{39}$$

we expect $\lim_{h \searrow 0} \boldsymbol{\beta}^h = \boldsymbol{\beta}^e$ and $\lim_{h \searrow 0} K_{I,II}^h(\boldsymbol{\beta}^h) = K_{I,II}(\boldsymbol{\beta}^e)$. These ideas are compactly shown in the following commutative diagram:

$$\begin{array}{ccc} \boldsymbol{\beta}^h & \xrightarrow{\mathcal{I}^h} & K_{I,II}^h \\ h \searrow 0 \downarrow & & \downarrow h \searrow 0 \\ \boldsymbol{\beta}^e & \xrightarrow{\mathcal{I}} & K_{I,II} \end{array}$$

The functional \mathcal{I}^h is defined as

$$\begin{aligned} \mathcal{I}^h[\boldsymbol{\beta}^h, \boldsymbol{\beta}^{\text{aux}}, \delta\boldsymbol{\gamma}] &= \sum_{g \in \mathcal{G}_\ell} \delta\boldsymbol{\gamma} \cdot \bar{\boldsymbol{\tau}}^h(\boldsymbol{\beta}^h, \boldsymbol{\beta}^{\text{aux}}) \Big|_{\mathbf{x}_g} w_g \\ &\quad - \sum_{g \in \mathcal{G}} \left[\bar{\boldsymbol{\Sigma}}(\boldsymbol{\beta}^h, \boldsymbol{\beta}^{\text{aux}}) : \nabla \delta\boldsymbol{\gamma} + \bar{\boldsymbol{\lambda}}(\boldsymbol{\beta}^h, \boldsymbol{\beta}^{\text{aux}}) \cdot \delta\boldsymbol{\gamma} \right] \Big|_{\mathbf{x}_g} w_g, \end{aligned} \tag{40}$$

where \mathcal{G} and \mathcal{G}_ℓ denote the set of quadrature points over \mathcal{B}_ℓ^h and $\mathcal{C}_\pm^h \cap B_\rho(\mathbf{x}_t)$, respectively. Each integration point g in \mathcal{G} or \mathcal{G}_ℓ has position \mathbf{x}_g and integration weight w_g . We assumed that all quadrature points over \mathcal{B}_ℓ^h belong to $\mathcal{B}_\ell^h \cap \mathcal{B}_\ell$, which is true for a small enough mesh size, to be able to evaluate $\boldsymbol{\beta}^{\text{aux}}$, which is defined over \mathcal{B}_ℓ . Additionally, we defined $\bar{\boldsymbol{\tau}}^h$ as an approximation to $\bar{\boldsymbol{\tau}}$ given by

$$\bar{\boldsymbol{\tau}}^h(\boldsymbol{\beta}^h, \boldsymbol{\beta}^{\text{aux}}) = \boldsymbol{\beta}^h : \boldsymbol{\sigma}(\boldsymbol{\beta}^{\text{aux}} \circ \mathbf{p}) \mathbf{n} \circ \mathbf{p} - \boldsymbol{\beta}^h \top \boldsymbol{\sigma}(\boldsymbol{\beta}^{\text{aux}} \circ \mathbf{p}) \mathbf{n} \circ \mathbf{p} - (\boldsymbol{\beta}^{\text{aux}} \top \bar{\boldsymbol{t}}) \circ \mathbf{p},$$

where $\mathbf{p} : \mathcal{C}_\pm^h \mapsto \mathcal{C}_\pm$ is the constant-radius projection of a point onto the crack:

$$\mathbf{p}(\mathbf{x}) := \Gamma(|\mathbf{x} - \mathbf{x}_t|). \tag{41}$$

This projection is well defined when \mathcal{C}_\pm^h and \mathcal{C}_\pm are close enough. Other projections are possible as well. This one is convenient, because it is also involved in the definition of $\boldsymbol{\beta}^{\text{TF}}$.

[§]A possible condition is that for each h , there exists a one-to-one map $\boldsymbol{\Psi}_h \in M_h = W^{1,\infty}(\mathcal{B}_\ell; \mathcal{B}_\ell^h)$ that converges to the identity in M_h at a suitable rate, with $\det \nabla \boldsymbol{\Psi}_h > \epsilon$ for some $\epsilon > 0$ uniformly in h , and such that $\mathcal{B}_\ell^h = \boldsymbol{\Psi}_h(\mathcal{B}_\ell)$ and $\mathcal{C}_\pm^h = \boldsymbol{\Psi}_h(\mathcal{C}_\pm)$. This is a type of condition for finite element approximations, and it is simply a condition on the way the approximate domains are to be constructed; we will not need to explicitly construct $\boldsymbol{\Psi}_h$ to compute the interaction integrals.

[¶]For example, $\mathbf{u} - \mathbf{u}_h \circ \boldsymbol{\Psi}_h \searrow 0$ in $H^1(\mathcal{B}_\ell; \mathbb{R}^2)$.

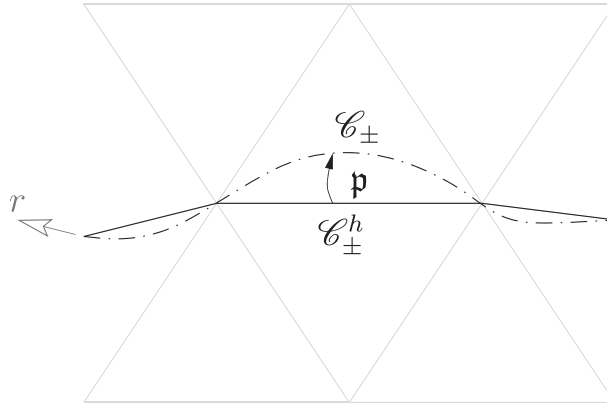


Figure 6. Example of constant radius projection for piecewise linear interpolations of \mathcal{C}_{\pm} .

Remark (Appearance of \mathbf{p} in the boundary integral)

The functional (40) is an approximation to integrals over $\mathcal{B}_{\mathcal{C}}$ and \mathcal{C}_{\pm}^h , and the quadrature points of \mathcal{G} and $\mathcal{G}_{\mathcal{C}}$ belong to them. The material variation and auxiliary fields are constructed over the exact domain $\mathcal{B}_{\mathcal{C}}$ and could be tangent or traction-free to its boundary \mathcal{C}_{\pm} , respectively, but not necessarily to its approximation \mathcal{C}_{\pm}^h . Furthermore, the traction $\bar{\mathbf{t}}$ is prescribed and only known over the exact crack \mathcal{C}_{\pm} . To address this difficulty, the auxiliary traction $\boldsymbol{\sigma}(\boldsymbol{\beta}^{\text{aux}})\mathbf{n}$, the auxiliary fields $\boldsymbol{\beta}^{\text{aux}}$ themselves, and the applied traction $\bar{\mathbf{t}}$ are evaluated at their constant radius projection onto the curved crack $\mathbf{p}(\mathbf{x})$ for $\mathbf{x} \in \mathcal{C}_{\pm}^h \cap B_{\rho}(\mathbf{x}_t)$. Note as well that $\delta\boldsymbol{\gamma} \circ \mathbf{p} = \delta\boldsymbol{\gamma}$, because $\delta\boldsymbol{\gamma}$ depends only on r .

Figure 6 shows an example for piecewise linear interpolations of the exact geometry along with its discrete approximation and the mapping \mathbf{p} in (41). As the mesh is refined, the difference between $\mathbf{p}(\mathbf{x})$ and \mathbf{x} should go to zero, and the Jacobian of the mapping $\mathbf{p} : \mathcal{C}_{\pm}^h \rightarrow \mathcal{C}_{\pm}$ should be very close to unity, thus permitting the composition in the boundary integral without introducing significant errors.

Remark (Convergence of the singular boundary integral)

Recall that if the applied crack-face traction is bounded at the origin, we expect the boundary integral of (8) to possess a singularity at the crack tip as $\boldsymbol{\beta}^{\text{aux}} \propto r^{-1/2}$ for $r \rightarrow 0$. Integrating a singular function using a standard Gaussian quadrature over a successively refined discretization was observed experimentally to lead to errors of the order $\mathcal{O}(h^{1/2})$ ([2, Appendix B]). Therefore, in the particular case in which $\bar{\mathbf{t}}$ is bounded and non-zero at \mathbf{x}_t , it is necessary to address the numerical integration of the singular function in order to preserve the expected convergence rate. Here, we computed the singular integral

$$\int_{\mathcal{C}_{\rho}^{\pm}} \delta\boldsymbol{\gamma} \cdot \boldsymbol{\beta}^{\text{aux}} \top \bar{\mathbf{t}} \, dS = \int_0^{\rho} (\delta\boldsymbol{\gamma} \cdot \boldsymbol{\beta}^{\text{aux}} \top \bar{\mathbf{t}})_{\pm}(r) |\Gamma'(r)| \, dr$$

by pulling back the integrand $(\delta\boldsymbol{\gamma} \cdot \boldsymbol{\beta}^{\text{aux}} \top \bar{\mathbf{t}})_{\pm}(r) |\Gamma'(r)|$ from $[0, \rho]$ to $\mathfrak{s}^{-1}([0, \rho])$ through the map $\mathfrak{s}(\tilde{r}) = (\tilde{r}^2/\rho - \tilde{r})q(\tilde{r}) + \tilde{r}$ with q of (18). Then, we simply use the quadrature rule $\mathcal{G}_{\mathcal{C}}$ over $\mathfrak{s}^{-1}([0, \rho])$. Namely, if we let $r_g := |\mathbf{x}_g - \mathbf{x}_t|$ for all $g \in \mathcal{G}_{\mathcal{C}}$, we compute the aforementioned integral as

$$\int_{\mathcal{C}_{\rho}^{\pm}} \delta\boldsymbol{\gamma} \cdot \boldsymbol{\beta}^{\text{aux}} \top \bar{\mathbf{t}} \, dS \approx \sum_{g \in \mathcal{G}_{\mathcal{C}}} \delta\boldsymbol{\gamma} \cdot \boldsymbol{\beta}^{\text{aux}} \top \bar{\mathbf{t}} \Big|_{\mathfrak{s}(r_g)} |\mathfrak{s}'(r_g)| w_g .$$

The mapping \mathfrak{s} effectively performs a local change of variable $r \mapsto r^2$ that removes the \sqrt{r} singularity of the integrand, thus allowing to recover optimal rates of convergence. The scaling of $1/\rho$ in \mathfrak{s} serves to ensure that the mapping \mathfrak{s} is injective over $[0, \rho]$.

Table I. Recapitulation of simplifications associated with the choice of fields.

Fields	$\mathcal{I}_1 \left(\mathcal{C}_\rho^\pm \right)$	$\mathcal{I}_2 \left(\mathcal{B}_\mathcal{C} \right)$	$\mathcal{I}_3 \left(\mathcal{B}_\mathcal{C} \right)$
$\delta \boldsymbol{\gamma}^{\text{UNI}}, \boldsymbol{\beta}^{\text{DFC}}$	–	$= 0, \forall r < \rho_I$	$\delta \boldsymbol{\gamma}^{\text{UNI}} \cdot \boldsymbol{\beta}^{\text{DFC}} \top \mathbf{b}$
$\delta \boldsymbol{\gamma}^{\text{TAN}}, \boldsymbol{\beta}^{\text{DFC}}$	$\delta \boldsymbol{\gamma} \cdot \left[\boldsymbol{\beta}^{\text{TF}} \top \bar{\boldsymbol{\tau}} - \boldsymbol{\beta}^h \top \boldsymbol{\sigma} \left(\boldsymbol{\beta}^{\text{DFC}} \right) \mathbf{n} \right]$	–	$\delta \boldsymbol{\gamma}^{\text{TAN}} \cdot \boldsymbol{\beta}^{\text{DFC}} \top \mathbf{b}$
$\delta \boldsymbol{\gamma}^{\text{TAN}}, \boldsymbol{\beta}^{\text{TF}}$	$\delta \boldsymbol{\gamma} \cdot \boldsymbol{\beta}^{\text{TF}} \top \bar{\boldsymbol{\tau}}$	–	–

A dash (–) stands for no simplification.

4.3. Summary of the method

We provide here a very concise summary of the method for the reader seeking a guideline for a rapid implementation.

The calculation of the stress intensity factors can be summarized in the following steps:

- Step 1:* Compute the approximation $\boldsymbol{\beta}^h$ to the gradient of the solution of (1).
- Step 2:* Construct $\delta \boldsymbol{\gamma}$ to be either $\delta \boldsymbol{\gamma}^{\text{UNI}}$ or $\delta \boldsymbol{\gamma}^{\text{TAN}}$ from (19) or (21), respectively.
- Step 3:* Construct $\boldsymbol{\beta}_{I,II}^{\text{aux}}$ to be either $\boldsymbol{\beta}_{I,II}^{\text{TF}}$ or $\boldsymbol{\beta}_{I,II}^{\text{DFC}}$ from (25) or (24), respectively.
- Step 4:* With the pairs $\delta \boldsymbol{\gamma}$, $\boldsymbol{\beta}_{I,II}^{\text{aux}}$ and \mathbf{b} , $\bar{\boldsymbol{\tau}}$, and $\boldsymbol{\beta}^h$ use (40) to compute the value of \mathcal{I}^h .
- Step 5:* Compute the value of $K_{I,II}^h$ with the previously \mathcal{I}^h following (38) (or (39) if appropriate).

We recapitulate in Table I the simplifications of each integrand associated with each choice of pairing of $\boldsymbol{\beta}^{\text{aux}}$ and $\delta \boldsymbol{\gamma}$.

5. NUMERICAL EXAMPLES

We next verify the proposed method through two examples. For each, we provide comparisons with analytical solutions. The first problem is concerned with a circular arc crack in an infinite medium subjected to far-field stresses. The second problem involves a power function crack subjected to crack face tractions and body forces.

For each example, we compare the convergence of the stress intensity factors for lower order methods, namely, traditional continuous Galerkin finite element methods for piecewise polynomial shape functions P^k , $k = 1, 2$, and for the higher order discontinuous Galerkin extended finite element method (DG-XFEM) [2]. Both methods are recapitulated in Section 5.1.

As discussed in Section 4, the interaction integral and hence the stress intensity factors are expected to converge at twice the rate of the derivatives of the solution. Thus, we are expecting to observe convergence of the order $\mathcal{O}(h)$ for lower order methods (whose derivatives converge as $\mathcal{O}(h^{0.5})$) and $\mathcal{O}(h^2)$ for the higher order DG-XFEM (whose derivatives converge as $\mathcal{O}(h^1)$), where h is the maximum diameter of a triangle in each mesh in the family of meshes under consideration.

In the following examples, we will provide systematic convergence curves of the error in the solution and in the computation of the stress intensity factors. Tabulated errors and computed convergence rates will accompany the curves.

We will present two error measures of the solution, one over the interior of the domain and the other over the crack faces. The error in the solutions over the interior of the domain will be measured as the L^2 -norm of the error in the gradient of displacements over $\mathcal{B}_\mathcal{C}$, and that over the crack faces will be measured as the L^2 -norm of the error in the gradient of displacement weighted by r over \mathcal{C}_\pm^h . Namely, with $\boldsymbol{\beta}^e$ denoting the analytical solution of the gradient of displacement fields, we will consider as error measures

$$\left\| \boldsymbol{\beta}^h - \boldsymbol{\beta}^e \right\|_{L^2(\mathcal{B}_\mathcal{C})} = \left[\int_{\mathcal{B}_\mathcal{C}} \sum_{i,j} \left(\beta_{ij}^h - \beta_{ij}^e \right)^2 dV \right]^{1/2} \tag{42}$$

and

$$\left\| \boldsymbol{\beta}^h - \boldsymbol{\beta}^e \circ \mathbf{p} \right\|_{L^2(\mathcal{C}_{\pm}^h, r)} = \left[\int_{\mathcal{C}_{\pm}^h} \sum_{i,j} \left(\beta_{ij}^h - \beta_{ij}^e \circ \mathbf{p} \right)^2 r \, dS \right]^{1/2} = \left\| \left(\boldsymbol{\beta}^h - \boldsymbol{\beta}^e \circ \mathbf{p} \right) \sqrt{r} \right\|_{L^2(\mathcal{C}_{\pm}^h)}. \tag{43}$$

In (43), the analytical gradient of displacements is evaluated at their constant radius projection onto the exact geometry as discussed earlier in Section 4.2.

Remark (Appearance of \sqrt{r} in the $L^2(\mathcal{C}_{\pm}^h)$ norm)

The appearance of the \sqrt{r} factor is related to the scaling of the factors that multiply $\boldsymbol{\beta}^h$ in the integrand of \mathcal{I}_1 . Namely, $\boldsymbol{\beta}^h$ appears as $\boldsymbol{\beta}^{h \top} \boldsymbol{\sigma}(\boldsymbol{\beta}^{\text{aux}}) \mathbf{n}$ and $\boldsymbol{\beta}^h : \boldsymbol{\sigma}(\boldsymbol{\beta}^{\text{aux}}) \delta \boldsymbol{\gamma} \cdot \mathbf{n}$. In the former, we have $\boldsymbol{\sigma}(\boldsymbol{\beta}^{\text{aux}}) \mathbf{n} \sim r$ as $r \rightarrow 0$, as previously discussed in Section 3.4. In the latter, we need to consider the scaling of $\delta \boldsymbol{\gamma} \cdot \mathbf{n}$, which is either $\delta \boldsymbol{\gamma}^{\text{TAN}} \cdot \mathbf{n} = 0$ or $\delta \boldsymbol{\gamma}^{\text{UNI}} \cdot \mathbf{n} \sim r$, as well as the scaling $\boldsymbol{\sigma}(\boldsymbol{\beta}^{\text{aux}}) \sim 1/\sqrt{r}$, as $r \rightarrow 0$. Hence, $\boldsymbol{\beta}^h$ in the latter case is multiplied by a factor that scales as \sqrt{r} as $r \rightarrow 0$. Thus, only the rate of convergence of $\sqrt{r} \boldsymbol{\beta}^h$ is needed to evaluate the rate of convergence of \mathcal{I}_1 .

The error in the stress intensity factors will be measured by the normalized absolute value of the error in the computed stress intensity factors. Namely, let $K_{I,II}^h := K_{I,II}^h[\boldsymbol{\beta}^h]$ be computed with (38) (or (39)) and $K_{I,II}^e$ be the exact (analytical) stress intensity factors. We will be concerned with the behavior of

$$\frac{\left| K_{I,II}^h - K_{I,II}^e \right|}{\left| K_{I,II}^e \right|}.$$

We will also present for each example the value of the computed stress intensity factors for various values of ρ , that is, for different supports for $\delta \boldsymbol{\gamma}$. As the interaction integral in (8) is independent of ρ , we would like to test the independence of the computed stress intensity factors on the support of $\delta \boldsymbol{\gamma}$.

Lastly, we remark that for each example, we set the material constants to $\lambda = 277.7\bar{7}$, $\mu = 2500$ ($E = 1000$, $\nu = 0.2$), and we assumed a plane strain state.

5.1. Numerical solution of the elasticity problem

We consider two types of finite element methods over a family of meshes of triangles. In the following, the superscript $(\cdot)^h$ will denote quantities associated with the discrete approximation of the problem. For each mesh in the family, the domain \mathcal{B} is approximated by $\mathcal{B}^h = \bigcup_e T^e$, the collection of open, straight triangles T^e . Let \mathbb{V} denotes the set of all vertices in the mesh. Each mesh in the family conforms to the crack, namely, a node sits at the crack tip, and there is no edge with its two vertices on different sides of the crack. To handle the displacement discontinuity across the crack, vertices that lie on \mathcal{C}^{\pm} are duplicated and so are edges whose two vertices lie on \mathcal{C}^{\pm} . The union of these edges on either side of the crack forms the piecewise linear approximation \mathcal{C}_{\pm}^h to \mathcal{C}_{\pm} , and we set $\mathcal{B}_{\mathcal{C}}^h = \mathcal{B}^h \setminus \mathcal{C}_{\pm}^h$. For convenience, we define

$$\mathbb{V}^d = \{a \in \mathbb{V} \mid \mathbf{x}_a \in \partial_d \mathcal{B}\},$$

where \mathbf{x}_a represents the position vector of vertex a . In the following examples, we let $N_a \in H^1(\mathcal{B}_{\mathcal{C}}^h)$ be the P^k shape function associated with node $a \in \mathbb{V}$, $k = 1, 2$, such that $N_a(\mathbf{x}_b) = \delta_{ab}$ for all $a, b \in \mathbb{V}$, where δ_{ab} is the Kronecker delta. Of course, for the piecewise quadratic case $k = 2$, mid-edge nodes are added.

The two methods adopted here are as follows:

(1) *Standard finite element method.* We seek an approximate solution $\mathbf{u}^h \in \mathcal{S}^h$, with

$$\mathcal{S}^h = \left\{ \sum_{a \in \mathbb{V}} N_a \mathbf{u}_a = \mathbf{u}^h \in H^1(\mathcal{B}_\ell^h; \mathbb{R}^2) \mid \mathbf{u}^h(\mathbf{x}_a) = \bar{\mathbf{u}}(\mathbf{x}_a), \forall a \in \mathbb{V}^d \right\}.$$

We further let

$$\mathcal{V}^h = \left\{ \sum_{a \in \mathbb{V}} N_a \delta \mathbf{u}_a = \delta \mathbf{u}^h \in H^1(\mathcal{B}_\ell^h; \mathbb{R}^2) \mid \delta \mathbf{u}^h(\mathbf{x}_a) = 0, \forall a \in \mathbb{V}^d \right\}.$$

The numerical approximation of \mathbf{u} is obtained by finding $\mathbf{u}^h \in \mathcal{S}^h$ such that

$$\int_{\mathcal{B}_\ell^h} \boldsymbol{\sigma}(\boldsymbol{\beta}^h) : \nabla \delta \mathbf{u}^h dV = \int_{\mathcal{B}_\ell^h} \mathbf{b} \cdot \delta \mathbf{u}^h dV + \int_{\partial_\tau \mathcal{B} \cup \mathcal{C}_\pm^h} \bar{\mathbf{t}} \cdot \delta \mathbf{u}^h dS, \quad \forall \delta \mathbf{u}^h \in \mathcal{V}^h,$$

where

$$\boldsymbol{\beta}^h = \nabla \mathbf{u}^h = \sum_{a \in \mathbb{V}} \mathbf{u}_a \otimes \nabla N_a.$$

(2) *Discontinuous Galerkin extended finite element method.*

Here, we recapitulate the method proposed in [2] with slight improvements. Let $h < (1/2)[\text{dist}(\mathbf{x}_t, \partial \mathcal{B}) - \rho]$ and r_c be such that

$$\rho + h < r_c < \text{dist}(\mathbf{x}_t, \partial \mathcal{B}) - h$$

and

$$\mathcal{B}_h^E = \overline{\bigcup \{T^e \mid \text{area}[T^e \cap B_{r_c}(\mathbf{x}_t)] > 0\}}, \quad \mathcal{B}_h^U = \overline{\mathcal{B}^h \setminus \mathcal{B}_h^E},$$

be the enriched and unenriched regions, respectively. Then, we set

$$\mathbb{V}^E = \{a \in \mathbb{V} \mid \mathbf{x}_a \in \mathcal{B}_h^E\}, \quad \mathbb{V}^U = \{a \in \mathbb{V} \mid \mathbf{x}_a \in \mathcal{B}_h^U\}.$$

Hence, there are nodes that belong to both \mathbb{V}^E and \mathbb{V}^U . In fact, let $\Gamma_h^E = \partial \mathcal{B}_h^E$, then

$$\mathbb{V}^E \cap \mathbb{V}^U = \{a \in \mathbb{V} \mid \mathbf{x}_a \in \Gamma_h^E\}.$$

The DG-XFEM is built on the following set:

$$\begin{aligned} \mathcal{S}^h = \left\{ \mathbf{u}^h \in L^2(\mathcal{B}_\ell^h; \mathbb{R}^2) \mid \mathbf{u}^h = k_I \mathbf{u}^I + k_{II} \mathbf{u}^{II} + \sum_{a \in \mathbb{V}^E} N_a \mathbf{u}_a^E \text{ in } \mathcal{B}_h^E, k_I, k_{II} \in \mathbb{R}; \right. \\ \left. \mathbf{u}^h = \sum_{a \in \mathbb{V}^U} N_a \mathbf{u}_a^U \text{ in } \mathcal{B}_h^U, \mathbf{u}^h(\mathbf{x}_a) = \bar{\mathbf{u}}(\mathbf{x}_a), \forall a \in \mathbb{V}^d \right\}. \end{aligned}$$

The corresponding test space is given by

$$\begin{aligned} \mathcal{V}^h = \left\{ \delta \mathbf{u}^h \in L^2(\mathcal{B}_\ell^h; \mathbb{R}^2) \mid \delta \mathbf{u}^h = \delta k_I \mathbf{u}^I + \delta k_{II} \mathbf{u}^{II} + \sum_{a \in \mathbb{V}^E} N_a \delta \mathbf{u}_a^E \text{ in } \mathcal{B}_h^E, \delta k_I, \delta k_{II} \in \mathbb{R}; \right. \\ \left. \delta \mathbf{u}^h = \sum_{a \in \mathbb{V}^U} N_a \delta \mathbf{u}_a^U \text{ in } \mathcal{B}_h^U, \delta \mathbf{u}^h(\mathbf{x}_a) = \mathbf{0}, \forall a \in \mathbb{V}^d \right\}. \end{aligned}$$

Therefore, the kinematics of a typical function $\mathbf{u}^h \in \mathcal{S}^h$ is independent in \mathcal{B}_h^E and \mathcal{B}_h^U ; a discontinuity across Γ_h^E arises, which is defined as

$$\llbracket \mathbf{u}^h \rrbracket = \left[\mathbf{u}^h \Big|_{\Gamma_h^E} \right]_{\Gamma_h^E} - \left[\mathbf{u}^h \Big|_{\mathcal{B}_h^U} \right]_{\Gamma_h^E} = [k_I \mathbf{u}^I + k_{II} \mathbf{u}^{II} + \sum_{a \in \mathbb{V}^E \cap \mathbb{V}^U} N_a (\mathbf{u}_a^E - \mathbf{u}_a^U)]_{\Gamma_h^E}.$$

This discontinuous \mathbf{u}^h is handled through a DG-derivative $D_{DG} : \mathcal{S}^h \rightarrow \nabla_h \mathcal{S}^h + \mathcal{W}^h$:

$$D_{DG} : \mathbf{u}^h \mapsto \nabla_h \mathbf{u}^h + R(\llbracket \mathbf{u}^h \rrbracket),$$

where $\nabla_h \mathbf{u}^h = \nabla \mathbf{u}^h$ in each T^e , $R(\llbracket \mathbf{u}^h \rrbracket)$ is such that

$$\int_{\mathcal{B}_\epsilon^h} R(\llbracket \mathbf{u}^h \rrbracket) : \mathbf{w}^h \, dV = - \int_{\Gamma_h^E} \llbracket \mathbf{u}^h \rrbracket \otimes \mathbf{n} : \{\mathbf{w}^h\} \, dS, \quad \forall \mathbf{w}^h \in \mathcal{W}^h,$$

where

$$\mathcal{W}^h = \prod_e [P_1(T^e)]^{2 \times 2}$$

and on Γ_h^E

$$\{\mathbf{w}^h\} = \frac{1}{2} \left(\left[\mathbf{w}^h \Big|_{\mathcal{B}_h^E} \right]_{\Gamma_h^E} + \left[\mathbf{w}^h \Big|_{\mathcal{B}_h^U} \right]_{\Gamma_h^E} \right).$$

The solution to the problem stated in Section 2.1 is approximated by finding $\mathbf{u}^h \in \mathcal{V}^h$ such that

$$\begin{aligned} \int_{\mathcal{B}_\epsilon^h} \boldsymbol{\sigma}(\boldsymbol{\beta}^h) : D_{DG} \delta \mathbf{u}^h \, dV + 2\mu\alpha \int_{\mathcal{B}_\epsilon^h} R(\llbracket \mathbf{u}^h \rrbracket) : R(\llbracket \delta \mathbf{u}^h \rrbracket) \, dV \\ = \int_{\mathcal{B}_\epsilon^h} \mathbf{b} \cdot \delta \mathbf{u}^h \, dV + \int_{\partial_\tau \mathcal{B} \cup \mathcal{C}_\pm^h} \bar{\mathbf{t}} \cdot \delta \mathbf{u}^h \, dS, \quad \forall \delta \mathbf{u}^h \in \mathcal{V}^h, \end{aligned}$$

where α can be any positive real number and

$$\boldsymbol{\beta}^h = D_{DG} \mathbf{u}^h.$$

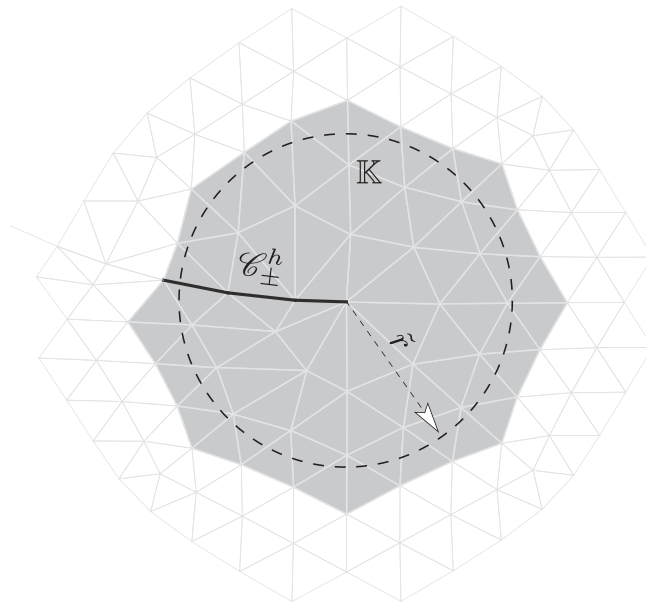


Figure 7. Example of a subset of elements \mathbb{K} in a finite element mesh. The elements in the shaded region are the elements over which the interaction integral is computed.

We conclude the section by remarking that the approximate domain of integration of the interaction integral for the particular choice of the method is given by the subset of elements with at least one vertex that lies within $B_\rho(x_I)$ that we denote by \mathbb{K} . Refer to Figure 7 for an illustration of the aforementioned subset.

Furthermore, we exploit the quadrature rule constructed over each element \mathcal{G}_{T^e} and its boundary $\mathcal{G}_{\partial T^e}$ to form \mathcal{G} and $\mathcal{G}_\mathcal{C}$, respectively. Namely, we let the numerical interaction integral, in this specific setting of finite element methods, become

$$\begin{aligned} \mathcal{I}^h[\boldsymbol{\beta}^h, \boldsymbol{\beta}^{\text{aux}}, \delta\boldsymbol{\gamma}] \approx & \sum_{T^e \in \mathbb{K}, |\partial T^e \cap \mathcal{C}_\pm^h| > 0} \sum_{g \in \mathcal{G}_{\partial T^e}} \delta\boldsymbol{\gamma} \cdot \bar{\boldsymbol{\tau}}^h(\boldsymbol{\beta}^h, \boldsymbol{\beta}^{\text{aux}})|_{x_g} w_g \\ & - \sum_{T^e \in \mathbb{K}} \sum_{g \in \mathcal{G}_{T^e}} \left[\bar{\boldsymbol{\Sigma}}(\boldsymbol{\beta}^h, \boldsymbol{\beta}^{\text{aux}}) : \nabla \delta\boldsymbol{\gamma} + \bar{\boldsymbol{\lambda}}(\boldsymbol{\beta}^h, \boldsymbol{\beta}^{\text{aux}}) \cdot \delta\boldsymbol{\gamma} \right] |_{x_g} w_g. \end{aligned}$$

5.2. Circular arc crack

We consider an infinite plate with a circular arc-shaped crack subjected to uniform tension from infinity. The analytical solution was derived in [34], and a recapitulation of the solution can be found in [35]. The resulting stress intensity factors for uniform far-field tension loading are (e.g., [36])

$$K_I^e = \frac{\sigma \cos\left(\frac{\alpha}{2}\right) \sqrt{\pi R \sin(\alpha)}}{\sin^2\left(\frac{\alpha}{2}\right) + 1}, \quad K_{II}^e = \frac{\sigma \sin\left(\frac{\alpha}{2}\right) \sqrt{\pi R \sin(\alpha)}}{\sin^2\left(\frac{\alpha}{2}\right) + 1},$$

where R is the radius of the circular arc crack, α is half the angle subtended by the crack, and σ is the far-field tension as shown in Figure 8.

Only a finite subdomain was considered, and exact tractions were specified on the boundaries. Given the symmetry of the problem, only half of the subdomain was modeled, and appropriate symmetry boundary conditions on the axis of symmetry were specified. Figure 9 shows a representation of the modeled subdomain and boundary conditions. For the simulations we took $\alpha = \pi/2$, $R = 1$, the modeled domain was given by $\mathcal{B} = [0, 2] \times [-1.5, 0.5]$, and the crack centered at the origin.

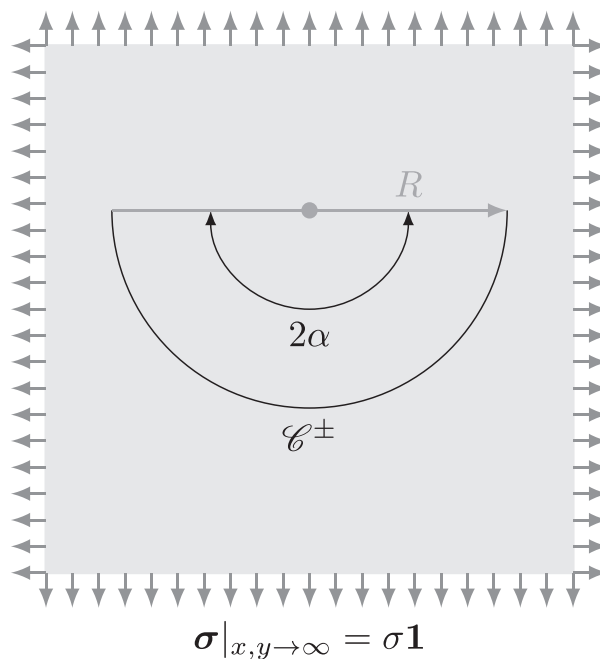


Figure 8. The circular arc crack problem.

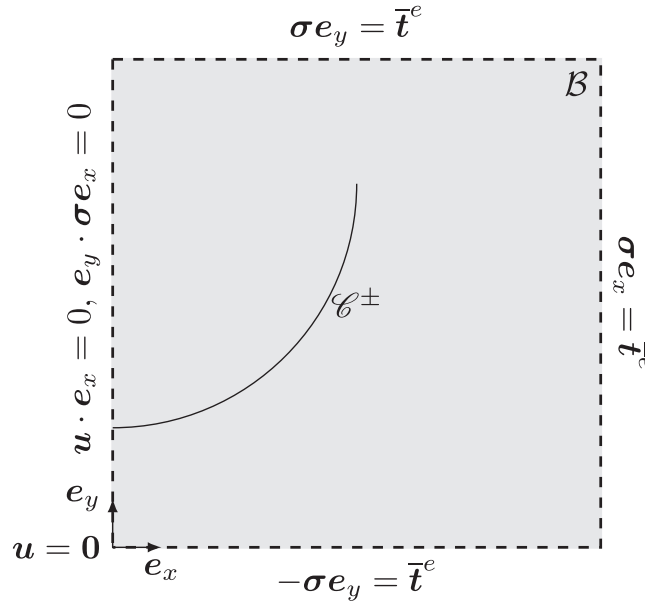


Figure 9. Modeled subdomain. Here, \mathbf{t}^e is the exact traction on a face.

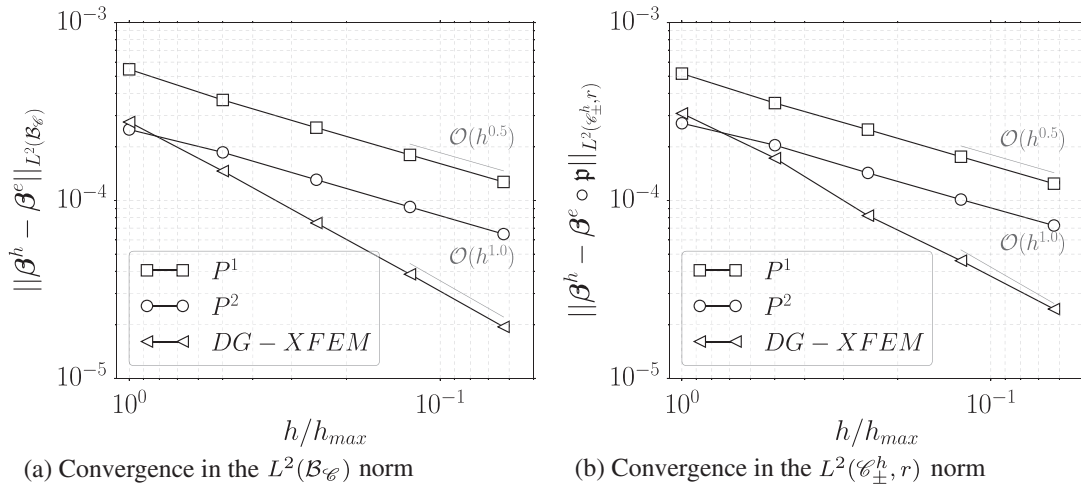


Figure 10. Convergence of the solution. (a) Convergence in the $L^2(\mathcal{B}_C)$ norm and (b) convergence in the $L^2(\mathcal{C}_\pm^h, r)$ norm. DG-XFEM, discontinuous Galerkin extended finite element method.

To establish the accuracy of the methods, the solution was computed for different levels of refinement of the discretized domain. The meshes were generated by conforming recursive subdivisions of the coarsest mesh to the exact geometry. The coarsest mesh size was $h_{max} \approx 1/6$.

The error measures (42) and (43) were observed to decrease as $\mathcal{O}(h^{1/2})$ for the lower order method and as $\mathcal{O}(h)$ for the second order method. Figure 10 shows the convergence plot of the solution, and Table II summarizes the error and the computed rates of convergence.

As expected, the error in the stress intensity factors is observed to converge with order $\mathcal{O}(h^1)$ and $\mathcal{O}(h^2)$ for the lower and higher order methods, respectively. Figure 11 provides the convergence curves for the stress intensity factors using the three pairings of material variation and auxiliary fields. Errors and computed rates are reported in Table III.

Lastly, we show that the evaluation of the interaction integral is independent of the support of $\delta\boldsymbol{\gamma}$. To this end, Figure 12 shows the error in computed stress intensity factors of the most refined

Table II. Convergence rates of the derivatives of the solution for the circular arc crack problem.

h_{\max}/h	$\ \beta^h - \beta^e\ _{L^2(\mathcal{B})}$					
	P^1		P^2		$DG - XFEM$	
	Error	\mathcal{O}	Error	\mathcal{O}	Error	\mathcal{O}
(a) Domain convergence						
1	0.00055	–	0.00025	–	0.00028	–
2	0.00037	0.57	0.00019	0.43	0.00015	0.92
4	0.00026	0.52	0.00013	0.51	0.00007	0.97
8	0.00018	0.51	0.00009	0.51	0.00004	0.96
16	0.00013	0.51	0.00006	0.51	0.00002	0.98
h_{\max}/h	$\ \beta^h - \beta^e \circ p\ _{L^2(\mathcal{C}_{\pm}^h, r)}$					
	P^1		P^2		$DG - XFEM$	
	Error	\mathcal{O}	Error	\mathcal{O}	Error	\mathcal{O}
(b) Trace convergence						
1	0.00055	–	0.00025	–	0.00028	–
2	0.00037	0.57	0.00019	0.43	0.00015	0.92
4	0.00026	0.52	0.00013	0.51	0.00007	0.97
8	0.00018	0.51	0.00009	0.51	0.00004	0.96
16	0.00013	0.51	0.00006	0.51	0.00002	0.98

DG-XFEM, discontinuous Galerkin extended finite element method.

mesh for five values of ρ/ρ_{\max} , ranging from ~ 0.7 to 1 with $\rho_{\max} = 0.5$. The independence of the interaction integral from the choice of the support of the material variation field is apparent from these results.

5.3. Power function crack

The second example we consider is the one of the power function crack $\mathcal{C} = \{(x, x^3) | x \in [0, 1]\}$ loaded by a force field \mathbf{b} and crack face traction $\bar{\mathbf{l}}$, see Figure 13.

The exact stress field is constructed by a superposition of a singular stress field $\hat{\sigma}^e$ with a bounded field $\sigma_{\mathbf{b}}$ as

$$\sigma^e = \hat{\sigma}^e + \sigma_{\mathbf{b}}.$$

The field $\hat{\sigma}^e$ is constructed as

$$\hat{\sigma}^e = (\sigma_{ij}^I + \sigma_{ij}^{II}) \mathbf{g}_i(0) \otimes \mathbf{g}_j(0),$$

where $\sigma_{ij}^{I,II}$ is given in Appendix A and is evaluated for values of $\vartheta \in [-\pi - \zeta(r), \pi - \zeta(r)]$, as discussed in Section 2.2. The bounded stress field $\sigma_{\mathbf{b}}$ is constructed as

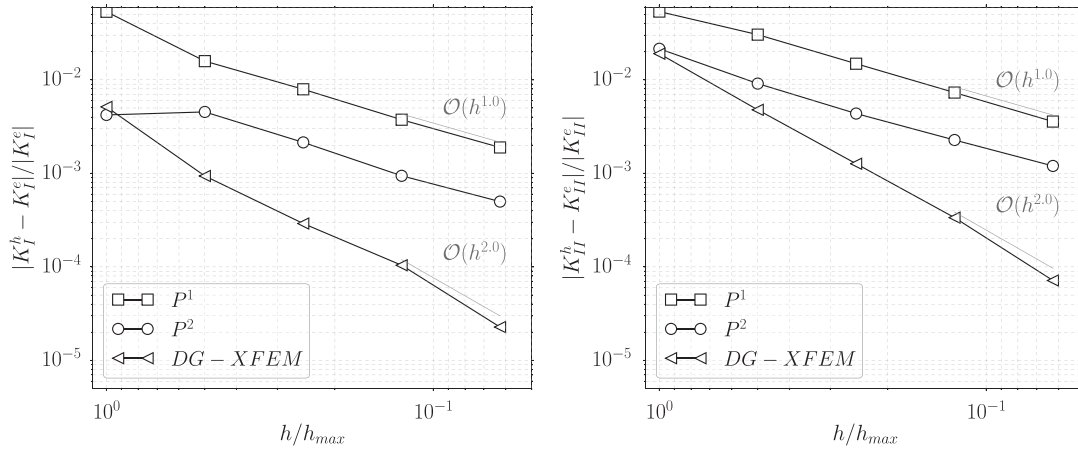
$$\sigma_{\mathbf{b}} = x \mathbf{e}_x \otimes \mathbf{e}_x + y \mathbf{e}_y \otimes \mathbf{e}_y.$$

Note that

$$\nabla \cdot \sigma^e = \nabla \cdot \hat{\sigma}^e + \nabla \cdot \sigma_{\mathbf{b}} = \nabla \cdot \sigma_{\mathbf{b}} = -\mathbf{b},$$

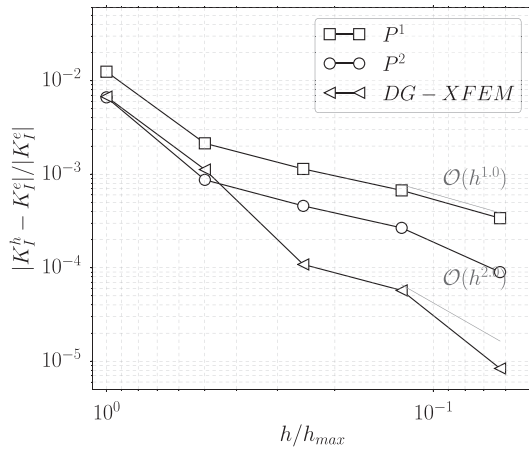
where

$$\mathbf{b} = -(\mathbf{e}_x + \mathbf{e}_y).$$

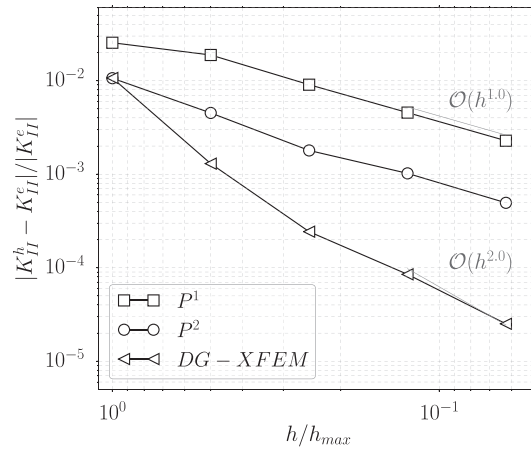


(a) $(\beta^{\text{aux}}, \delta\gamma) = (\beta_I^{\text{TF}}, \delta\gamma^{\text{TAN}})$.

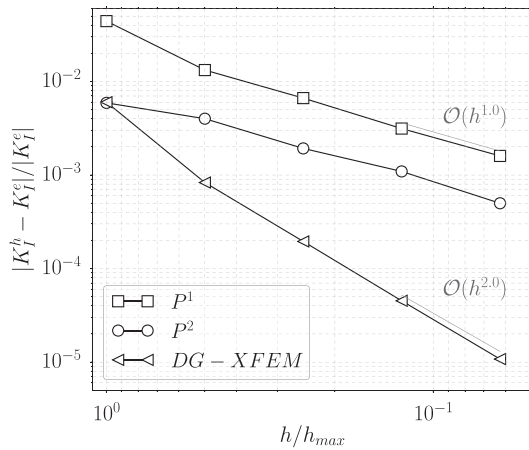
(b) $(\beta^{\text{aux}}, \delta\gamma) = (\beta_{II}^{\text{TF}}, \delta\gamma^{\text{TAN}})$.



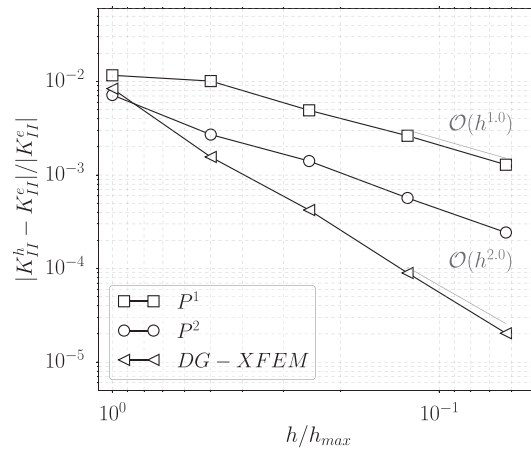
(c) $(\beta^{\text{aux}}, \delta\gamma) = (\beta_I^{\text{DFC}}, \delta\gamma^{\text{UNI}})$.



(d) $(\beta^{\text{aux}}, \delta\gamma) = (\beta_{II}^{\text{DFC}}, \delta\gamma^{\text{UNI}})$.



(e) $(\beta^{\text{aux}}, \delta\gamma) = (\beta_I^{\text{DFC}}, \delta\gamma^{\text{TAN}})$.



(f) $(\beta^{\text{aux}}, \delta\gamma) = (\beta_{II}^{\text{DFC}}, \delta\gamma^{\text{TAN}})$.

Figure 11. Convergence of the stress intensity factors for the circular arc crack. (a) $(\beta^{\text{aux}}, \delta\gamma) = (\beta_I^{\text{TF}}, \delta\gamma^{\text{TAN}})$, (b) $(\beta^{\text{aux}}, \delta\gamma) = (\beta_{II}^{\text{TF}}, \delta\gamma^{\text{TAN}})$, (c) $(\beta^{\text{aux}}, \delta\gamma) = (\beta_I^{\text{DFC}}, \delta\gamma^{\text{UNI}})$, (d) $(\beta^{\text{aux}}, \delta\gamma) = (\beta_{II}^{\text{DFC}}, \delta\gamma^{\text{UNI}})$, (e) $(\beta^{\text{aux}}, \delta\gamma) = (\beta_I^{\text{DFC}}, \delta\gamma^{\text{TAN}})$, and (f) $(\beta^{\text{aux}}, \delta\gamma) = (\beta_{II}^{\text{DFC}}, \delta\gamma^{\text{TAN}})$. DG-XFEM, discontinuous Galerkin extended finite element method.

Table III. Convergence rates for stress intensity factors.

h_{\max}/h	P^1				P^2				discontinuous Galerkin extended finite element method ($DG - XFEM$)			
	K_I		K_{II}		K_I		K_{II}		K_I		K_{II}	
	Error	\mathcal{O}	Error	\mathcal{O}	Error	\mathcal{O}	Error	\mathcal{O}	Error	\mathcal{O}	Error	\mathcal{O}
(a) Traction-free auxiliary fields ($\beta^{\text{aux}} = \beta^{\text{TF}}$) and tangential material variation ($\delta\gamma = \delta\gamma^{\text{TAN}}$).												
1/1	5e-02	–	5e-02	–	4e-03	–	2e-02	–	5e-03	–	2e-02	–
1/2	2e-02	1.75	3e-02	0.82	5e-03	0.11	9e-03	1.24	9e-04	2.45	5e-03	1.99
1/4	8e-03	1.00	1e-02	1.03	2e-03	1.08	4e-03	1.06	3e-04	1.69	1e-03	1.91
1/8	4e-03	1.08	7e-03	1.02	9e-04	1.19	2e-03	0.94	1e-04	1.48	3e-04	1.91
1/16	2e-03	0.99	4e-03	1.03	5e-04	0.92	1e-03	0.92	2e-05	2.20	7e-05	2.23
(b) Divergence-free ($\beta^{\text{aux}} = \beta^{\text{DFC}}$) and unidirectional material variation ($\delta\gamma = \delta\gamma^{\text{UNI}}$).												
1/1	1e-02	–	3e-02	–	7e-03	–	1e-02	–	7e-03	–	1e-02	–
1/2	2e-03	2.54	2e-02	0.44	9e-04	2.93	5e-03	1.23	1e-03	2.59	1e-03	3.04
1/4	1e-03	0.91	9e-03	1.05	5e-04	0.92	2e-03	1.33	1e-04	3.37	2e-04	2.42
1/8	7e-04	0.76	5e-03	0.99	3e-04	0.78	1e-03	0.81	6e-05	0.92	8e-05	1.52
1/16	3e-04	0.99	2e-03	1.00	9e-05	1.57	5e-04	1.05	8e-06	2.77	3e-05	1.76
(c) Divergence-free ($\beta^{\text{aux}} = \beta^{\text{DFC}}$) and tangential material variation ($\delta\gamma = \delta\gamma^{\text{TAN}}$).												
1/1	4e-02	–	1e-02	–	6e-03	–	7e-03	–	6e-03	–	8e-03	–
1/2	1e-02	1.74	1e-02	0.20	4e-03	0.56	3e-03	1.41	8e-04	2.85	2e-03	2.43
1/4	7e-03	1.00	5e-03	1.04	2e-03	1.06	1e-03	0.94	2e-04	2.09	4e-04	1.88
1/8	3e-03	1.08	3e-03	0.90	1e-03	0.82	6e-04	1.31	5e-05	2.11	9e-05	2.23
1/16	2e-03	0.97	1e-03	1.03	5e-04	1.13	2e-04	1.23	1e-05	2.07	2e-05	2.14

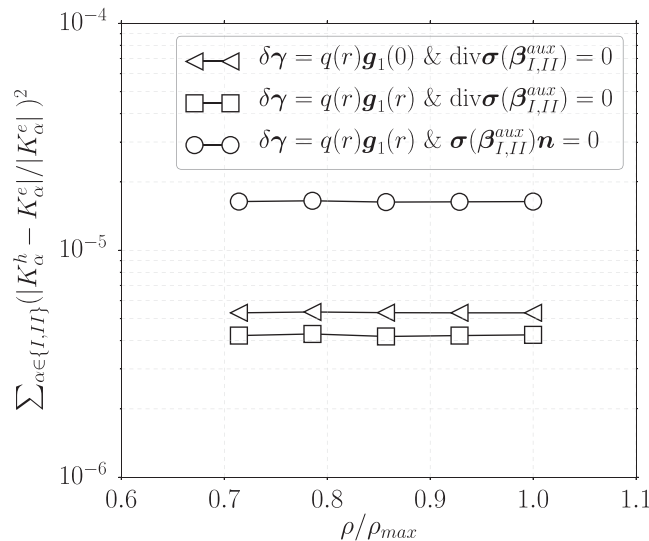


Figure 12. Contour independence of the interaction integral.

It is worth remarking that the stress intensity factors of σ^e will correspond to those of the singular field $\hat{\sigma}^e$ without any perturbation from the bounded field. In fact, given that both $\lim_{r \rightarrow 0} \sqrt{r}\hat{\sigma}^e$ and $\lim_{r \rightarrow 0} \sqrt{r}\sigma_b = \mathbf{0}$ exist, we have

$$\lim_{r \rightarrow 0} \sqrt{r}\sigma^e = \lim_{r \rightarrow 0} \sqrt{r}(\hat{\sigma}^e + \sigma_b) = \lim_{r \rightarrow 0} \sqrt{r}\sigma_{I,II} + \lim_{r \rightarrow 0} \sqrt{r}\sigma_b = \lim_{r \rightarrow 0} \sqrt{r}\sigma_{I,II}.$$

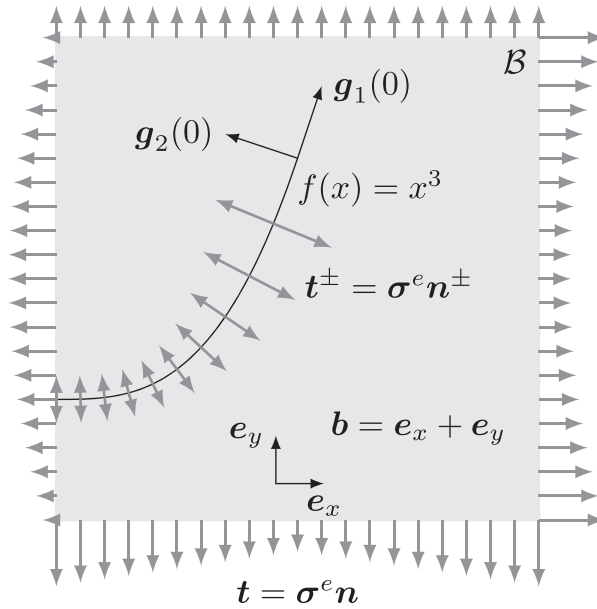


Figure 13. The power function crack problem.

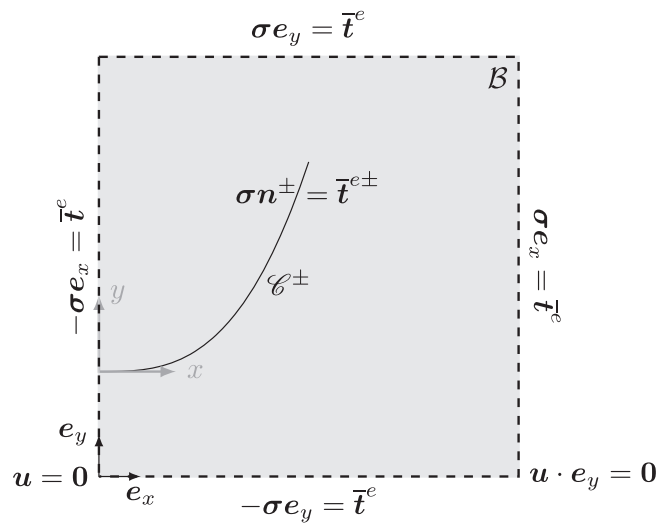


Figure 14. Modeled subdomain.

For our example, we take the stress intensity factors of $\hat{\sigma}^e$ to be $K_I^e = K_{II}^e = 1$.

Figures 13 and 14 show the schematic of the problem and the modeled domain with the applied boundary conditions, respectively. For the simulations, the modeled domain was given by $\mathcal{B} = [0, 2] \times [-0.25, 1.75]$.

Like for the previous example, we computed the solution for several levels of refinement and investigated the convergence of the computed stress intensity factors. The coarsest mesh size was $h_{max} \approx 1/6$.

The error measures (42) and (43) were observed to decrease as $\mathcal{O}(h^{1/2})$ and $\mathcal{O}(h)$ for the first and second order methods, respectively. The values are plotted in Figure 15, and the errors and the computed rates of convergence are tabulated in Table IV.

The stress intensity factors were observed to converge to the analytical value as $\mathcal{O}(h)$ and $\mathcal{O}(h^2)$ when using the solution of the first and second order methods, respectively. The values of the error

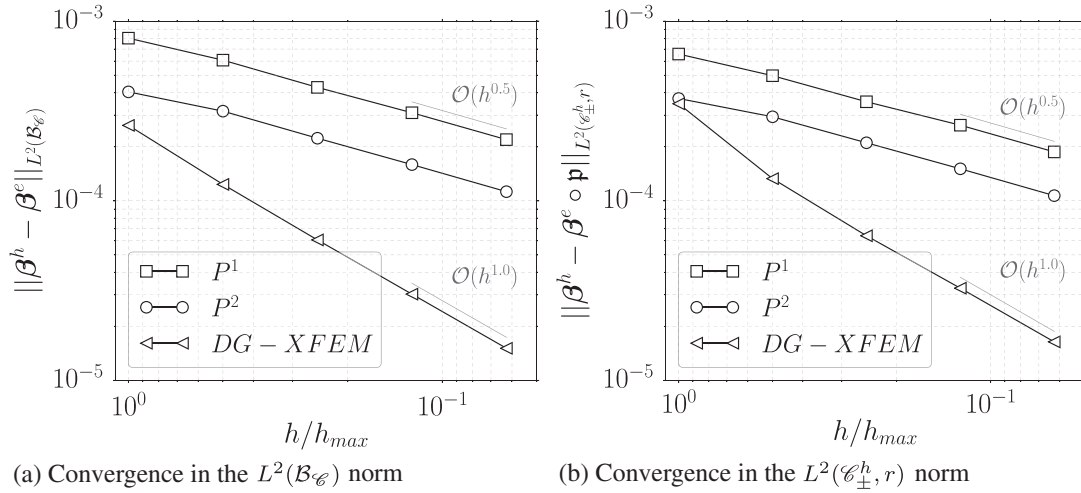


Figure 15. Convergence of the solution. (a) Convergence in the $L^2(\mathcal{B}_E)$ norm and (b) convergence in the $L^2(\mathcal{C}_{\pm}^h, r)$ norm. DG-XFEM, discontinuous Galerkin extended finite element method.

Table IV. Convergence rates of the derivatives of the solution for the circular arc crack problem.

h_{\max}/h	P^1		P^2		$DG - XFEM$	
	Error	\mathcal{O}	Error	\mathcal{O}	Error	\mathcal{O}
(a) Domain convergence						
$\ \beta^h - \beta^e\ _{L^2(\mathcal{B})}$						
1	0.00080	–	0.00040	–	0.00026	–
2	0.00061	0.41	0.00032	0.36	0.00012	1.09
4	0.00043	0.50	0.00022	0.50	0.00006	1.03
8	0.00031	0.47	0.00016	0.49	0.00003	1.00
16	0.00022	0.50	0.00011	0.50	0.00002	1.00
(b) Trace convergence						
$\ \beta^h - \beta^e \circ p\ _{L^2(\mathcal{C}_{\pm}^h, r)}$						
1	0.00066	–	0.00037	–	0.00035	–
2	0.00050	0.40	0.00029	0.34	0.00013	1.39
4	0.00036	0.48	0.00021	0.48	0.00006	1.06
8	0.00026	0.43	0.00015	0.48	0.00003	0.97
16	0.00019	0.49	0.00011	0.50	0.00002	0.99

DG-XFEM, discontinuous Galerkin extended finite element method.

in the stress intensity factors are plotted in Figure 16, and the errors and the computed convergence rates are provided in Table V.

Lastly, in Figure 17, we illustrate the independence of the interaction integral on the size of the support of $\delta\gamma$ by plotting the stress intensity factors for five values of ρ/ρ_{\max} ranging from ~ 0.7 to 1, for $\rho_{\max} = 0.5$.

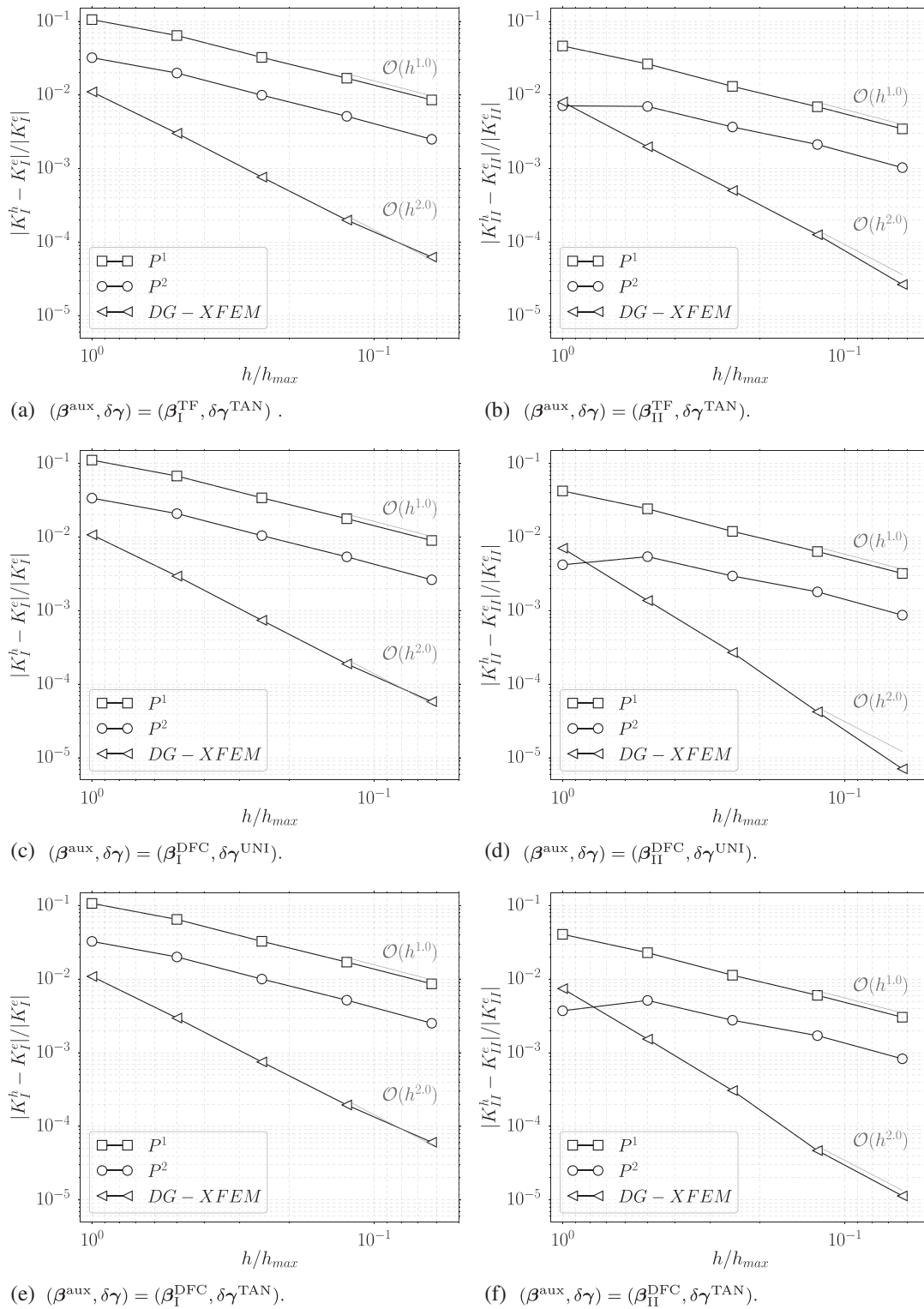


Figure 16. Convergence of the stress intensity factors for the power function crack problem. (a) $(\beta^{\text{aux}}, \delta\gamma) = (\beta_I^{\text{TF}}, \delta\gamma^{\text{TAN}})$, (b) $(\beta^{\text{aux}}, \delta\gamma) = (\beta_{II}^{\text{TF}}, \delta\gamma^{\text{TAN}})$, (c) $(\beta^{\text{aux}}, \delta\gamma) = (\beta_I^{\text{DFC}}, \delta\gamma^{\text{UNI}})$, (d) $(\beta^{\text{aux}}, \delta\gamma) = (\beta_{II}^{\text{DFC}}, \delta\gamma^{\text{UNI}})$, (e) $(\beta^{\text{aux}}, \delta\gamma) = (\beta_I^{\text{DFC}}, \delta\gamma^{\text{TAN}})$, and (f) $(\beta^{\text{aux}}, \delta\gamma) = (\beta_{II}^{\text{DFC}}, \delta\gamma^{\text{TAN}})$. DG-XFEM, discontinuous Galerkin extended finite element method.

Table V. Convergence rates for stress intensity factors.

h_{\max}/h	P^1				P^2				$DG - XFEM$			
	K_I		K_{II}		K_I		K_{II}		K_I		K_{II}	
	Error	\mathcal{O}	Error	\mathcal{O}	Error	\mathcal{O}	Error	\mathcal{O}	Error	\mathcal{O}	Error	\mathcal{O}
(a) Traction-free auxiliary fields ($\beta^{\text{aux}} = \beta^{\text{TF}}$) and tangential material variation ($\delta\gamma = \delta\gamma^{\text{TAN}}$).												
1/1	1e-01	–	5e-02	–	3e-02	–	7e-03	–	1e-02	–	8e-03	–
1/2	6e-02	0.72	3e-02	0.81	2e-02	0.69	7e-03	0.03	3e-03	1.87	2e-03	2.02
1/4	3e-02	0.99	1e-02	1.01	1e-02	1.00	4e-03	0.93	8e-04	1.98	5e-04	1.98
1/8	2e-02	0.94	7e-03	0.92	5e-03	0.95	2e-03	0.79	2e-04	1.94	1e-04	2.00
1/16	9e-03	0.98	3e-03	1.00	2e-03	1.04	1e-03	1.04	6e-05	1.67	3e-05	2.25
(b) Divergence-free ($\beta^{\text{aux}} = \beta^{\text{DFC}}$) and unidirectional material variation ($\delta\gamma = \delta\gamma^{\text{UNI}}$).												
1/1	1e-01	–	4e-02	–	3e-02	–	4e-03	–	1e-02	–	7e-03	–
1/2	7e-02	0.72	2e-02	0.81	2e-02	0.70	5e-03	0.37	3e-03	1.86	1e-03	2.36
1/4	3e-02	0.99	1e-02	1.02	1e-02	0.99	3e-03	0.87	7e-04	2.00	3e-04	2.35
1/8	2e-02	0.94	6e-03	0.91	5e-03	0.96	2e-03	0.72	2e-04	1.97	4e-05	2.68
1/16	9e-03	0.98	3e-03	0.99	3e-03	1.04	9e-04	1.05	6e-05	1.70	7e-06	2.58
(c) Divergence-free ($\beta^{\text{aux}} = \beta^{\text{DFC}}$) and tangential material variation ($\delta\gamma = \delta\gamma^{\text{TAN}}$).												
1/1	1e-01	–	4e-02	–	3e-02	–	4e-03	–	1e-02	–	7e-03	–
1/2	6e-02	0.72	2e-02	0.83	2e-02	0.70	5e-03	0.47	3e-03	1.88	2e-03	2.28
1/4	3e-02	0.99	1e-02	1.02	1e-02	1.00	3e-03	0.90	8e-04	1.98	3e-04	2.32
1/8	2e-02	0.94	6e-03	0.91	5e-03	0.95	2e-03	0.69	2e-04	1.94	5e-05	2.71
1/16	9e-03	0.98	3e-03	0.99	3e-03	1.04	8e-04	1.05	6e-05	1.68	1e-05	2.05

DG-XFEM, discontinuous Galerkin extended finite element method.

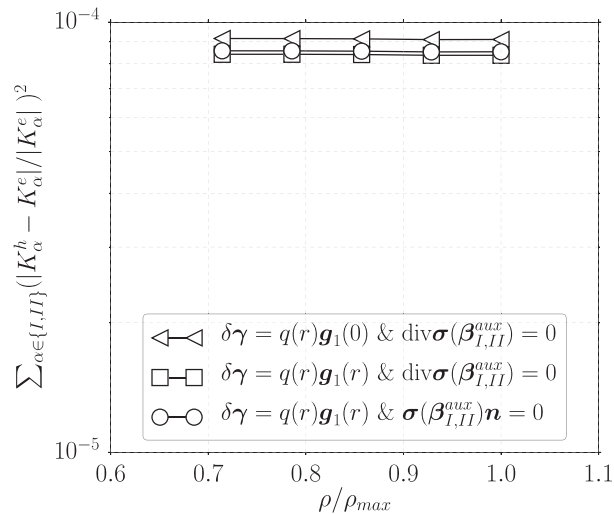


Figure 17. Contour independence of the interaction integral.

6. CONCLUDING REMARKS

In this work, we sought after the calculation of the stress intensity factors without explicitly evaluating the limit in their definition. To this end, we presented the interaction integral functional as a tool capable of addressing the challenge. The challenging tasks for obtaining convergent computations of the stress intensity factors using the interaction integral functional are the construction

of material variation and auxiliary fields. Particularly for curvilinear cracks, such fields need to be carefully constructed to ensure the well-posedness of the functional and also to guarantee an ease of evaluation.

As part of this work, we proposed two admissible material variation fields and two admissible auxiliary fields. We then provided simplified expressions of the interaction integral functional for three combinations of the material variation and auxiliary fields. Numerical approximation of the functional was provided alongside remarks on its implementation. Through numerical examples, we verified the method by comparing the computed stress intensity factors with analytical ones. Remarks are provided throughout to support the observed convergence and well-posedness of the formulation. In this work, we have thus provided a suite of material variation and auxiliary fields that, used in conjunction with the interaction integral functional, provide rapidly convergent stress intensity factors for curvilinear cracks.

APPENDIX A: MODES I AND II ASYMPTOTIC SOLUTIONS

We recall in the succeeding texts the components of displacement, gradient of displacements, and the stress fields for a straight crack lying on the axis $\vartheta = \pm\pi, r \in [0, \infty)$ as derived in [14]. The components are given for a set of right-handed orthonormal basis with the 1 axis aligned with the crack.

With $\kappa = 3 - 4\nu$ for plane strain and $\kappa = (3 - 4\nu)/(1 + \nu)$ for plane stress, the displacements are given by

$$u_1^I = \sqrt{\frac{r}{2\pi}} \frac{1}{2\mu} \cos\left(\frac{\vartheta}{2}\right) (\kappa - \cos(\vartheta)),$$

$$u_2^I = \sqrt{\frac{r}{2\pi}} \frac{1}{2\mu} \sin\left(\frac{\vartheta}{2}\right) (\kappa - \cos(\vartheta)),$$

$$u_1^{II} = \sqrt{\frac{r}{2\pi}} \frac{1}{2\mu} \sin\left(\frac{\vartheta}{2}\right) (2 + \kappa + \cos(\vartheta)),$$

$$u_2^{II} = \sqrt{\frac{r}{2\pi}} \frac{1}{2\mu} \cos\left(\frac{\vartheta}{2}\right) (2 - \kappa - \cos(\vartheta)).$$

The gradient of displacements are given by

$$\nabla u_{11}^I(r, \vartheta) = \frac{1}{4\mu\sqrt{2\pi r}} \cos\left(\frac{\vartheta}{2}\right) (-\cos(\vartheta) + \cos(2\vartheta) + \kappa - 1),$$

$$\nabla u_{12}^I(r, \vartheta) = \frac{1}{4\mu\sqrt{2\pi r}} \sin\left(\frac{\vartheta}{2}\right) (\cos(\vartheta) + \cos(2\vartheta) + \kappa + 1),$$

$$\nabla u_{21}^I(r, \vartheta) = \frac{1}{4\mu\sqrt{2\pi r}} \sin\left(\frac{\vartheta}{2}\right) (\cos(\vartheta) + \cos(2\vartheta) - \kappa - 1),$$

$$\nabla u_{22}^I(r, \vartheta) = \frac{1}{4\mu\sqrt{2\pi r}} \cos\left(\frac{\vartheta}{2}\right) (\cos(\vartheta) - \cos(2\vartheta) + \kappa - 1),$$

$$\nabla u_{11}^{II}(r, \vartheta) = -\frac{1}{4\mu\sqrt{2\pi r}} \sin\left(\frac{\vartheta}{2}\right) (\cos(\vartheta) + \cos(2\vartheta) + \kappa + 1),$$

$$\nabla u_{12}^{II}(r, \vartheta) = \frac{1}{4\mu\sqrt{2\pi r}} \cos\left(\frac{\vartheta}{2}\right) (-\cos(\vartheta) + \cos(2\vartheta) + \kappa + 3),$$

$$\nabla u_{21}^{II}(r, \vartheta) = \frac{1}{4\mu\sqrt{2\pi r}} \cos\left(\frac{\vartheta}{2}\right) (-\cos(\vartheta) + \cos(2\vartheta) - \kappa + 1),$$

$$\nabla u_{22}^{II}(r, \vartheta) = \frac{1}{4\mu\sqrt{2\pi r}} \sin\left(\frac{\vartheta}{2}\right) (\cos(\vartheta) + \cos(2\vartheta) - \kappa + 3).$$

Lastly, the stress components are given by

$$\sigma_{11}^I(r, \vartheta) = \frac{1}{\sqrt{2\pi r}} \left[1 - \sin\left(\frac{\vartheta}{2}\right) \sin\left(\frac{3\vartheta}{2}\right) \right] \cos\left(\frac{\vartheta}{2}\right),$$

$$\sigma_{22}^I(r, \vartheta) = \frac{1}{\sqrt{2\pi r}} \left[\sin\left(\frac{\vartheta}{2}\right) \sin\left(\frac{3\vartheta}{2}\right) + 1 \right] \cos\left(\frac{\vartheta}{2}\right),$$

$$\sigma_{12}^I(r, \vartheta) = \frac{1}{\sqrt{2\pi r}} \sin\left(\frac{\vartheta}{2}\right) \cos\left(\frac{\vartheta}{2}\right) \cos\left(\frac{3\vartheta}{2}\right),$$

$$\sigma_{11}^{II}(r, \vartheta) = -\frac{1}{\sqrt{2\pi r}} \sin\left(\frac{\vartheta}{2}\right) \left[\cos\left(\frac{\vartheta}{2}\right) \cos\left(\frac{3\vartheta}{2}\right) + 2 \right],$$

$$\sigma_{22}^{II}(r, \vartheta) = \frac{1}{\sqrt{2\pi r}} \left[1 - \sin\left(\frac{\vartheta}{2}\right) \sin\left(\frac{3\vartheta}{2}\right) \right] \cos\left(\frac{\vartheta}{2}\right),$$

$$\sigma_{12}^{II}(r, \vartheta) = \frac{1}{\sqrt{2\pi r}} \sin\left(\frac{\vartheta}{2}\right) \cos\left(\frac{\vartheta}{2}\right) \cos\left(\frac{3\vartheta}{2}\right).$$

Note that the aforementioned fields satisfy

$$K_I[\nabla \mathbf{u}^I] = 1, \quad K_{II}[\nabla \mathbf{u}^I] = 0,$$

$$K_I[\nabla \mathbf{u}^{II}] = 0, \quad K_{II}[\nabla \mathbf{u}^{II}] = 1.$$

APPENDIX B: CONVERGENCE OF A CONTINUOUS AFFINE FUNCTIONAL

In this appendix, we first state and prove a proposition about the convergence of continuous linear functionals of a convergent family of solutions. This proof is essentially adapted from similar results in [32, 33]. We next apply this result to the interaction integral functional (37).

Proposition

Let V be a Hilbert space and $V^h \subset V$ be its finite dimensional approximation. Let $a : V \times V \rightarrow \mathbb{R}$ be a bilinear, continuous, and coercive form with $a(u, v) \leq C_1 \|u\|_V \|v\|_V$ for all $u, v \in V$. Let $F : V \rightarrow \mathbb{R}$ be a continuous linear functional and let $G : V \rightarrow \mathbb{R}$ be a continuous affine functional. Take u to be the solution to $a(u, v) = F(v)$, $\forall v \in V$ and u^h the solution to $a(u^h, v^h) = F(v^h)$, $\forall v^h \in V^h$. Let $w \in V$ be the unique member of V such that $a(v, w) = G(v) - G(0)$, $\forall v \in V$. We further assume that there exist positive real numbers C_2 and C_3 independent of h such that $\|u - u^h\|_V \leq C_2 h^k$ and $\inf_{w^h \in V^h} \|w - w^h\|_V \leq C_3 h^k$. Then, there exists C independent of h such that

$$|G(u) - G(u^h)| \leq C h^{2k}.$$

Proof

From the definition of w ,

$$G(u) - G(u^h) = a(u - u^h, w).$$

Furthermore, note that for any $w^h \in V^h$,

$$a(u - u^h, w) = a(u - u^h, w - w^h) + a(u - u^h, w^h) = a(u - u^h, w - w^h),$$

where we have taken advantage of Galerkin orthogonality, that is,

$$a(u - u^h, w^h) = a(u, w^h) - a(u^h, w^h) = F(w^h) - F(w^h) = 0.$$

Therefore, we have

$$|G(u) - G(u^h)| = |a(u - u^h, w - w^h)| \leq C_1 \|u - u^h\|_V \|w - w^h\|_V.$$

Because w^h is arbitrary, we have

$$|G(u) - G(u^h)| \leq C_1 \|u - u^h\|_V \inf_{w^h \in \mathcal{V}^h} \|w - w^h\|_V \leq C_1 C_2 C_3 h^{2k}.$$

Taking $C = C_1 C_2 C_3$ yields the conclusion. \square

The application of this proposition to the interaction integral functionals here requires some additional work to account for the difference between domains in curvilinear cracks and the use of quadrature rules. However, disregarding these differences and assuming that $\mathcal{B}_\ell^h = \mathcal{B}_\ell$ and that exact integration is carried out, we have $\mathcal{I}^h = \mathcal{I}$, and for the standard finite element method in Section 5.1, $\mathcal{V}^h \subset H^1(\mathcal{B}_\ell; \mathbb{R}^2)$, $\forall h$. Now, in its first argument, $\mathcal{I}[\boldsymbol{\beta}, \boldsymbol{\beta}^{\text{TF}}, \delta\boldsymbol{\gamma}^{\text{TAN}}]$ is affine and continuous in $L^2(\mathcal{B}_\ell; \mathbb{R}^{2 \times 2})$, so we set $G(\mathbf{u}) = \mathcal{I}[\nabla\mathbf{u}, \boldsymbol{\beta}^{\text{TF}}, \delta\boldsymbol{\gamma}^{\text{TAN}}]$ (c.f. (37)) and can use the aforementioned proposition. Because for the standard finite element method, it is known that there exists $C > 0$ independent of h such that $\|\mathbf{u} - \mathbf{u}^h\|_{H^1(\mathcal{B}_\ell; \mathbb{R}^2)} \leq Ch^k$, $\forall h$, then

$$|G(\mathbf{u}) - G(\mathbf{u}^h)| \leq \bar{C} h^{2k}$$

for some $\bar{C} \in \mathbb{R}^+$.

This proposition is not directly applicable to the discontinuous Galerkin method in Section 5.1, because in this case, it is also necessary to account for the use of an approximation space that does not conform to H^1 . Finally, the two functionals $G(\mathbf{u}) = \mathcal{I}[\nabla\mathbf{u}, \boldsymbol{\beta}^{\text{DFC}}, \delta\boldsymbol{\gamma}]$ in (35) and (36) are not continuous in $H^1(\mathcal{B}_\ell; \mathbb{R}^2)$, because of the evaluations of $\nabla\mathbf{u}$ on the crack faces, so we cannot directly apply the aforementioned result.

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