

1 **Using sensor data to dynamically map large-scale models to**
2 **site-scale forecasts: A case study using the National Water**
3 **Model**

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6 **Key Points:**

- 7 • Approach to dynamically map large-scale model forecasts to site-scale prediction
8 using local sensor data
- 9 • Successful case study using publicly-available outputs of National Water Model and
10 180 water level sensors
- 11 • Performance analysis, generalizability of approach to other data & models, and
12 open-sourced software implementation

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Abstract

There has been an explosive growth in the ability to model large water systems. While these models are effective at routing water across massive scales, they do not yet forecast the street-level information desired by local decision makers. Simultaneously, the increasing affordability of sensors has made it possible for even small communities to measure the state of their watersheds. However, these real-time measurements are often not attached to a predictive model, thus making them less useful for applications like flood warnings. In this paper we ask the question: how can highly localized forecasts be generated by fusing site-scale sensor measurements with outputs from large-scale models? Rather than altering the larger physical model, our approach uses the outputs of the unmodified model as the inputs to a *dynamical system*. To evaluate the approach, a case study is carried out across the US state of Iowa using publicly-available measurements from over 180 water level sensors and outputs from the National Water Model. The approach performs well across a third of the studied sites, as quantified by a high normalized root mean squared error. A performance classification is carried out based on Principal Component Analysis and Random Forests. We discuss how these results will enable stakeholders with local measurements to quickly benefit from large-scale models without needing to run or modify the models themselves. The results are also placed into a broader sensor-placement context to provide guidance on how investments into local measurements can be made to maximize predictive benefits.

1 Introduction

As computational power has grown, so has the ability of hydrologists to model complex hydraulic and hydrologic systems [Blöschl *et al.*, 2014]. No longer limited to the study of single stream reaches or small watersheds, increasing access to supercomputers and graphical processing units (GPUs) is now enabling a new generation of massive models, some of which would have seemed infeasible even recently. Presently, one exciting example is the United States' National Water Model (NWM) which provides forecasts for nearly 2.7 million stream and river reaches across the continental US [Office of Water Prediction, 2017]. While very impressive in scale, the performance of the model across most of these locations has still to be evaluated. Beyond numerical modeling, a variety of studies have also highlighted the potential of big data in hydrology, wherein large quantities of data are analyzed to provide scientific insight and improve forecasting performance (e.g.

45 *Karandish and Šimůnek* [2016]; *Tiwari and Adamowski* [2015]; *Chang et al.* [2017]; *Demir*
46 *and Krajewski* [2013]; *Gilles et al.* [2012]). As such, there is now an unprecedented oppor-
47 tunity to begin leveraging advances in computing and data science to explore a variety of
48 large and complex water challenges.

49 Advances in computation have also been accompanied by improved access to real-
50 time measurements. Wireless sensor networks have become much more affordable [*Jin*
51 *et al.*, 2010] and cloud-based services are now readily available, even to small research
52 groups (e.g. Amazon Web Services, Microsoft Azure, Google Cloud etc.). The open source
53 hardware movement (e.g. *Wong and Kerkez* [2016]; *Gilles et al.* [2012]; *Bitella et al.* [2014];
54 *Bartos et al.* [2017]) is empowering many technological non-experts, such as decision
55 makers and small research groups, who can now deploy their own sensors to measure
56 a variety of water parameters in near real-time. This is allowing important, but limited,
57 sources of data, such as USGS gauges, to be supplemented by a variety of smaller and
58 stakeholder-relevant measurements.

59 These advances still do not appear to be ushering in new wave of water manage-
60 ment. At the level of individual communities or cities, water managers seek answers to
61 very practical and neighborhood-specific questions. For example, forecasting the water
62 level at specific bridges or highway overpasses can help trigger flood alerts or dispatch
63 emergency response personnel. Given their spatial extent and therefore low availability of
64 relevant data, such as bathymetry and forcing data, large numerical models may not al-
65 ways be accurate at high resolutions, meaning that their forecasts may not be immediately
66 useful to decision makers. Additionally, units and variables that are important to model-
67 ers (e.g. flow) may not be the units and variables that decision makers care about (e.g.
68 water level under a bridge). Alternatively, sensor observation alone may only go so far.
69 While making a direct measurement at any specific site may provide real-time informa-
70 tion to decision makers, it does not provide a forecast or warning without a model. There
71 is, however, an opportunity to fuse the forecasting benefits of large-scale models with the
72 site-level accuracy offered by local measurements.

73 In this paper we ask the question: how can highly localized forecasts be generated
74 by fusing site-scale sensor measurements with outputs from larger-scale physical models?
75 Instead of increasing the complexity of the physical model or re-calibrating it to match the
76 local measurement, our approach leaves the physical model unchanged and uses a dynam-

77 ical systems transformation to map the large-scale model outputs to site-scale conditions.
78 To evaluate this approach, we carry out a case study in which water levels, as measured
79 by a sensor, are predicted from modeled flows made by a publicly-available and large-
80 scale physical model. This will illustrate how city managers and other stakeholders, who
81 have access to local measurements, can quickly benefit from large-scale models without
82 needing to run or modify the models themselves. Specifically, we apply this methodology
83 to the outputs of the US National Water Model and a publicly-available data set of hourly
84 water level observations, made by over 180 sensors across the entire US state of Iowa. Be-
85 yond evaluating predictive performance, a *Random Forest*-based classification analysis is
86 also carried out to evaluate under which conditions the approach is expected to perform
87 well. The paper concludes with a discussion on the generalizability of the approach and
88 places the findings into into a broader context of making *big* models and data *useful* to
89 stakeholders.

90 1.1 Background

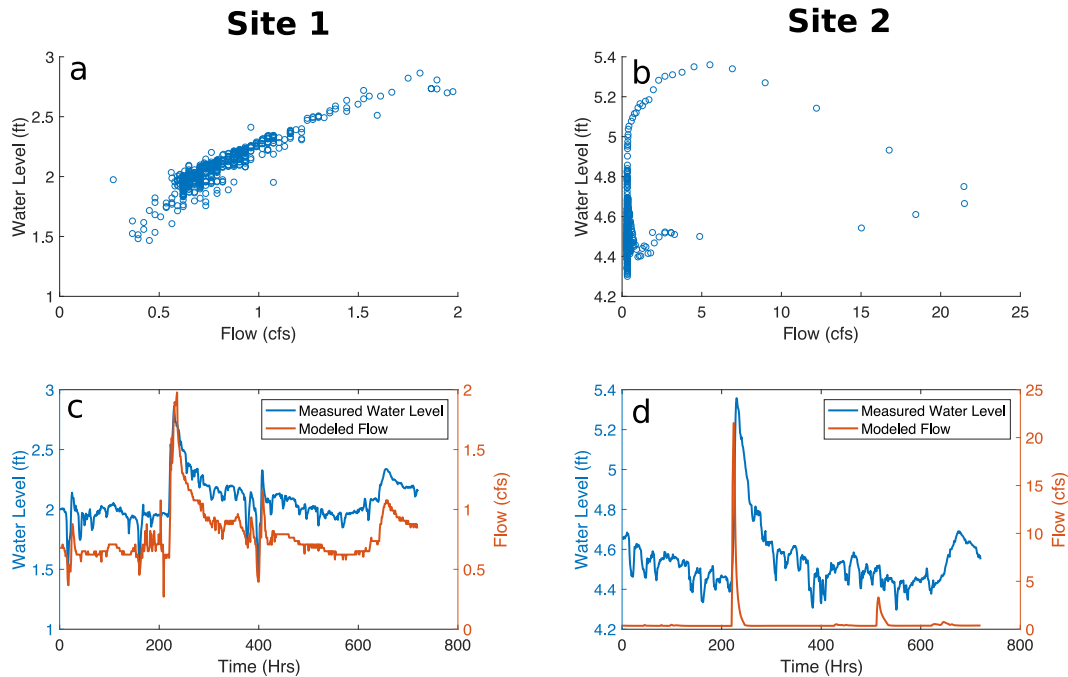
91 To illustrate the challenges that may be faced when translating macro-modeled out-
92 puts to high-resolution local conditions, we begin by using the outputs from the US Na-
93 tional Water Model (NWM) to predict water levels at sites of interest. The desire to pre-
94 dict water levels, rather than flow, is motivated by two factors. Firstly, water levels are
95 necessary for local flood inundation mapping [*Gilles et al.*, 2012]. Secondly, and more
96 importantly, local measurements of flow are expensive and rarely available. Water level
97 sensors, on the other hand, are relatively inexpensive to deploy and maintain, making them
98 a more realistically available data source [*Jin et al.*, 2010].

99 Given its spatial extent, the NWM assumes trapezoidal stream cross sections, which
100 are derived from the National Hydrography Dataset [*US Geological Survey*, 2017]. A
101 mapping of flows to heights for specific sites may thus not be directly evident, since each
102 location will have its own nuanced topographic and hydraulic properties. As such, there
103 is a motivation to discover how the outputs of this large numerical model can be trans-
104 lated to site-specific parameters that are not directly modeled. If a clear relationship can
105 be established between the modeled flows and measured heights for any given location,
106 the forecasts of the NWM could then be used to provide authorities with precise localized
107 flood inundation maps. This would allow local water managers, who have access to their

108 own measurements and knowledge of local inundation elevations, to benefit directly from
109 the expertise embedded in the larger NWM.

110 Traditionally, rating curves have been a primary tool for deriving stream flows from
111 stage measurements and vice-versa [Hersch, 1999]. Reliable rating curves require a rel-
112 atively long history of stage and discharge measurements. Measurement-constraint alter-
113 natives have been proposed (e.g. *Damangir and Abedini* [2014], *Aricò et al.* [2010]), but
114 often only work under limited conditions. Secondly, even when a long history of obser-
115 vations is available, rating curves can have large uncertainties, particularly related to het-
116 eroscedasticity [Petersen-Overlier, 2004], extrapolation outside of the history [Kuczera,
117 1996], hysteresis [Perumal et al., 2004], measurement error [Westerberg et al., 2011], and
118 backwater effects [Hidayat et al., 2011]. Most importantly, however, in the context of our
119 proposed problem, the flows are modeled rather than measured, which poses additional
120 challenges when attempting to estimate site-specific water levels. Indeed, the NWM's util-
121 ity in predicting water levels has recently been studied by other researchers on smaller
122 scales. For instance, *Javaheri et al.* [2018] used an Ensemble Kalman Filter in conjunction
123 with the Height Above Nearest Drainage (HAND) method to predict water levels from the
124 NWM. This methodology should work well for locations where a rating curve can be de-
125 veloped using the NWM flow estimates.

126 To illustrate the challenge of deriving local height estimates from modeled flows,
127 we compare the output of the NWM to two independent water level measurements made
128 on small bridges in Iowa (Figure 1). For the first example (Figure 1a), it is qualitatively
129 apparent that there is a strong relationship between the modeled flows and the measured
130 heights. This is supported by a dynamical agreement between the two time series (Figure
131 1c), which align well temporally, with clear agreement of the hydrograph peaks, as well
132 as a generally good agreement on the rates of the rising and falling limbs. This provides a
133 reliable rating curve and makes a strong case that the flow forecasts of the model could be
134 used to predict future heights. On the other hand, for the second example (Figure 1b), the
135 relationship between modeled flows and measured height is not nearly as clear. While the
136 presence of a rain storm is evident in each time series (Figure 1d), it is unclear how the
137 dynamics of each variable are correlated. The modeled flow is temporally coarse and does
138 not match the dynamics of the measured water level. Without a clear rating curve, it may
139 seem difficult to establish a relationship between modeled forecasts and measured heights,
140 which may limit the apparent utility of the modeled forecast to this specific site.



141 **Figure 1.** Measured water levels made by bridge sensors and modeled flows derived from the NWM for
 142 two example sites in the state of Iowa. The first example demonstrates a relatively strong relationship between
 143 modeled flows and measured water levels, while the second site does not.

144 When modeled flows do not directly align with local observations, one alternative
 145 is to directly assimilate the local measurements into the bigger model, thus improving its
 146 accuracy. Data assimilation is an established field in the hydrologic modeling community,
 147 relying on methods such as the Kalman Filter [Beck, 1987] or Particle Filter [Moradkhani
 148 *et al.*, 2005] to guide the model states toward the locally-measured values. In fact, the cur-
 149 rent version of the NWM performs a computationally low-cost form of data assimilation
 150 called Newtonian Nudging [Hoke and Anthes, 1976], whereby federal streamflow measure-
 151 ments from the United States Geological Survey (USGS) are used to "nudge" the model
 152 toward observed values. While the high quality and reliability of USGS gauges has been
 153 verified on many occasions (e.g. *U. S. Geological Survey and Koltun [2015]*, *U. S. Geolog-
 154 ical Survey and Southard [2013]*), the number of gauges is limited compared to the scale
 155 and resolution of the NWM. As such, the NWM will benefit from assimilating alternative
 156 sources of information into its operation.

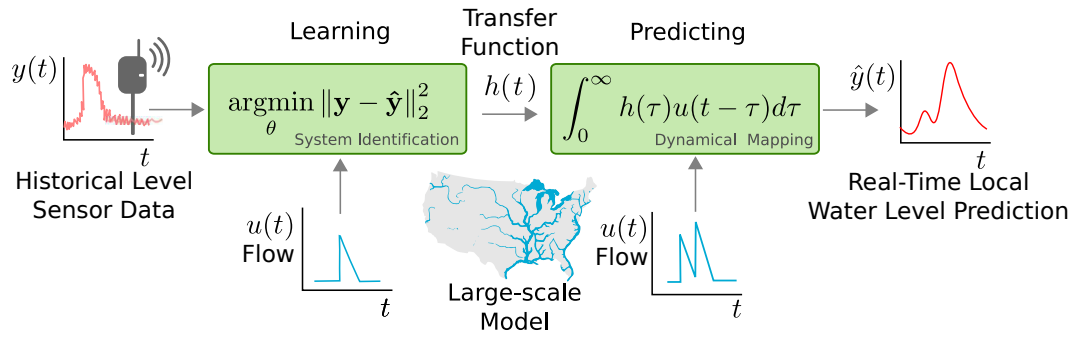
157 Expanding the coverage of the measurement network used by the NWM, such as
 158 measurements made by individual communities, poses a number of practical challenges

159 in the context of data assimilation. Firstly, given the sheer number of sensor manufactur-
160 ers, deployment standards, and maintenance schedules, some sources of local data may be
161 more reliable than others. Since measurement errors can propagate into the bigger model,
162 assimilating local data thus poses questions regarding accuracy. Data will need to be ap-
163 proved and quality checks will be needed to ensure that any faulty sensors do not damage
164 the model's integrity. Computational capacity will also need to be increased to ensure a
165 growing number of assimilation points can be integrated. Given the sheer diversity of lo-
166 cal water measurements and logistics associated with large-scale data assimilation, it is
167 unclear when or if all of them will ever be ingested into the NWM. For those local wa-
168 ter officials who do trust their own measurements, an alternative approach may still allow
169 them to benefit from the existing forecasts offered by the NWM.

170 1.2 Approach and Contributions

171 Motivated by the challenges posed in the prior section, the major contribution of
172 this paper is a computational approach by which independently-measured observations are
173 combined with the output of a larger physical or numerical model to provide a dynamical
174 forecast of local site conditions. In other words, historical model forecasts and indepen-
175 dent historical measurements will be used to derive high-resolution and dynamical fore-
176 casts for a site of interest. The output will be an automated tool chain, which allows end-
177 users to benefit from the expertise embedded in a large model without needing to update
178 the model itself (Figure 2). Specifically, the approach will be evaluated by fusing outputs
179 of the NWM and a large publicly-accessible stream sensor network in the state of Iowa
180 [Gilles *et al.*, 2012]. Since these measurements have not been used in the NWM, they pro-
181 vide an independent data set for the evaluation of the approach. Practically, a successful
182 demonstration of the approach will permit water managers, who may be inclined to invest
183 into local measurements, to benefit directly from forecasts made by the NWM.

188 Since a one-to-one stage-discharge mapping is not possible for all sites (Figure 1b)
189 our approach is based on dynamical systems theory [Luenberger, 1979]. Here, we treat
190 the output of the physical model as the input to a *dynamical system*, with the idea that
191 while the physical model may capture the general timing and magnitude of impulses, these
192 outputs need to be mapped through a dynamical transfer function, to achieve agreement
193 with measured values. Effectively, the approach will *learn* the response of a dynamical
194 system, whose input is the physical model and output is the measured stage, and use it to



184 **Figure 2.** Conceptual diagram of dynamical mapping methodology. Historical measurements made by a
 185 sensor are used to "learn" a dynamical mapping between modeled flows and measured water levels. Once the
 186 parameters of the mapping are learned, water levels can then be predicted by dynamically transforming the
 187 modeled flows.

195 transform model forecasts to water level estimates. At a low-order level, this approach is
 196 analogous to *learning* a unit hydrograph [Nash, 1957], which have been used to map rain-
 197 fall to flows (Yang and Han [2006], Cluckie and Harpin [1982]). However, simple single-
 198 order unit hydrographs are known to work mostly for smaller scale catchments [Damangir
 199 and Abedini, 2014]. Our approach addresses this limitation by expanding the order of the
 200 underlying system to be able to reflect more nuanced site-specific conditions.

201 The first part of this paper presents the theory, implementation and application of
 202 this approach to a large set of over 180 stream height observations. Secondly, we conduct
 203 a performance analysis which evaluates under which conditions the proposed approach
 204 will perform well. Given the sheer number of sites, each of which has a large number of
 205 physiographic features, a simple classification approach will not be adequate. Therefore,
 206 two analytical tools (principal component analysis and random forests) are used to deter-
 207 mine which features explain when our approach can be used to reliably predict local con-
 208 ditions. The results of this analysis will provide a general sensor placement guide to help
 209 maximize the potential of mapping NWM output to local sites.

210 **2 Methods**

211 **2.1 System Identification Theory**

212 We frame the problem of mapping a physical model output $u(t)$ to a measured sen-
 213 sor value $y(t)$ as a transfer function operation, which can be represented in the time do-

214 main as a convolution with an impulse function $h(t)$ [Luenberger, 1979]:

$$215 \quad y(t) = \int_{\tau=0}^{\infty} h(\tau)u(t - \tau)d\tau. \quad (1)$$

216 In our case study, the physical model output $u(t)$ represents the flow modeled by the
 217 NWM, while $y(t)$ are the height measurements made by a water level sensor at some lo-
 218 cation. The transfer function $h(t)$ can be converted to its frequency domain representation
 219 $H(S)$ using a Laplace transform:

$$220 \quad \begin{aligned} Y(s) &= H(s)U(s) \\ 221 \quad H(s) &= \frac{Y(s)}{U(s)} \\ 222 \quad &= \frac{b_0s^n + b_1s^{n-1} + \dots + b_{n-1}s + b_n}{s^n + a_1s^{n-1} + \dots + a_{n-1}s + a_n} \end{aligned} \quad (2)$$

223 where (a_0, a_1, \dots, a_n) and (b_0, b_1, \dots, b_n) are the n^{th} order coefficients of the transfer
 224 function. More generally, the roots of the numerator's polynomial are known as the *ze-*
 225 *ros* and the roots of the denominator are known as the *poles* of the system. Since transfer
 226 functions are equivalent to systems of linear differential equations, an increase in the order
 227 of the system reflects the ability to represent more nuanced dynamics. Given a system or-
 228 der (i.e. number of poles and zeros), the goal is to learn the transfer function coefficients
 229 from prior measurement and modeled values, after which Equation 1 can be used to trans-
 230 form any future modeled flows to their corresponding heights. In the dynamical systems
 231 literature, this problem is broadly referred to as System Identification [Luenberger, 1979].
 232 A common approach to learning the parameter $\theta := [a_1, \dots, a_n, b_0, \dots, b_n]$ of the model
 233 relies on the formulation

$$234 \quad y(t) = \hat{y}(t, \mathbf{u}; \theta) + \epsilon(t, \theta) \quad (3)$$

235 where the measured output is a function of the predicted output \hat{y} given parameter
 236 set θ , which is corrupted by a noise term $\epsilon(t, \theta)$. Finding an estimate of the parameters $\hat{\theta}$
 237 can be framed as an optimization problem that seeks to minimize the difference between
 238 modeled and observed values. Here we use the mean squared error as the loss function:

$$239 \quad \begin{aligned} \hat{\theta}(\mathbf{y}, \mathbf{u}) &= \arg \min_{\theta} \|\mathbf{y} - \hat{\mathbf{y}}\|_2^2 \\ 240 \quad &= \arg \min_{\theta} \sum_{t=1}^n (y(t) - \hat{y}(t, \mathbf{u}; \theta))^2 \end{aligned} \quad (4)$$

Our approach uses a Gauss-Newton method [Bjorck, 1996] to iteratively approach the minimum through the use of a gradient-based solver:

$$\theta^{(k+1)} = \theta^{(k)} - (\mathbf{J}^T \mathbf{J})^{-1} \mathbf{J}^T \epsilon(\theta^{(k)}) \quad (5)$$

where $\epsilon(\theta^{(k)}) = \mathbf{y} - \hat{\mathbf{y}}$ is a vector of the errors at iteration k , and \mathbf{J} is the Jacobian matrix:

$$\mathbf{J} = \begin{pmatrix} \frac{\partial \epsilon_1(\theta^{(k)})}{\partial \theta_1^{(k)}} & \cdots & \frac{\partial \epsilon_1(\theta^{(k)})}{\partial \theta_n^{(k)}} \\ \vdots & \ddots & \vdots \\ \frac{\partial \epsilon_m(\theta^{(k)})}{\partial \theta_1^{(k)}} & \cdots & \frac{\partial \epsilon_m(\theta^{(k)})}{\partial \theta_n^{(k)}} \end{pmatrix} \quad (6)$$

The Jacobian is a matrix of all the first-order partial derivatives of the error. Therefore, at each iteration, the parameterization of the transfer function model ($\hat{\theta} = [a_1, \dots, a_n, b_0, \dots, b_n]$) yields an estimated signal $\hat{\mathbf{y}}$ that approaches the true signal \mathbf{y} . Once θ is learned using time series of the inputs and outputs, forecasts can be made using Equation 1. A visual summary of the approach is provided in Figure 2.

2.2 Data sources and implementation

To promote transparency, reproducibility, and broader adoption by others, the authors have made all the formatted data, source code, and supplementary information available freely as an open source implementation on <https://github.com/kLabUM/NWM/>.

The approach was evaluated across two large data sources. These included the outputs of the US National Water Model, which served as the inputs $u(t)$ to our method. The second data set included 182 independently-measured (not assimilated into or used in the calibration of the NWM) streamgages across the state of Iowa, which represented the sensor measurements $y(t)$. The objective was to compare how well local water depths could be predicted by dynamically mapping the flows estimated flows by the NWM. Along with a summary of performance, an extensive analysis was also carried out using Principal Component Analysis [Ouyang, 2005] and Logit Boosted Random Forests [Freund et al., 1996] to classify under which conditions the proposed approach may perform reliably.

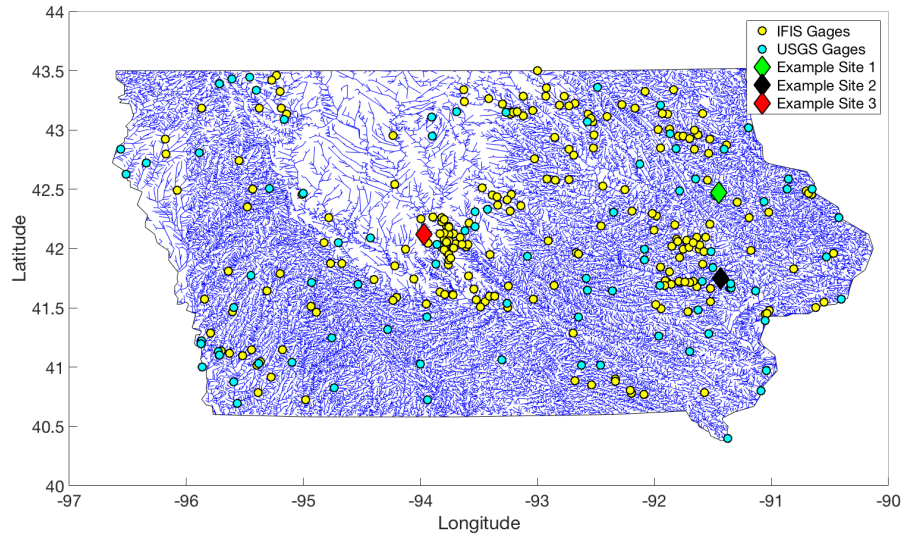
2.2.1 Data Source: The National Water Model

The National Water Model (NWM) became operational in the fall of 2016, and is continuing to be developed by the Office of Water Prediction at NOAA. The NWM estimates flow for approximately 2.7 million stream reaches across the continental United

268 States. At its core, the NWM relies on large-scale Muskingum-Cunge routing, which is
269 coupled with a gridded subsurface flow routing scheme [Office of Water Prediction, 2017].
270 The model is forced by rainfall from the Multi-Radar/Multi-Sensor System (MRMS) [Na-
271 tional Severe Storms Laboratory, 2017] as well as a suite of model outputs ingested by
272 WRF-Hydro [Office of Water Prediction, 2017]. Land surface processes, such as snowmelt,
273 evapotranspiration, infiltration, and groundwater transfer, are simulated using Noah-MP [Of-
274 fice of Water Prediction, 2017]. Additionally, the NWM assimilates measurements from
275 the national network of USGS streamgages. Given the continental scale of the model, a
276 major appeal is that it routes flows from far away regions and covers locales that are often
277 not captured by any other models. This should make it attractive for smaller communities
278 seeking flash flood or streamflow forecasts but who may not have their own modeling re-
279 sources. The NWM outputs hourly *nowcasts*, as well as 1-18 hour short-term forecasts, 0-
280 10 days medium-term forecasts, and 0-30 days long-term forecasts [Office of Water Predic-
281 tion, 2017]. Presently, modeled flows from the previous two days are freely available for
282 download in NetCDF format on the National Centers for Environmental Prediction server
283 (<ftp://ftp.ncep.noaa.gov/pub/data/nccf/com/nwm>). The NWM also provides an Analysis
284 and Assimilation product which gives a three-hour *hindcast*. Because the NWM's fore-
285 casting ability is constantly being updated and improved, this paper uses this product to
286 provide an upper bound baseline for our dynamical mapping approach.

287 **2.2.2 Data Source: The Iowa Flood Information System Sensors**

288 The Iowa Flood Center (IFC) was established in 2008 in response to the increas-
289 ing frequency of flooding in the state [Gilles *et al.*, 2012]. One of their major initiatives
290 was establishing the Iowa Flood Information System (IFIS), which provides real-time
291 stream conditions and flood warning alerts [Demir and Krajewski, 2013; Iowa Flood Cen-
292 ter, 2017]. IFIS ingests data from approximately 500 stream sensors, of which half are
293 managed by the USGS and half are managed by the IFC (Figure 3). IFC gauges are pri-
294 marily composed of bridge-mounted ultrasonic water level sensors, which transmit sub-
295 hourly measurements across a wireless connection. Historical depth measurements are
296 freely available on the IFIS website across a rolling 30 day window [Iowa Flood Center,
297 2017]. In this paper, we focus on the 220 bridge-mounted sensors that the IFC manages,
298 since these sensors were not used in the calibration of the NWM. As such, they provide
299 an independent validation data set for the proposed method.



300 **Figure 3.** Visualization of the nearly 62,000 streams modeled by the NWM in the state of Iowa. USGS
301 gages, which are assimilated into the NWM, are denoted as cyan circles. Locations of the IFIS water level
302 sensors are denoted as yellow circles, with diamonds denoting the three example sites used in this paper.

303 **2.2.3 Implementation**

304 Outputs from NWM and IFIS gauge measurements were recorded using an auto-
305 mated *Python* script on an hourly basis from October 2016 through May 2017 across
306 the state of Iowa. IFIS gauge readings were logged in real-time as measurements became
307 available. Out of the 220 candidate sites, 182 were co-located with outputs of the NWM
308 and deemed to have a continuous record. For small data gaps (few missing points), lin-
309 ear interpolation was applied to create continuous time series. The NWM and IFIS time-
310 series were linked by location, thereby providing individual model-measurement pairs that
311 could be used in our dynamical mapping approach. Data from October to December were
312 used to train our system identification approach, while data from March to May were used
313 for validation. To reduce potential impacts of wintertime conditions (freezing, snow, and
314 snowmelt), which may have influenced NWM outputs and gauge maintenance, data across
315 January and February were not used in the analysis. Additionally, stationarity of the phys-
316 iographic features was assumed (e.g. it was assumed that erosion does not change stream
317 width or roughness over the study period).

Prior to applying the system identification procedure, sensor data were linearly de-trended to remove the impact of base flows, which was necessary to ensure that the transfer functions would decay to zero following a storm event. Since the complexity of the dynamical mapping was not known a priori, an ensemble of 14 different transfer functions was learned using the training data, with each mapping having varying numbers of poles and zeros (see Equation 2). These included all possible pole-zero pairings for first through fourth order systems ([0 poles, 1 zero],[1 pole, 1 zero],[0 poles, 2 zeros],..., [4 poles, 4 zeros]). This allowed for the average and upper-bound performance of the approach to be compared across mappings of varying complexities. The final software toolchain was implemented in MATLAB, using an implementation of the System Identification procedures from *Ljung* [1987]. For comparison, a standard regression rating curve procedure [*U.S. Geological Survey et al.*, 2010] was also implemented, whereby prior stage-discharge relationships (October-December) were used to predict future values (March-May). The normalized root mean squared error (nRMSE), which is equivalent to the Nash Sutcliffe Efficiency (NSE) [*Nash and Sutcliffe*, 1970], was used to evaluate performance:

$$E = 100 \left(1 - \frac{\|y - \hat{y}\|}{\|y - \bar{y}\|} \right) \quad (7)$$

where E is the NSE in percent, y is the vector of observed water level, \hat{y} is the vector of predicted water level, \bar{y} is the mean of the observed water level, and $\|\cdot\|$ is the Euclidean norm [*Deza and Deza*, 2009]. For interpretation, a value of 100% would imply a perfect prediction of water levels, a value of 0% would imply a prediction that is as good as taking the historical average of water levels, and a value less than 0% indicates a performance more inadequate than taking the average. An NSE of 50% or above is generally considered the lower bound for a good predictor [*Krause et al.*, 2005], which is the threshold we adopted in this paper. The analysis considered the NSE of both the best ensemble member and the ensemble average.

2.3 Performance Classification

One major goal of this paper is to investigate under which conditions the proposed dynamical mapping approach will work well. Not all locations may benefit directly from our approach, even if investments into local sensors are made. Evaluating which features explain this behavior will be crucial to informing where investments into sensors should be made to maximally leverage the NWM. To classify the performance of our approach

349 under various physiographic conditions, we used a combination of Principal Component
350 Analysis [Hastie et al., 2001] and Random Forest Classifiers [Freund et al., 1996].

351 The NWM is built on a number of distinct physiographic features from the National
352 Hydrography Dataset (NHD) [US Geological Survey, 2017]. These include the channel
353 bottom width, elevation, Manning’s roughness, channel slope, and Strahler stream order
354 [Strahler, 1957]. Some features, such as side slope, were available, but were uniform
355 throughout the study area and thus deemed uninformative. Some features served as prox-
356 ies for others that would have been too labor intensive to derive (e.g. stream order and
357 relative drainage area). We also calculated an additional feature, which captures the dis-
358 tance of a given water level sensor to the nearest USGS gauge. This will indicate whether
359 our approach performs better near official NWM data assimilation locations. Overall, this
360 provided six features that may be used to explain the performance of our dynamical map-
361 ping approach. For example, intuition would suggest that our approach would work well
362 on larger rivers, where the NWM may be able to capture flow dynamics more accurately
363 than in smaller, ungauged basins. This, however, has to be confirmed, especially given the
364 array of other complex features that may explain performance.

365 Since some of the features analyzed in this study (e.g. stream order vs. bottom
366 width) may exhibit collinearity or multicollinearity, they must be orthogonalized to max-
367 imize the ability to classify around them. Before the performance is classified, our ap-
368 proach used Principal Component Analysis (PCA) to shift the six dimensional feature
369 space into an orthogonal subspace [Hastie et al., 2001]. PCA changes the coordinates of
370 the features, with the objective of finding a new set of features that are linear combina-
371 tions of the original features. PCA initially determines the direction in which the greatest
372 amount of variance lies, defines the first axis to align with that direction, and then iter-
373 atively re-orient subsequent axes such that each axis is aligned in the direction of next
374 greatest variance. In doing so, the features are de-correlated and combined into composite
375 principal axes that should maximize the ability to discover higher-dimensional hyperplanes
376 that can be used during classification. In consideration of succinctness, a theoretical dis-
377 cussion of our PCA implementation is provided in Appendix A: .

378 Once the features that describe all of the 182 sensor locations were PCA-transformed,
379 each of the sites was labeled based on performance of the dynamical mapping. The pre-
380 dictive performance was labeled in a binary sense, whereby sites with a maximum NSE

381 of 50% or greater were deemed to perform well (label 1), while any remaining sites were
382 labeled as inadequate (label 0). The performance classification was then implemented as
383 a *supervised learning* procedure, where the final classification seeks to predict how well
384 the dynamical mapping approach will perform for a given set of features. While various
385 classification algorithms exist, our approach used a statistical learning tool known as Logit
386 Boosted Random Forests, or *Adaboost* with trees [Freund et al., 1996].

387 *Adaboost* generates a large number of "weak learners" [Hastie et al., 2001] in the
388 form of small classification trees. A weak learner is a model that is only slightly better
389 than randomly guessing [Zhou, 2012]. A classification tree partitions a feature space us-
390 ing a series of binary splits, resulting in a large number of labeled bins. For example, a
391 simple tree may categorize the performance of our dynamical approach as either "high"
392 or "low". These labels are assigned to training data, as described above. The tree may
393 be trained to classify based on decision variables, such as stream order or elevation. The
394 training data are then placed into bins, or leaves. The label of each bin is assigned based
395 on majority voting, or how many "high" and "low" labels are inside each bin. The deci-
396 sion variables are used to improve binning. For example, all of the data may first be split
397 up by elevation to create two bins. These bins may then be split further by stream order
398 to reduce the label variance inside each bin. In this example, this would create only four
399 bins. The low number of bins, and thus low complexity, of such a classifier is the reason
400 why it is called a weak learner. Given their relative simplicity, these trees tend to have
401 very low bias but also very high variance [Hastie et al., 2001]. This can be addressed by
402 generating an ensemble, or a *forest*, of many trees. Going a step further, we seek to en-
403 sure that each tree is developed to provide as much information gain as possible. In Logit
404 Boosted Random Forests, each data entry (i.e. labeled row of our data matrix \mathbf{T}) is given
405 an initial weight w_i . Then, as new trees are learned, the data entries are re-weighted so
406 as to emphasize where the model is failing. That is, the final algorithm (Algorithm 1)
407 ensures that misclassified data are stressed more in the learning of the next tree. In this
408 implementation, \mathbf{t} is the input data (i.e. a row of \mathbf{T}), y is the observed data (1 for a site
409 labeled as well-performing site, -1 for a bad site), N is the number of observations, M is
410 the number of trees, p_m is the probability output from tree m , and $H(z) := \mathbf{1}_{[z>0]}$ is the
411 Heaviside step function [Hastie et al., 2001]. The M classification trees are learned in an
412 iterative fashion.

```

1 Initialize  $w_i = \frac{1}{N}, i = 1, 2, \dots, N;$ 
2 for  $m=1, 2, \dots, M$  do
3   Learn classification tree that outputs  $p_m(\mathbf{t}_i) = P_w(y = 1|\mathbf{t}_i) \in [0, 1]$  with weights  $w_i;$ 
4   Set  $f_m(\mathbf{t}_i) \leftarrow \frac{1}{2} \log \frac{p_m(\mathbf{t}_i)}{1-p_m(\mathbf{t}_i)};$ 
5   Set  $w_i \leftarrow w_i \exp[-y_i f_m(\mathbf{t}_i)], i = 1, 2, \dots, N,$  and renormalize such that  $\sum_i w_i = 1;$ 
6 end
7 Output classifier as  $H[\sum_{m=1}^M f_m(\mathbf{t})]$ 

```

Algorithm 1: Logit Boosted Random Forest

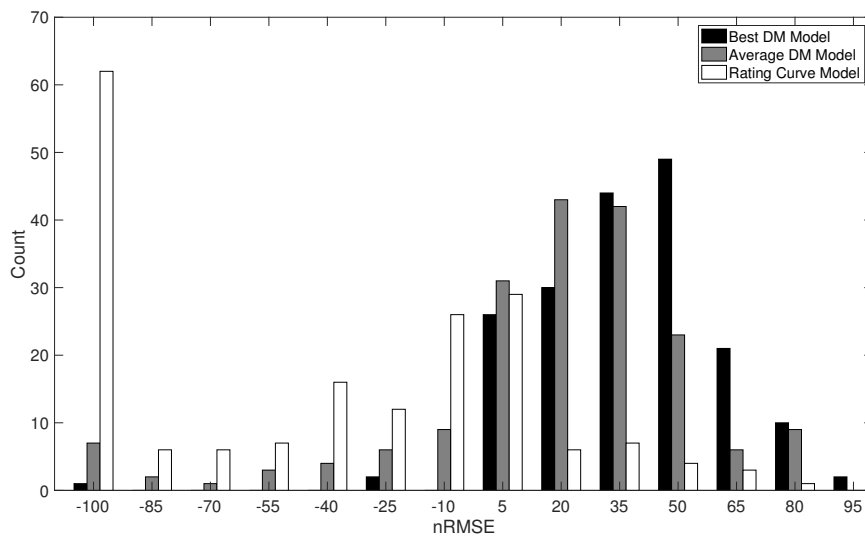
413 The logit function (line 4 of the algorithm) is used to re-weight the inputs (line
414 5). Because of the form of the logit function, much larger values exist closer to 0 and
415 1. The result is that if the data entry \mathbf{t}_i is classified properly and with high probability,
416 then $\exp[-y_i f_m(\mathbf{t}_i)]$ in line 5 will trend towards zero. If it is classified improperly with
417 high probability, then this term will approach infinity. This ensures the re-weighting will
418 target poorly classified data on the next iteration and that properly classified data will be
419 largely ignored. After learning all M models, any new input \mathbf{t} can be provided and, when
420 summing over all $f_m(x)$ trees, a prediction can be made for whether a site will be a good
421 candidate for our dynamical mapping approach. A good site will be one that sums to be
422 greater than 0 and a bad site will sum to be less.

423 3 Results

424 3.1 Dynamical Mapping Performance

425 After training and applying the dynamical mapping (DM) procedure across all 182
426 sensor locations, predictions at approximately one-third of the sites (55/182) exceeded the
427 desired 50% NSE threshold, while performance across 90 sites exhibited an NSE of at
428 least 40%. The overall performance of the approach is summarized in Figure 4, showing
429 that the DM procedure consistently performed better than a simple rating curve approach.
430 Indeed, in all but 8 cases, water levels were predicted more accurately using the proposed
431 DM approach compared to a regression between measured levels and NWM-modeled
432 flows. The order (i.e. the number of poles and zeros) of the transfer functions that had the
433 best performance was not consistent site-to-site. The order is analogous to the complexity
434 of the transfer function, where higher order systems are capable of describing complex dy-

435 namics. However, complexity of the model in the frequency domain does not necessarily
 436 lead to improved results in the time domain since the accuracy depends on the amount of
 437 information that can be extracted from the system [Rojas *et al.*, 2010]. Here, information
 438 is tied to the number of frequencies present in the signal. Put simply, if the dynamics of
 439 the measured water level and NWM can be described by a small frequency range (which
 440 may be unique to each site), then making the transfer function more complex will lead to
 441 improvements in performance since the additional coefficients will be relatively small in
 442 magnitude. In many cases, higher-order poles and zeroes may actually cancel, thus effec-
 443 tively providing an implicitly low-order model. As such, higher complexity may not nec-
 444 essarily improve model performance. There were no site-specific physiographic features,
 445 which could be used to determine what this upper bound was in our case study.

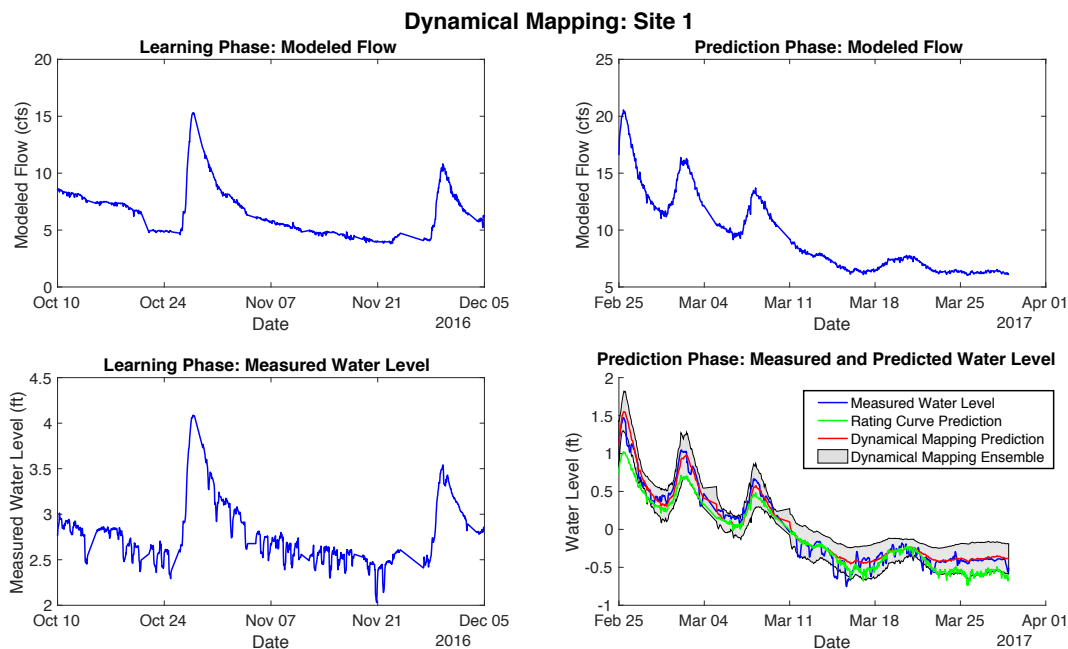


446 **Figure 4.** Histograms of prediction performance (nRMSE) evaluated across 182 sensor locations. A com-
 447 parison is made between the best dynamical mapping (black), ensemble of dynamical mappings (gray), and a
 448 simple regression-based rating curve approach (white)

449 Given the sheer number of sites used in the analysis, this section will evaluate three
 450 locations in detail, while the remainder are plotted in the supplementary information. The
 451 three sites were selected to reflect three types of performance, as measured by NSE. The
 452 first is a location for which our DM approach provides a strong predictive performance,
 453 in large part due to a high correlation between the NWM predicted discharge and the ob-
 454 served stage. The second site exhibits strong predictive performance, despite the NWM

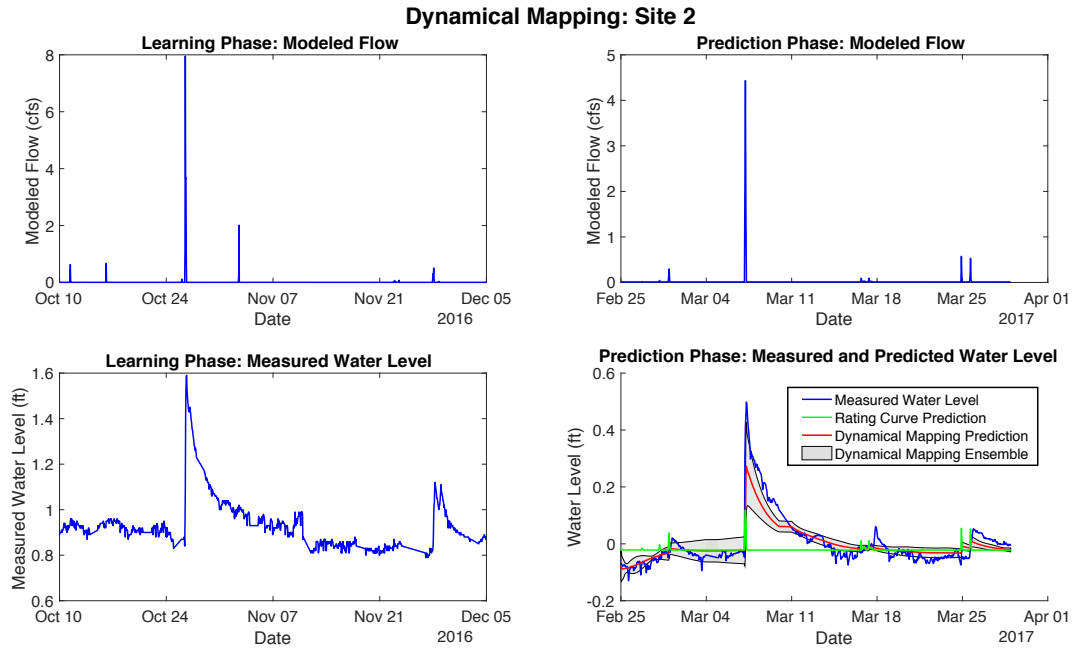
455 providing coarse outputs. The final example illustrates a case in which there is a limited
456 ability to predict observed heights from flows.

457 The first example demonstrates a case of strong predictive performance (Figure
458 5). The left column of the figure displays the training data, which includes the NWM
459 model outputs and measured water levels for the Fall of 2016. The right column shows
460 the NWM outputs and measured water levels for the Spring of 2017, as well as the wa-
461 ter level predictions made by our DM approach. Specifically, the bottom right panel is the
462 average prediction made by our approach across all 14 transfer functions (red line, with
463 gray area indicating variability within the ensemble) compared to the measured water lev-
464 els (blue line). Overall, our DM procedure performed well at this site, with an average
465 NSE close to 80%. Predictions of water levels at this site using a simpler regression-based
466 rating curve performed nearly as well, with an average NSE of 76%.



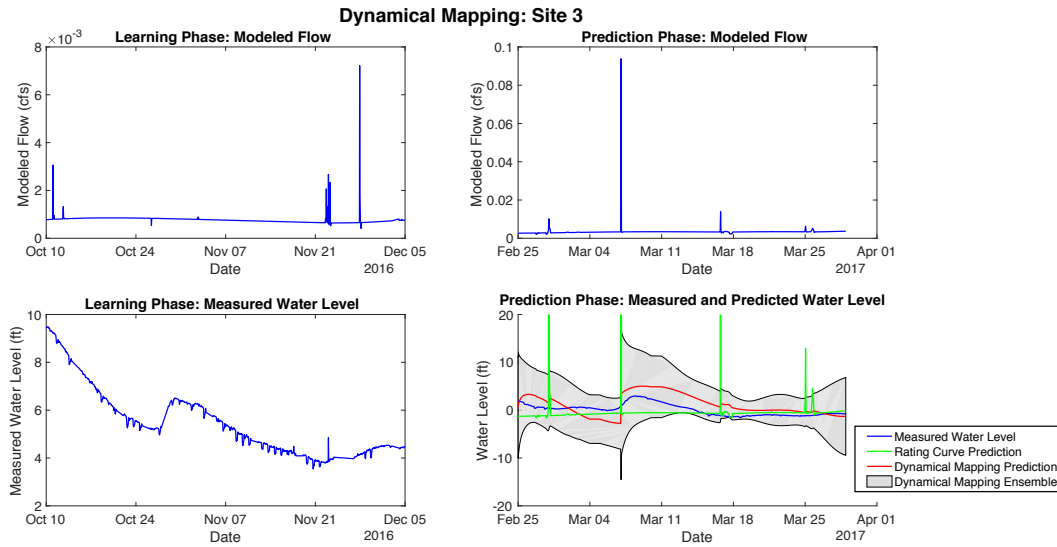
467 **Figure 5.** Dynamically mapping modeled flows to local water levels on site 1 (see Figure 3). Data used
468 to "learn" the mapping parameters are plotted in the left column, while the resulting mapping is applied to
469 future data in the right column. For this example site, the dynamical mapping performs relatively well (NSE
470 of 80%). A simple regression-based rating curve approach (not plotted) performs strongly as well, with an
471 NSE of 76%.

472 The second example (Figure 6) illustrates a case where a simple regression approach
 473 did not perform well (NSE of -4%). The modeled flows were quite impulsive and not rep-
 474 resentative of observed dynamics. However, when our DM approach was used, the results
 475 improved significantly, with an average NSE of over 50%.



476 **Figure 6.** Dynamically mapping modeled flows to local water levels on site 2, following convention used in
 477 Figure 5. For this example site, the dynamical mapping performs relatively well (54% NSE), while a simple
 478 regression-based rating curve approach does not (-4% NSE).

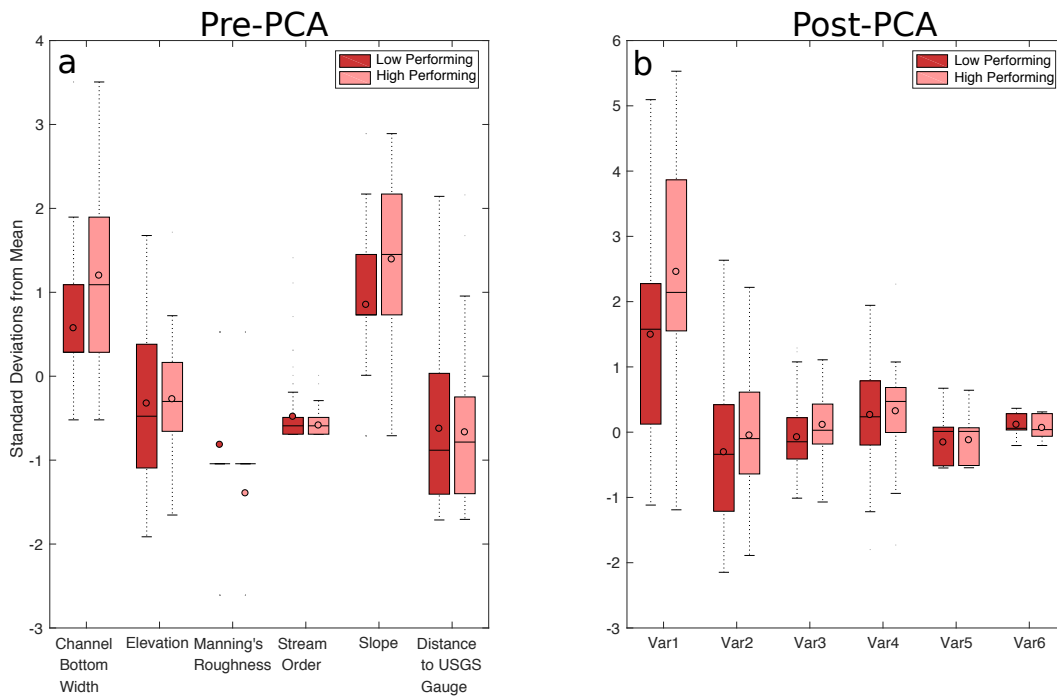
479 Finally, the third example (Figure 7) illustrates a location at which no good predic-
 480 tive performance can be reached, regardless of the approach used. As evident in the fig-
 481 ure, the measurements reflected a slowly changing system, while the NWM showed a se-
 482 ries of rapid impulses. An average NSE of -145% was obtained using our DM approach,
 483 with only one of the 14 ensembled transfer functions showing a slightly favorable NSE
 484 (49%). The rating curve method was even more ineffective, with an NSE of -14,900%.
 485 Further, the rating curve method predicted water levels well outside the realm of possibil-
 486 ity given its linear extrapolation (close to 6km water height).



487 **Figure 7.** Dynamically mapping modeled flows to local water levels on site 3, following convention in
 488 Figure 5. For this example site, neither the dynamical mapping (0% NSE) nor regression-based rating curve
 489 (NSE of $-14,900\%$) perform well.

490 3.2 Performance Classification

491 The specific performance of the DM approach across sites 1-3 could be loosely de-
 492 scribed by channel width, the stream order, and the distance to a USGS gage. Sites 1, 2
 493 and 3 had channel widths of 20ft, 10ft, and 5ft; stream orders of 5, 3, and 1; and distance
 494 to USGS gages of .2km, 2.2km, and 5.6km, respectively. More broadly, using the 50%
 495 NSE criterion, 55 of the 182 sites were labeled as locations of *high* performance, while
 496 127 were labeled as *low* performing, reflecting the ability of our DM approach to predict
 497 flows from NWM outputs. These labels were then used to determine the combination of
 498 physiographic characteristics that describes the conditions under which the DM approach
 499 exhibits high performance. The normalized distributions of each physiographic feature,
 500 split by performance criteria, is shown in Figure 8a. This normalization was performed
 501 relative to all 64000 streams in Iowa, not just the 182 sites studied. Overall, little distinc-
 502 tion was evident between high-performing and low-performing sites, with the distribution
 503 of each physiographic feature showing similar means and variances. The distributions of
 504 channel bottom width and channel slope showed the relatively largest discrepancy, sug-
 505 gesting that sites at which the DM approach performed well had a larger stream width and
 506 slope than lower-performing sites. However, the bounds on these distributions were not
 507 sufficient to determine a consistent labeling.



508 **Figure 8.** Boxplots representing the relative distribution of features, when split by the ability of a dynamical mapping to predict water levels from modeled flows. For any given feature, a clear difference between
 509 the two distributions would indicate that this feature describes a general condition for the dynamical approach to work well. a) This plot shows the splits based on stream physiographic features. It is not apparent in this
 510 figure that any features describe a general condition for the DM approach to work well. b) This plot shows the splits based on principal components (new variables 1-6). Here, the first principal components exhibits the
 511 strongest difference between the high and low performing sites, illustrating a potentially strong indicator of prediction performance.
 512
 513
 514
 515

516 Applying PCA to the physiographic features across the entire state of Iowa resulted
 517 in a 62000×6 data matrix. The resulting principal components are shown in Table 1.
 518 Each entry in a column of this table can be interpreted as the relative influence of a physiographic variable to a particular principal component. For example, considering the first
 519 principal component, which explains the greatest amount of variability in the physiographic data, it becomes apparent that the channel bottom width and the stream order
 520 both increase as the first principal component score increases. On the other hand, the
 521 Manning's roughness decreases as the principal component score increases. As such, if
 522 a stream reach in the data set has a large first component score, it will be relatively larger
 523 and smoother than other streams. Further, a number of the components exhibited oppos-
 524
 525

526 ing physiographic relationships. For example, for the second principal component, streams
 527 closer to a USGS gauge and located at a relatively higher elevation had relatively higher
 528 component values. For the fourth component, this relationship was reversed, as stream
 529 reaches at higher elevations and located further away from a USGS gauge tend to have
 530 higher component values. Similar interpretative examples could be provided for the other
 531 principal components.

532 **Table 1.** Principal Components resulting from applying PCA to features of 62,000 streams across the entire
 533 state of Iowa. σ is the singular value associated with that component whose relative magnitude indicates the
 534 amount of variability the component explains in the data.

Stream Feature	Comp. 1 $\sigma = 3.16$	Comp. 2 $\sigma = 1.09$	Comp. 3 $\sigma = 0.86$	Comp. 4 $\sigma = 0.77$	Comp. 5 $\sigma = 0.08$	Comp. 6 $\sigma = 0.04$
Bottom Width	0.534	0.140	0.185	-0.041	0.693	-0.423
Elevation	-0.198	0.648	0.182	0.712	0.016	0.000
Manning's Roughness	-0.535	-0.143	-0.170	0.008	0.720	0.382
Slope	-0.254	-0.075	0.932	-0.240	-0.015	0.062
Order	0.545	0.141	0.105	-0.004	0.023	0.819
Proximity to USGS gage	-0.165	0.717	-0.157	-0.659	-0.012	0.003

535 The performance of the DM approach, split by principal components, is shown in
 536 Figure 8b. Compared to splitting based on just physiographic features (Figure 8a), a more
 537 distinct clustering was evident for a few of the new variables. This is especially true for
 538 the first principle component, for which a larger component score generally corresponded
 539 with higher performance of the DM approach. While the other principal components did
 540 not exhibit as large of a discrepancy, the opposing physiographic relationships within
 541 each of their principal components, as noted above, suggested that application of a Logit
 542 Boosted Random Forest would enable effective classification.

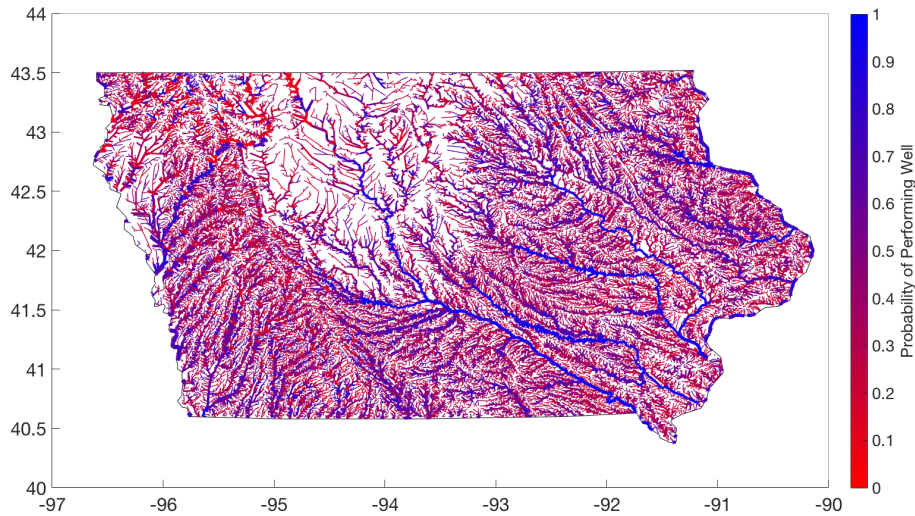
543 To understand why Random Forests were considered as an appropriate next step,
 544 consider an example variable with a range of 0 to 1. Class 1 may have values of less than
 545 0.25 or greater than 0.75, whereas class 2 may have a range of 0.25 to 0.75. The mean of
 546 the two classes would be indistinguishable, but the variance would be different. Therefore,
 547 one may conclude that this variable is not very useful for prediction. However, placing

548 two binary splits on that data (at 0.25 and 0.75) will yield a very strong classifier. Now, if
549 one considers 6 variables in a high-dimensional space, finding similar split points through
550 visual inspection would be difficult, if not impossible. Instead, all 6 can be ingested into
551 the Logit Boosted Random Forest to leverage all potential partitions. If a variable provides
552 no predictive power, then very few split points will occur on that variable and the perfor-
553 mance should not be impacted.

554 After applying the Logit Boosted Random Forest algorithm (Algorithm 1), cross val-
555 idation reflected a 75% accuracy in classifying whether the DM approach would work or
556 not (Area under *receiver-operator curve* was 0.69). The resulting Random Forest model
557 was then applied to all 62,000 PCA-transformed stream reaches in Iowa. The outputs were
558 standardized on a scale from 0-1, indicating the probability that our DM algorithm would
559 perform well at transforming NWM outputs to water levels. The final results are plotted
560 for all stream reaches in Iowa in Figure 9, where the color blue is used to denote loca-
561 tions at which the DM approach is expected to perform well. It is important to note that
562 this map covers many more streams than are measured by the 182 level sensors. As such,
563 it should be interpreted as a map of potential future sensor sites. That is, placing a level
564 sensor into any of the dark blue regions should correspond, on average, with a higher like-
565 lihood of successfully mapping NWM outputs to water levels using our DM approach.

569 **4 Discussion**

570 In lieu of recalibrating or expanding the complexity of a large numerical model,
571 there may instead be immediate benefits to be gained by using sensor data to "learn" how
572 larger-scale model outputs map to site-level conditions. To start, at approximately 30 of
573 the 180 sites, a strong flow-to-height relationship already existed. Some of those sites
574 were located close to USGS gages, which are assimilated into the NWM. For instance,
575 Site 1 (Figure 5) is located only a few hundred meters from a USGS gage. Due to di-
576 rect assimilation, the numerical model is likely to represent the nuanced flow dynamics
577 more accurately at these locations, which leads to more reliable rating curves. In these
578 instances, even a simple regression would have sufficed to predict local water levels. Nat-
579 urally, our dynamical mapping approach performed well in all of these cases, too, since it
580 can be generalized as a linear transformation [Ljung, 1987].



566 **Figure 9.** Map of site performance potential across the state of Iowa, showing a spectrum of locations
567 where the dynamical mapping approach is expected to perform well in predicting local water levels from flows
568 (blue) to those where it will likely not perform well (red).

581 While a simple regression may work in some cases, the number of instances where
582 it can be used is fairly small. By comparing modeled flows from the NWM to measured
583 water levels, our analysis demonstrated that these mappings are often not straightforward.
584 Given the lack of a clear one-to-one mapping, a regression-based approach, or one that is
585 based on simple physical equations, may not perform well because it does not account for
586 the temporal transformation of the input signal. As such, a major benefit of our approach
587 relates to its ability to make predictions when modeled values and local measurements
588 do not exhibit a clear point-to-point relationship. This is particularly evident in cases
589 where site-scale dynamics were accurately reconstructed despite the fact that large-scale
590 NWM outputs appeared like a rapid set of impulses (Figure 6). To this end, a dynamical
591 mapping, parameterized through system identification, shows promise as a general tool to
592 transform modeled values to more accurate local predictions.

593 Our specific case study of the NWM reveals a number of generalizable requirements
594 for the dynamical mapping to work well. Regardless of model- or site-specific dynamics,
595 the modeled values and sensor measurements should generally agree in relative magnitude
596 and timing. In other words, if the modeled flows show an increase over a period of time, a
597 corresponding rise in water levels should be measured as well. This could occur irrespec-

598 tive of specific dynamical features. Namely, even if the modeled values appear as a set of
599 sudden impulses, they can be adequately mapped to the more continuous in-situ sensor
600 values if a sufficient level of agreement exists between the two times series. In the case of
601 a hydrologic model, when using routing procedures like Muskingum-Cunge, particularly
602 in headwater areas, it is not uncommon for flows to be modeled as "flashy" or as a series
603 of brief spikes. While the physical model may not be designed to account for nuanced
604 site-level dynamics, it may, in fact, be routing the mass of water correctly. In such cases,
605 our approach can be used to represent these site-level dynamics by relying on the ability
606 of the larger model to explain the underlying inputs. This is quite powerful, as it suggests
607 that in many cases the site-level complexity can be explained without changing much, if
608 anything, about the larger underlying numerical model. Rather, it may often be possible to
609 rely on local sensor data to explain how modeled values are transformed to local observa-
610 tions.

611 Our classification analysis brings to bear under which conditions the DM approach
612 may not perform well. In fact, at over two thirds of the evaluated sites our approach did
613 not perform well in mapping NWM flows to local water levels, as quantified by the 50%
614 NSE criterion. This may not necessarily be a limitation of the actual approach, but rather
615 an indicator that the approach will improve as the physical model becomes more generally
616 representative of local flows. In many cases, there was simply a general lack of temporal
617 agreement between the numerical model and the measured data, with many instances of
618 false positives and false negatives (e.g Figure 7). There were many instances during which
619 the NWM predicted a change in flows, while no change in heights was ever measured.
620 Similarly, many sensors measured storms that were never seen in the NWM. Naturally, our
621 approach will not work under these conditions, since it requires changes in the inputs to
622 be mapped to changes in outputs. Of course, our DM approach could benefit by including
623 additional local data (e.g. independently-made rainfall measurements), but this increases
624 its complexity, increases implementation overhead, and decreases its generalizability. This
625 would defeat the original goal of simply relying on a publicly-available physical model
626 that someone else updates and maintains. To that end, we expect that the performance of
627 the DM approach will improve as the underlying physical model is improved, which is an
628 ongoing and promising effort within the NWM community.

629 A number of insights, specific to the NWM, also emerged from our performance
630 classification. Given the size, complexity, and collinearity of the data set, we illustrated

631 quickly that a simple classification of performance, based on individual physiographic
632 features, does not provide much insight (Figure 8a). One take-away, though not strongly
633 consistent, appears to be that our dynamical mapping performs well on larger streams and
634 rivers. This should be intuitive, since the NWM would be expected to represent larger
635 gauged rivers more effectively than smaller upstream headwater catchments. Furthermore,
636 Muskingum-Cunge methods have been shown to work quite well in laboratory settings,
637 but can introduce errors in field settings that, while negligible at small scales, can have
638 major impacts as these errors propagate [Perumal *et al.*, 2009; Sahoo, 2013].

639 While the the application of PCA removed the challenge of using correlated fea-
640 tures to explain the performance of the DM approach, the intuitive interpretation of prin-
641 cipal components reaches a limit quickly. To that end, our application of Logit Boosted
642 Random Forests allowed for the creation of a map that summarizes the expected perfor-
643 mance of our approach across all 62,000 streams in Iowa (Figure 9). This visual represen-
644 tation provided an intuitive means by which to assess broader performance. As expected,
645 our DM approach is expected to perform well across the major rivers in the state (thicker
646 lines in map). Given their size, these streams are more likely to be instrumented by USGS
647 gauges, meaning the NWM is more likely to accurately estimate flows. Many of the re-
648 maining streams on the performance map (Figure 9) showed roughly a 50% probability of
649 successfully applying the dynamical mapping. Most of these were characterized by a mid-
650 level stream order. These streams are likely more sensitive to local precipitation dynamics,
651 which may not be captured by the MRMS precipitation product used by the NWM. At
652 finer resolutions, the noise in the MRMS estimates may have a greater impact on the over-
653 all accuracy since feeding noisy observations into a non-linear model may amplify errors.
654 As water is routed through the system, the spatiotemporal accuracy of the precipitation
655 estimates likely has less of an impact as the overall volume is correct. This suggests that
656 improved precipitation inputs have the potential to dramatically improve the accuracy of
657 the NWM at higher resolutions, which should, in turn, improve the performance of our
658 dynamical mapping.

659 Given its impressive extent, the NWM already shows great promise to provide high-
660 resolution forecasts. Increasing the resolution, parameterization, and complexity of the un-
661 derlying numerical model is one way of reaching the ultimate goal of high resolution local
662 forecasts. Alternatively, as our case study demonstrated, the existing model may already
663 be very strong in many locations, but its outputs just have to be mapped to site-specific

664 features using locally-available sensor data and a suitable mathematical transform. In other
665 words, outputs from NWM, though still in their early stages, can be useful in estimating
666 highly-local water levels *even now*. Nonetheless, our results may also provide a guide to
667 help improve the numerical model. The map in Figure 9 intuitively conveys a general as-
668 sessment of the performance of the underlying numerical model. Since the NWM is a
669 relatively new model, it would be expected to initially perform well at larger scales. Even
670 with this general trend, there are still lower-order streams on the map that suggest the pos-
671 sibility of successfully applying our DM approach. These red and purple regions on the
672 map (0-50% chance of applying the dynamical transformation) may be of interest to mod-
673 elers as locations at which the numerical model could be improved to reduce false posi-
674 tive and negative forecasts. Improving the model on these stream sections will likely also
675 translate to better model performance on stream reaches that share similar physiographic
676 or PCA-transformed features.

677 From a water management perspective, the benefits of our DM approach may al-
678 ready be realizable operationally. This is true for a number of already existing sensor loca-
679 tions, as well as potentially other similar streams on the map in Figure 9. A simple web-
680 service application [Wong and Kerkez, 2016] could be written to extract NWM outputs and
681 fuse them with local sensor data. If the dynamical mapping is reliable at this location, the
682 site would benefit immediately from a localized water level forecast. Alternatively, if lo-
683 cal measurements are not available, the map in Figure 9 could be used to deploy low-cost
684 sensors at locations that maximize the probability of using the DM approach. Given the
685 general structure and input data of the NWM across the US, we also anticipate that sim-
686 ilar maps could be created for regions outside of Iowa by relying on the results from this
687 study. For the approach to become operational, a moving training window may be needed
688 to account for varying hydrologic regimes or seasons (e.g. spring vs summer). The ef-
689 fect of hydrological regimes was not evaluated as part of our case study due to the time
690 required to log data from the NWM and level sensors. Fortunately, the training window
691 needed for fitting the DM model is quite small compared to that of a rating curve, which
692 will allow for effects of seasonality to be evaluated in the future.

693 **5 Conclusions**

694 In this paper, we provided a means by which outputs from a large-scale model can
695 be fused with local sensor data to provide site-level forecasts. The novelty of the approach

696 relies on using the outputs of the physical models as the inputs into a dynamical mapping
 697 that *learns* what a specific sensor will measure. This is quite powerful, as it does not rely
 698 on the modification of the actual physical model or the direct assimilation of the sensor
 699 data, both of which would be infeasible for smaller communities. Instead, the approach
 700 is general, in that it can be directly repeated for any combination of sensor-model pairs.
 701 As such, the approach developed here could be applied directly without any modification
 702 of our open-source code. While the approach will not work under all conditions, it may
 703 already provide an immediate benefit to a large number of locations.

704 In the age of *Big Data in Hydrology* we contend that even models can be viewed as
 705 just one of many streams of data that will enable decision making. Overall, the approach
 706 of dynamically mapping outputs from large models to local sites may work for a number
 707 of models beyond just the NWM. The ability to use the approach with short data histories
 708 (e.g. only a few months of training data) makes it appealing for urban applications, where
 709 land use changes may occur rapidly and system re-identification may need to occur fre-
 710 quently. In such cases, our approach could be combined with popular urban water models,
 711 such as the stormwater management model (SWMM) to provide improved forecasts of ur-
 712 ban flooding or sewer flows. More examples can be given, but we anticipate that the our
 713 data-driven approach could be generalized for many hydrologic and hydraulic models.

714 **A: Principal Component Analysis**

715 The goal of PCA is to find the weighting vectors, or *principal components*, that yield
 716 linear combinations of the original feature space. We define $\mathbf{X} \in \mathbb{R}^{n \times d}$ as the data matrix
 717 with n rows of observations and d features, which in our case is populated with the phys-
 718 iographic features of the nearly 62,000 stream reaches in Iowa. Before PCA is applied, all
 719 input features also need to be standardized in magnitude to reduce impacts of overweigh-
 720 ting some features over others [Hastie *et al.*, 2001]. By standardizing across each variable,
 721 one can consider the relative impacts of each more effectively.

722 To find the first principal component, \mathbf{w}_1 , we find a unit vector that maximizes the
 723 variance of \mathbf{X} , that is:

$$724 \quad \mathbf{w}_1 = \arg \max_{\|\mathbf{w}\|=1} \|\mathbf{X}\mathbf{w}\|^2 = \arg \max_{\|\mathbf{w}\|=1} \frac{\mathbf{w}^T \mathbf{X}^T \mathbf{X} \mathbf{w}}{\mathbf{w}^T \mathbf{w}} \quad (\text{A.1})$$

725 This is a Rayleigh quotient [Horn and Johnson, 1990], and therefore the solution to this
 726 maximization problem is the largest eigenvector (i.e. the eigenvector of the largest eigen-

727 value) of $\mathbf{X}^T \mathbf{X}$. Each successive principal component is the next largest eigenvector of
728 $\mathbf{X}^T \mathbf{X}$. Therefore, rather than solving iteratively for each principal component, it is possible
729 to consider the singular value decomposition (Equation A.2) of the data matrix \mathbf{X} :

$$730 \quad \mathbf{X} = \mathbf{U} \Sigma \mathbf{W}^T \quad (\text{A.2})$$

$$731 \quad \mathbf{X}^T \mathbf{X} = \mathbf{W} \Sigma \mathbf{U}^T \mathbf{U} \Sigma \mathbf{W}^T = \mathbf{W} \Sigma^2 \mathbf{W}^T \quad (\text{A.3})$$

732 As such, the eigenvectors of $\mathbf{X}^T \mathbf{X}$ are the rows of \mathbf{W} , meaning the principal components
733 are the right singular vectors of our data matrix. Therefore, to place our data in an orthog-
734 onal feature space such that all the variables are de-correlated, the new data matrix, \mathbf{T} , is
735 simply:

$$736 \quad \mathbf{T} = \mathbf{X} \mathbf{W} \quad (\text{A.4})$$

737 Using this matrix will lead to a more stable classification procedure, will reduce the like-
738 lihood of over fitting, and will enable more complex interactions between features to be
739 captured [Hastie et al., 2001].

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748 be found at <https://github.com/kLabUM/NWM> while all data can be found at *Fries and*
749 *Kerkez* [2017].

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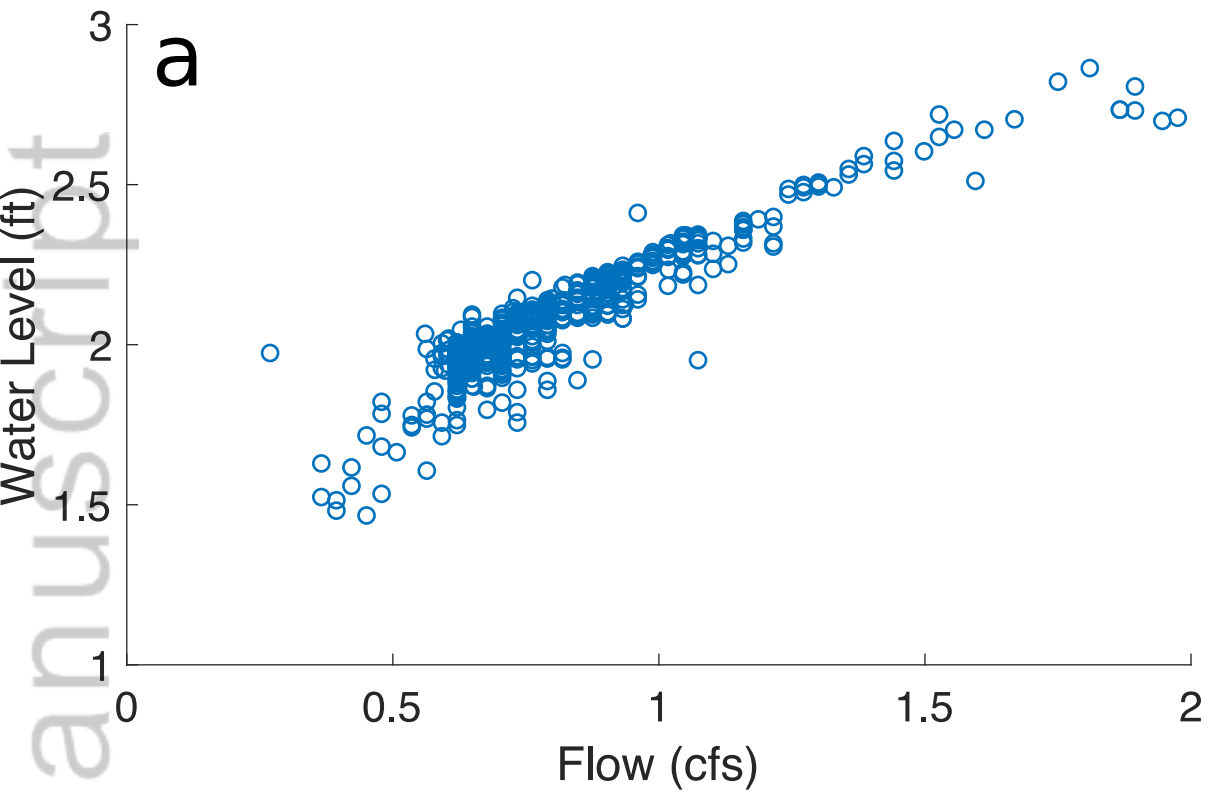
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Figure 1.

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Site 1



Site 2

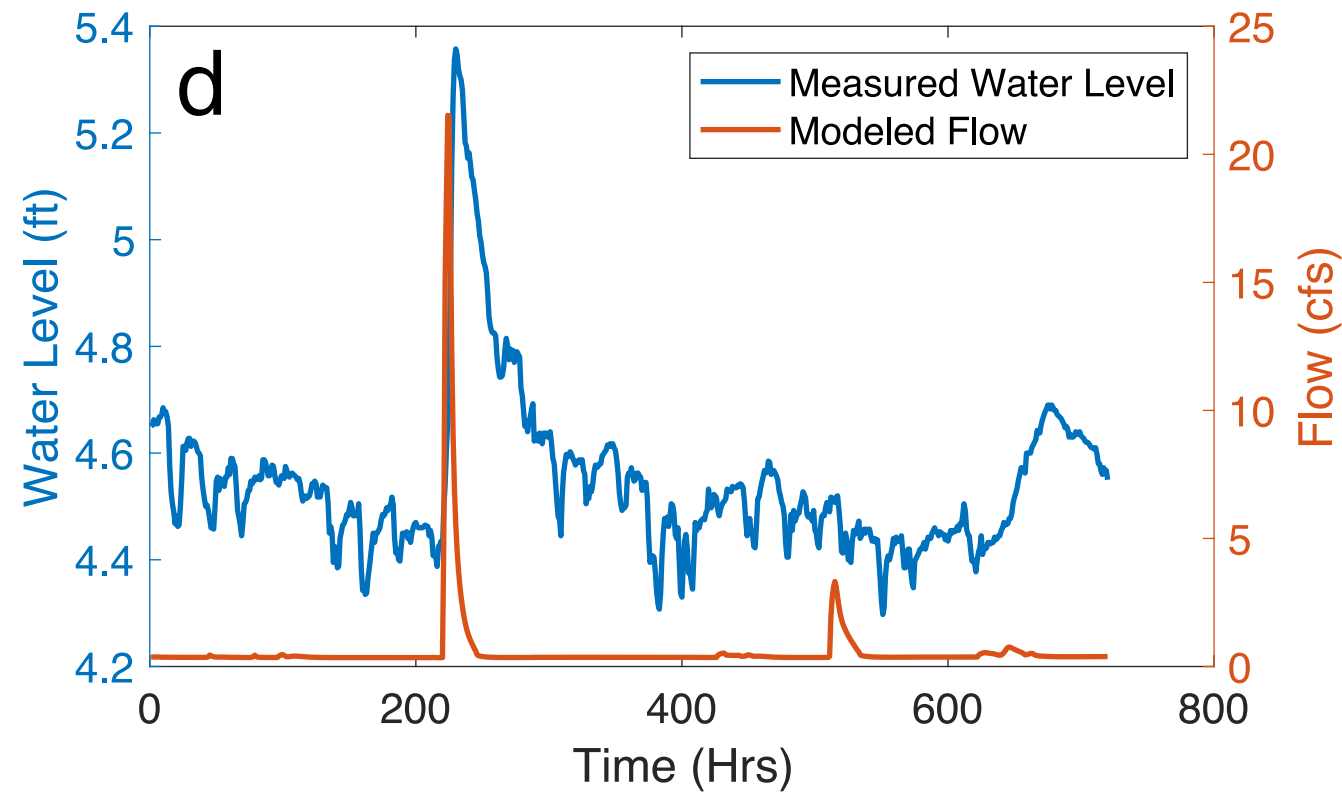
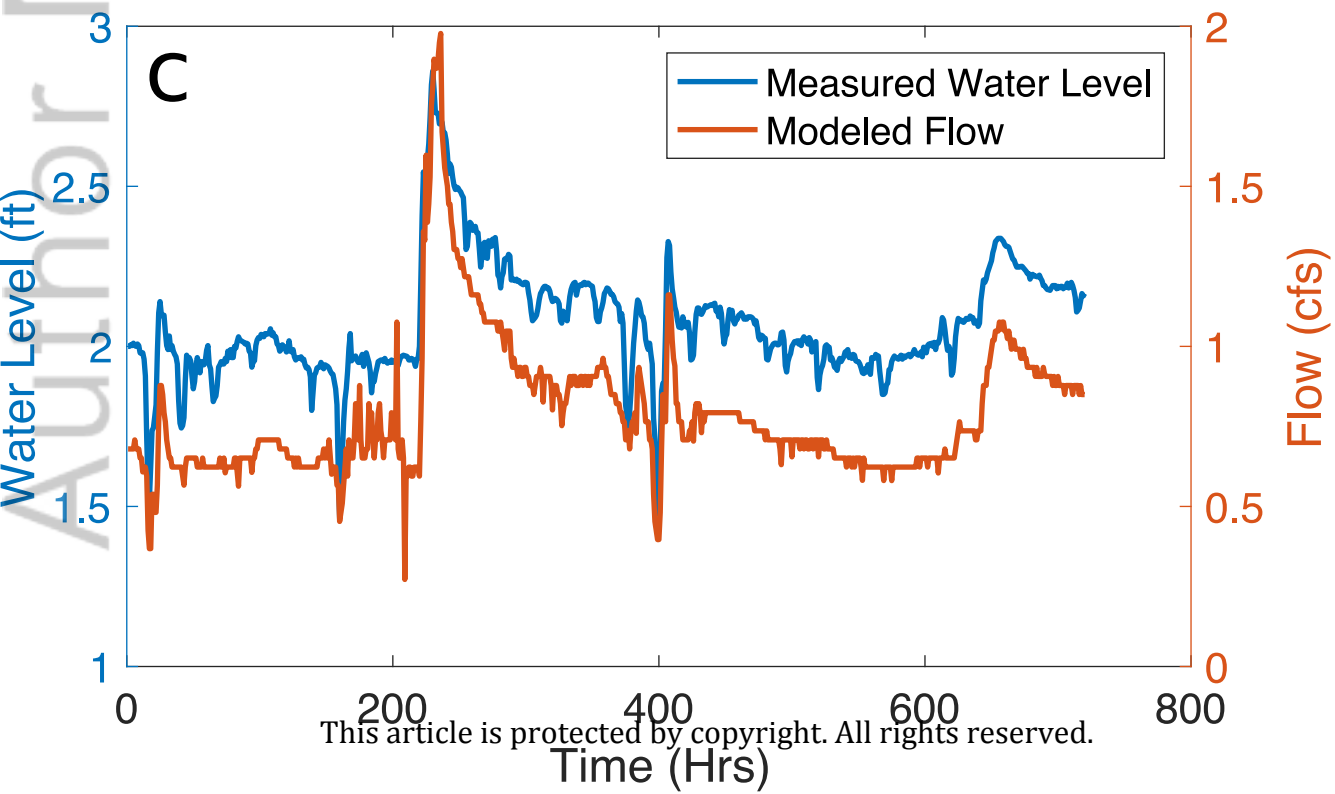
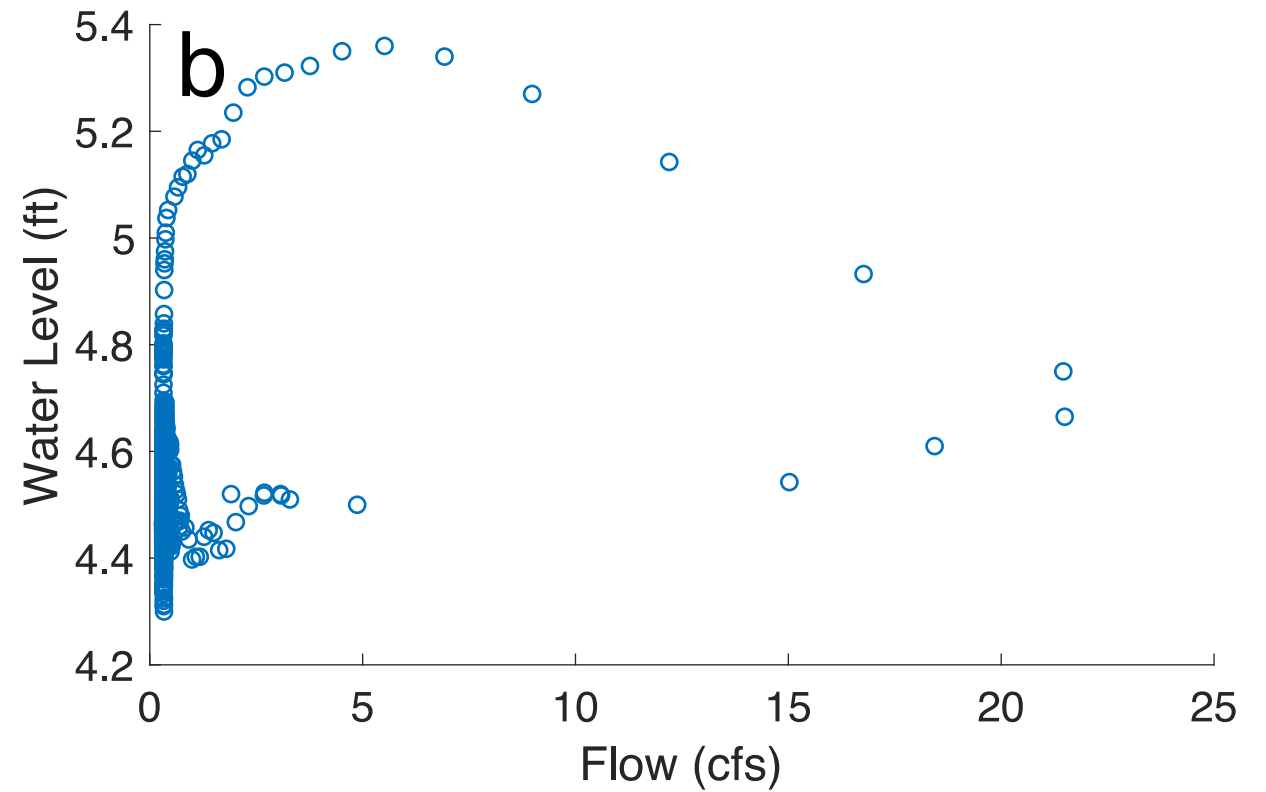


Figure 2.

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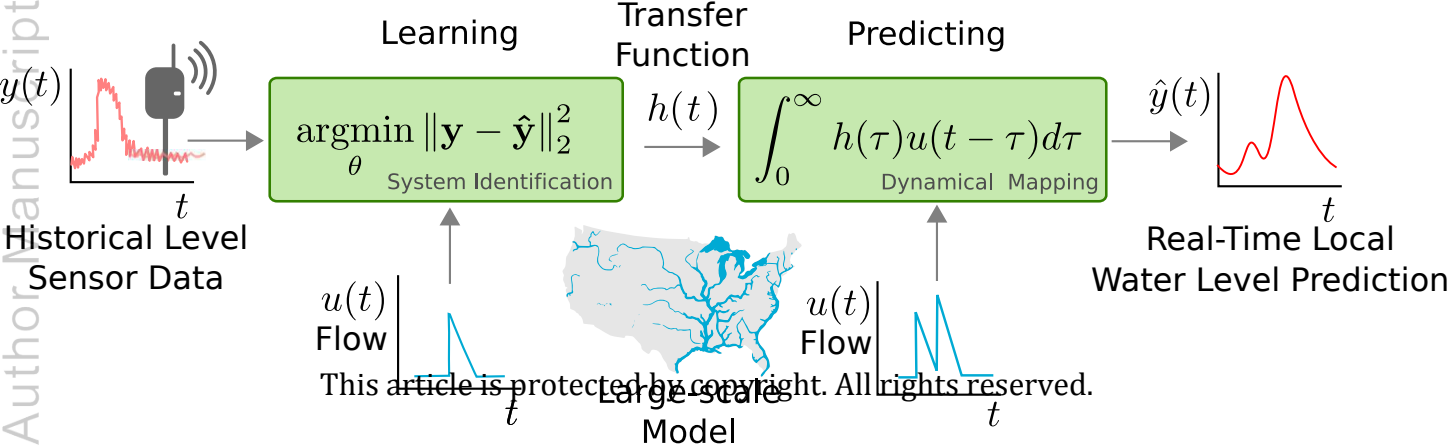


Figure 3.

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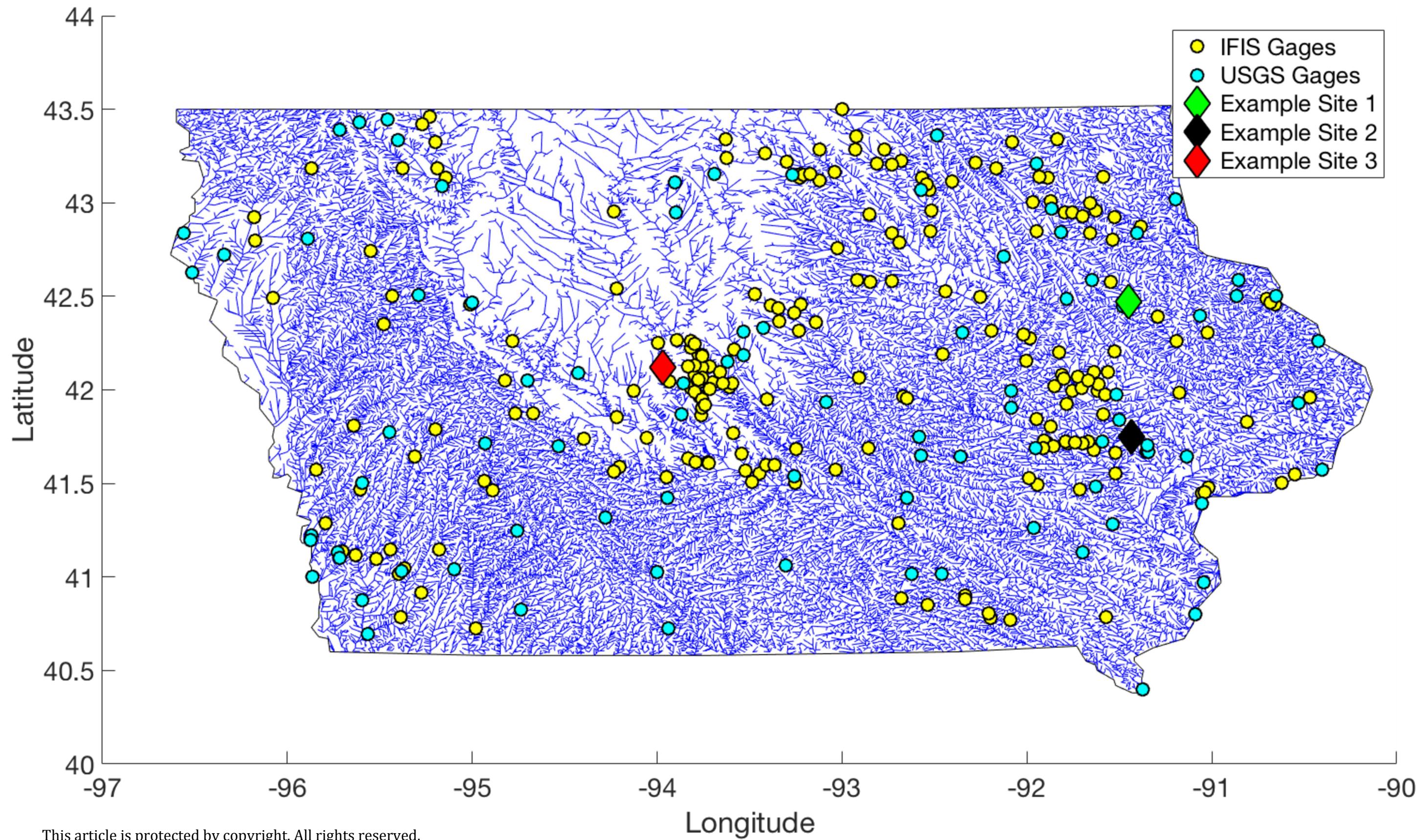


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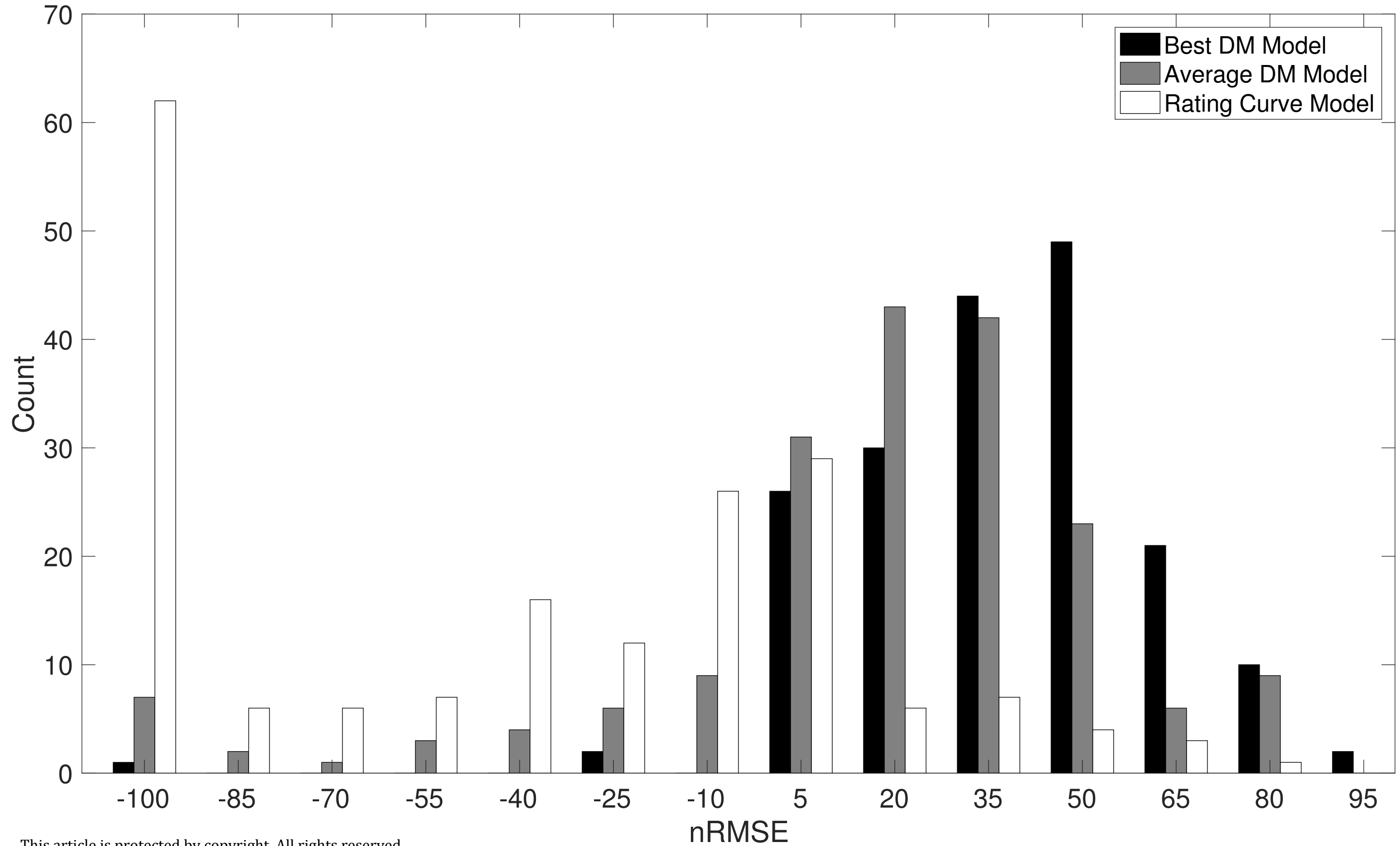
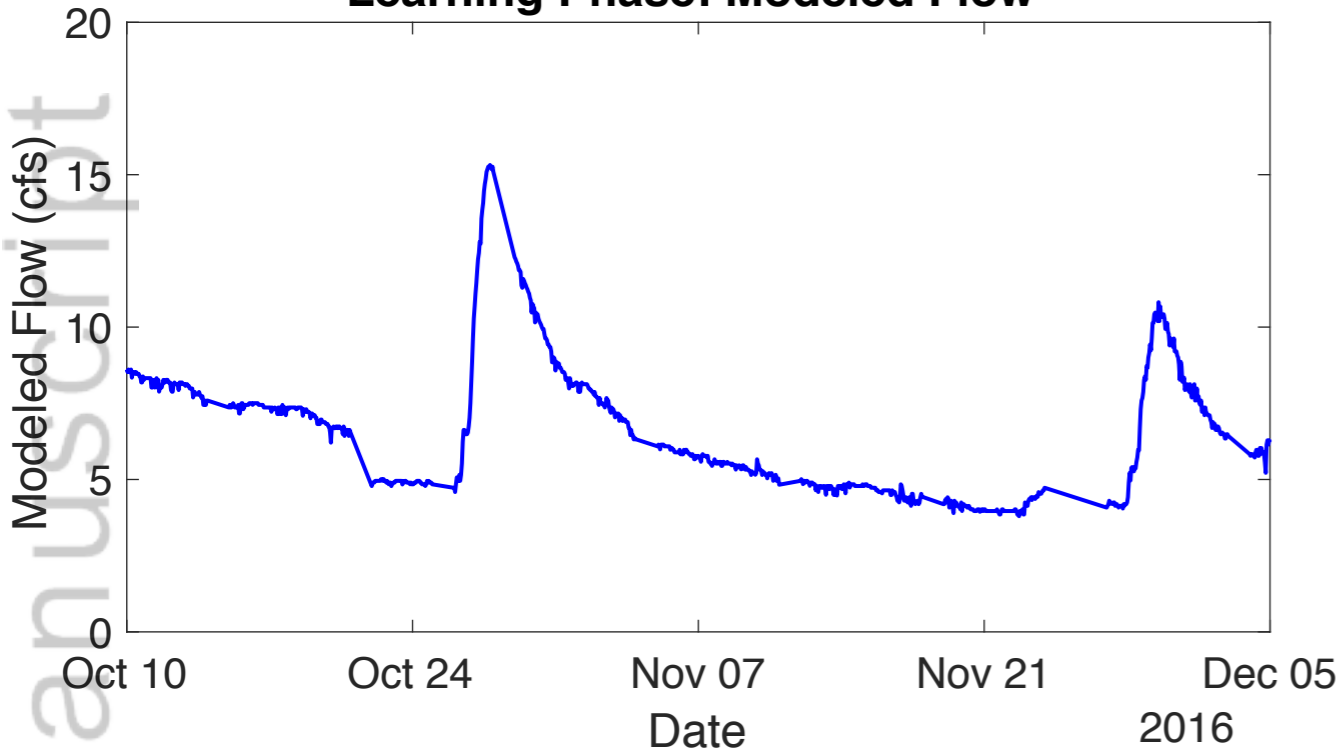


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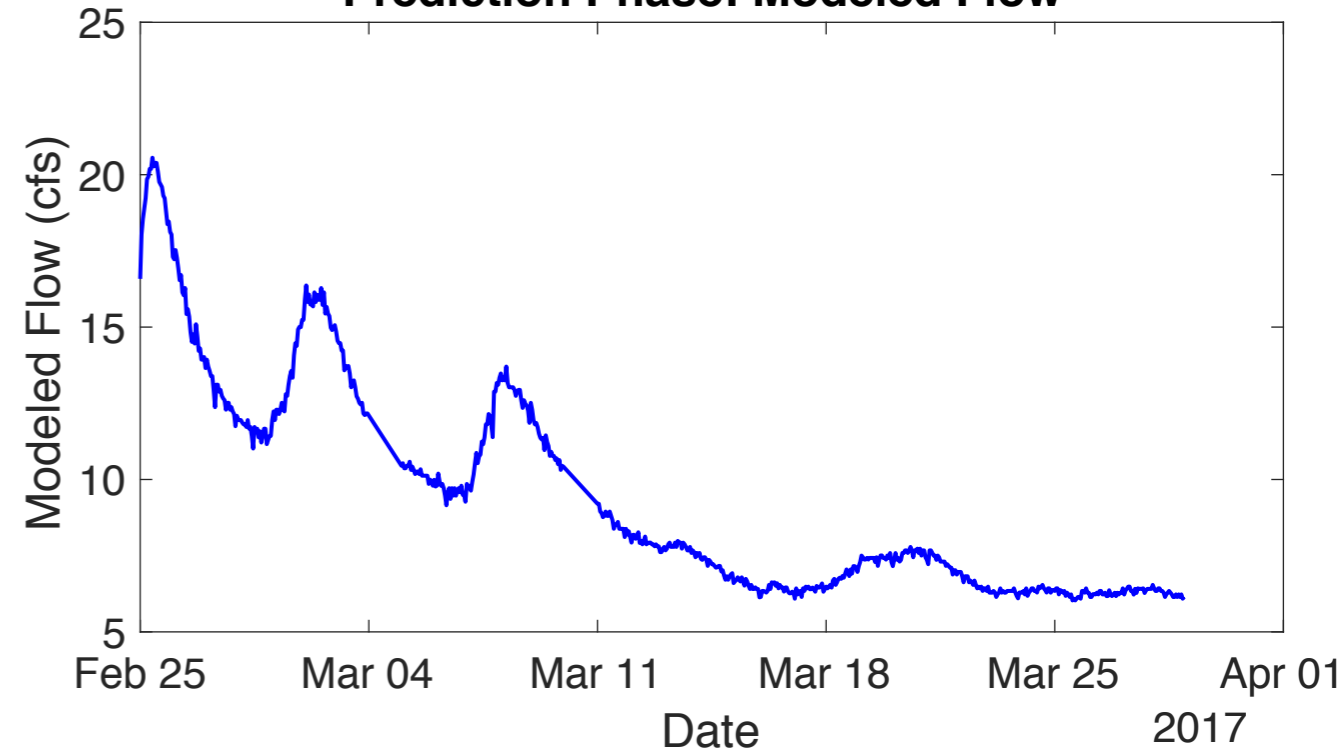
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Dynamical Mapping: Site 1

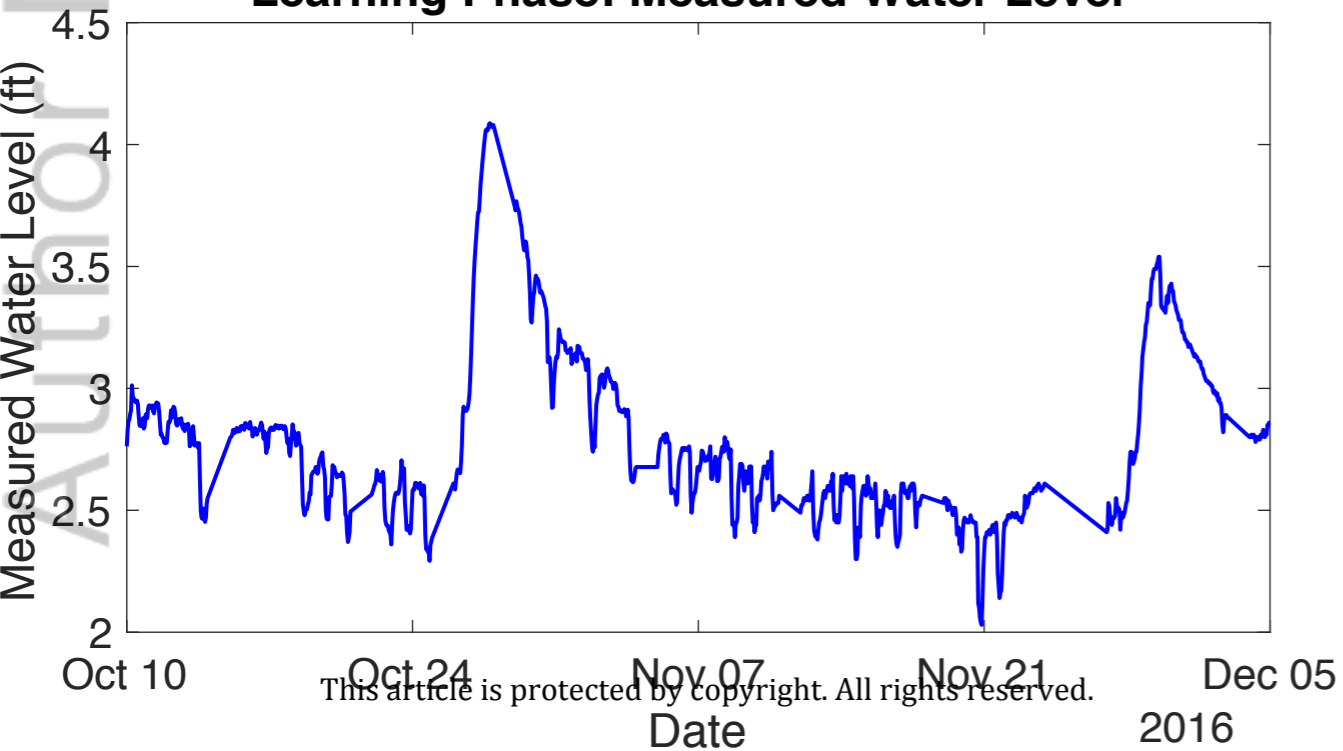
Learning Phase: Modeled Flow



Prediction Phase: Modeled Flow



Learning Phase: Measured Water Level



Prediction Phase: Measured and Predicted Water Level

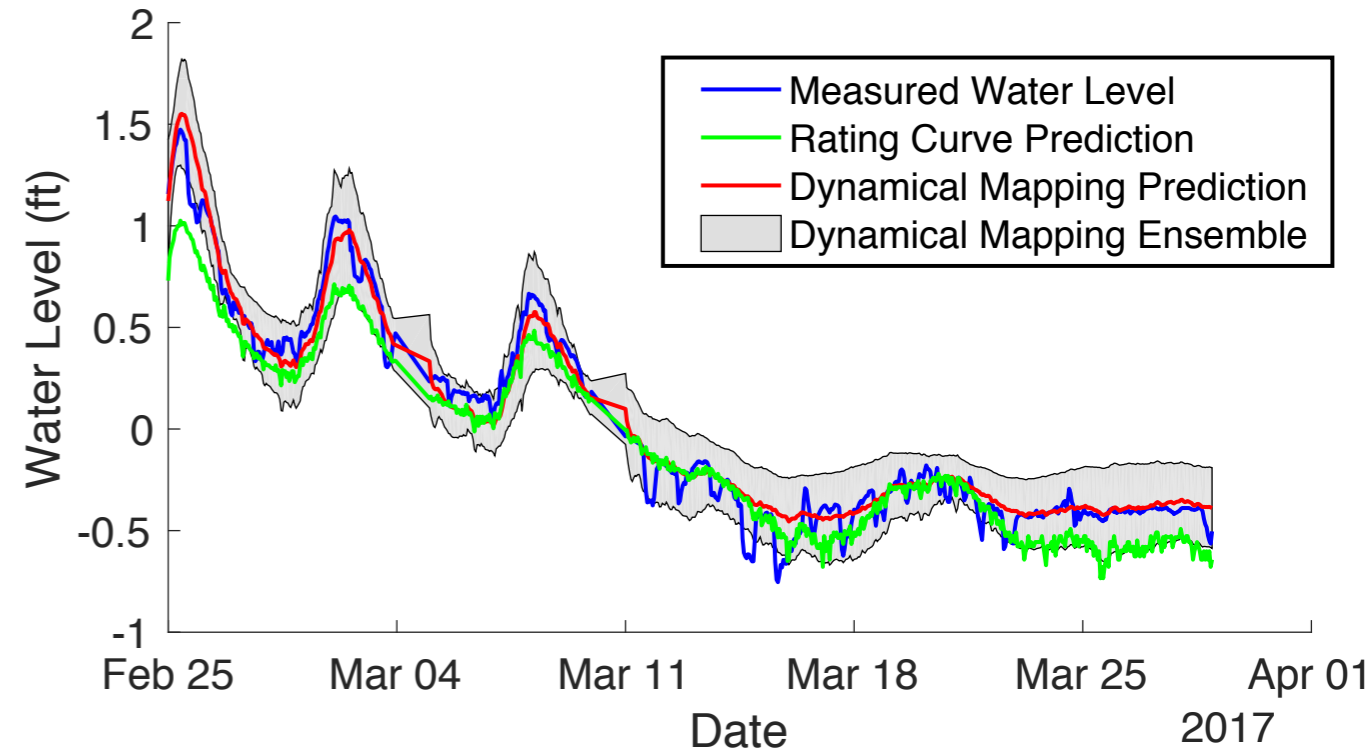
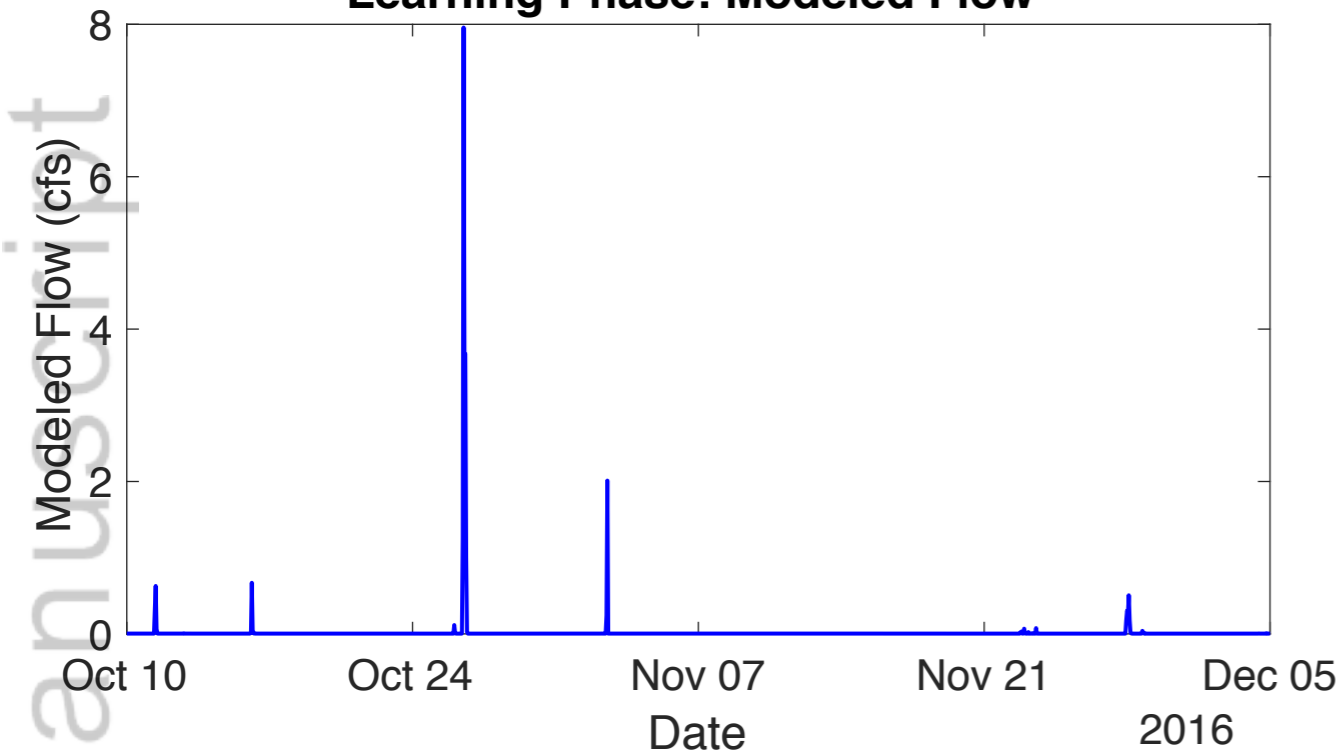


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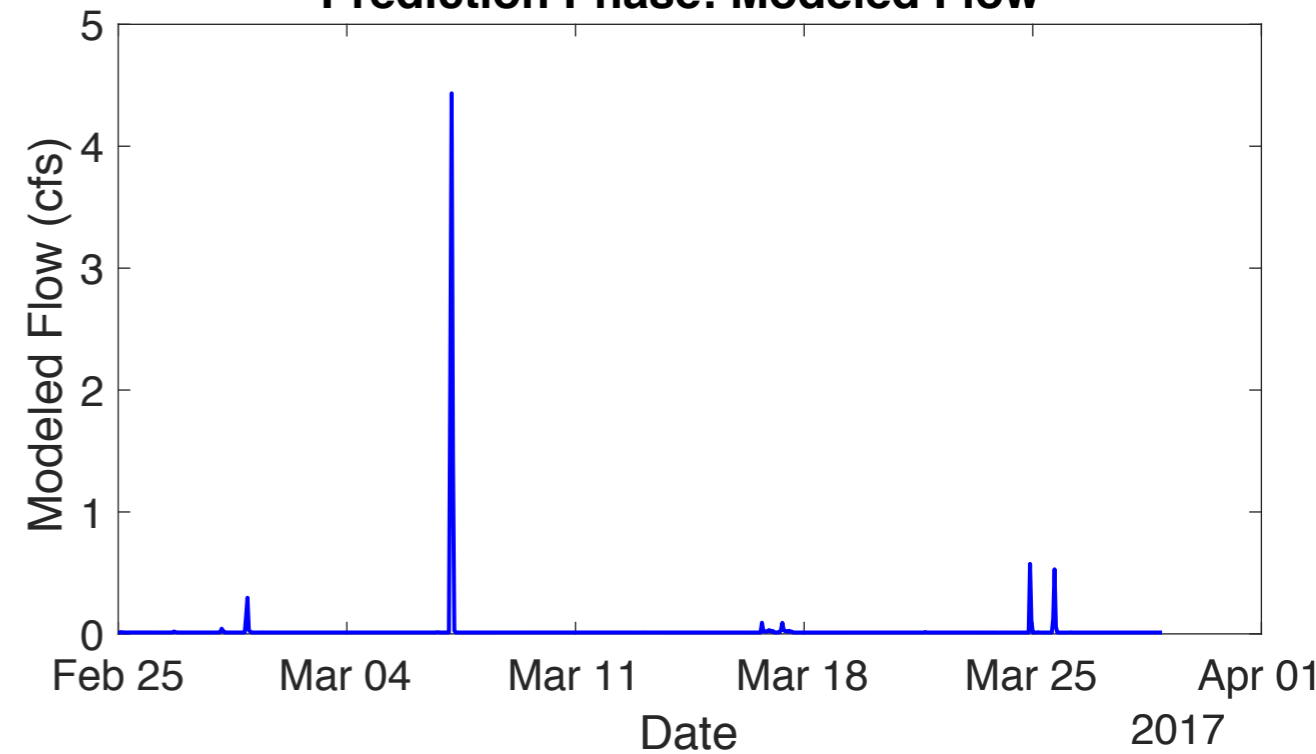
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Dynamical Mapping: Site 2

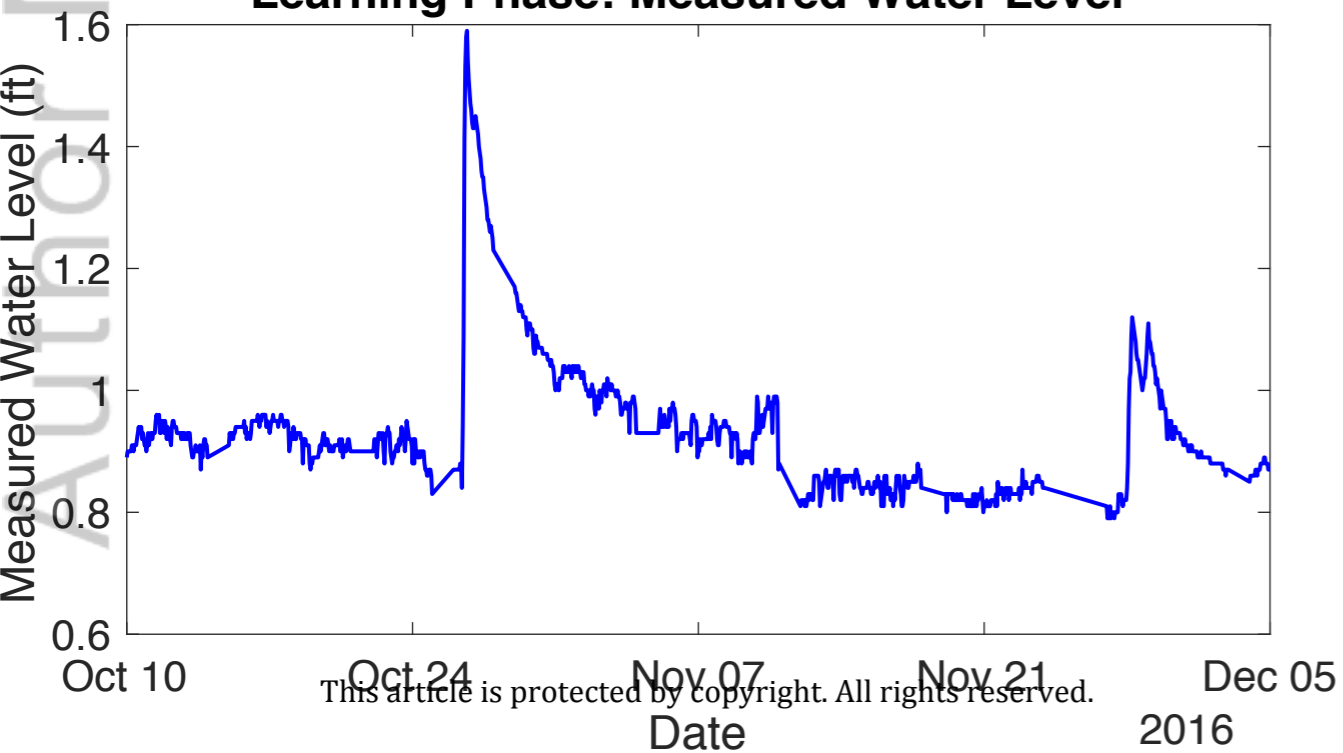
Learning Phase: Modeled Flow



Prediction Phase: Modeled Flow



Learning Phase: Measured Water Level



Prediction Phase: Measured and Predicted Water Level

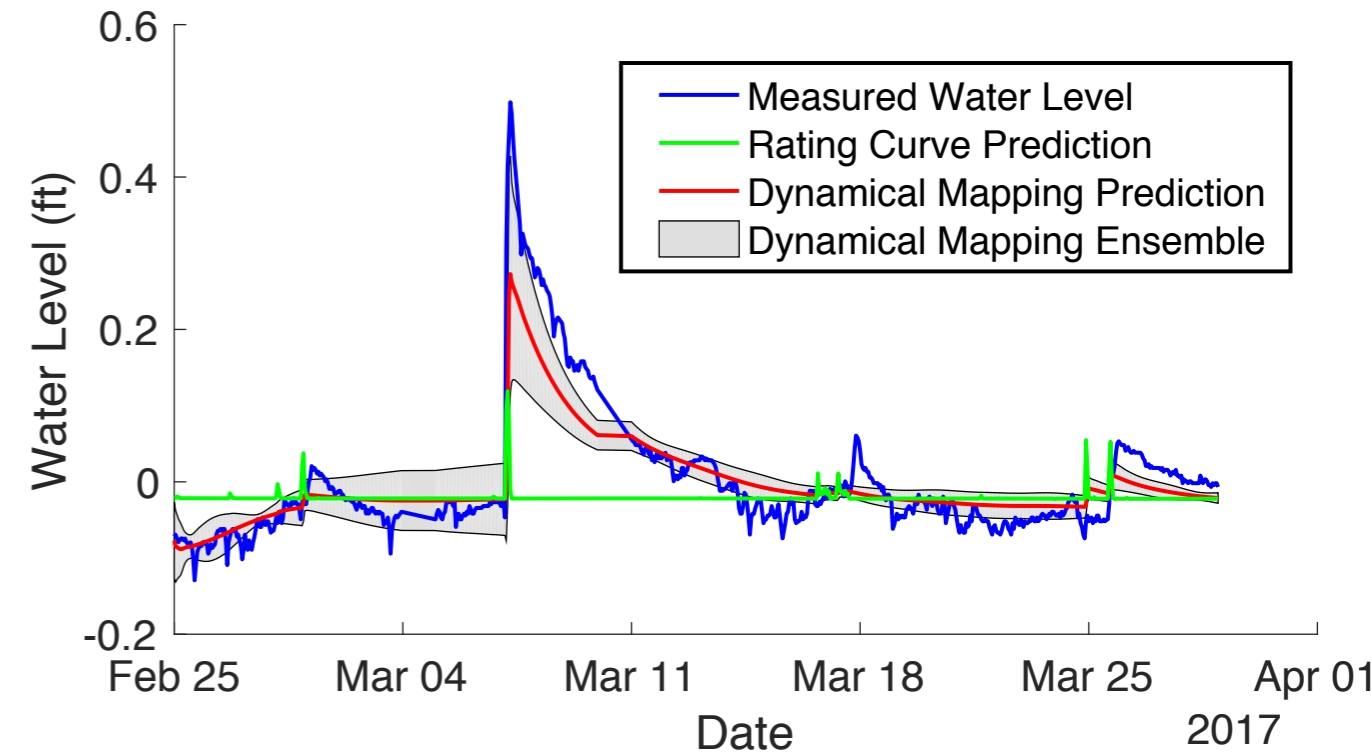


Figure 7.

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Dynamical Mapping: Site 3

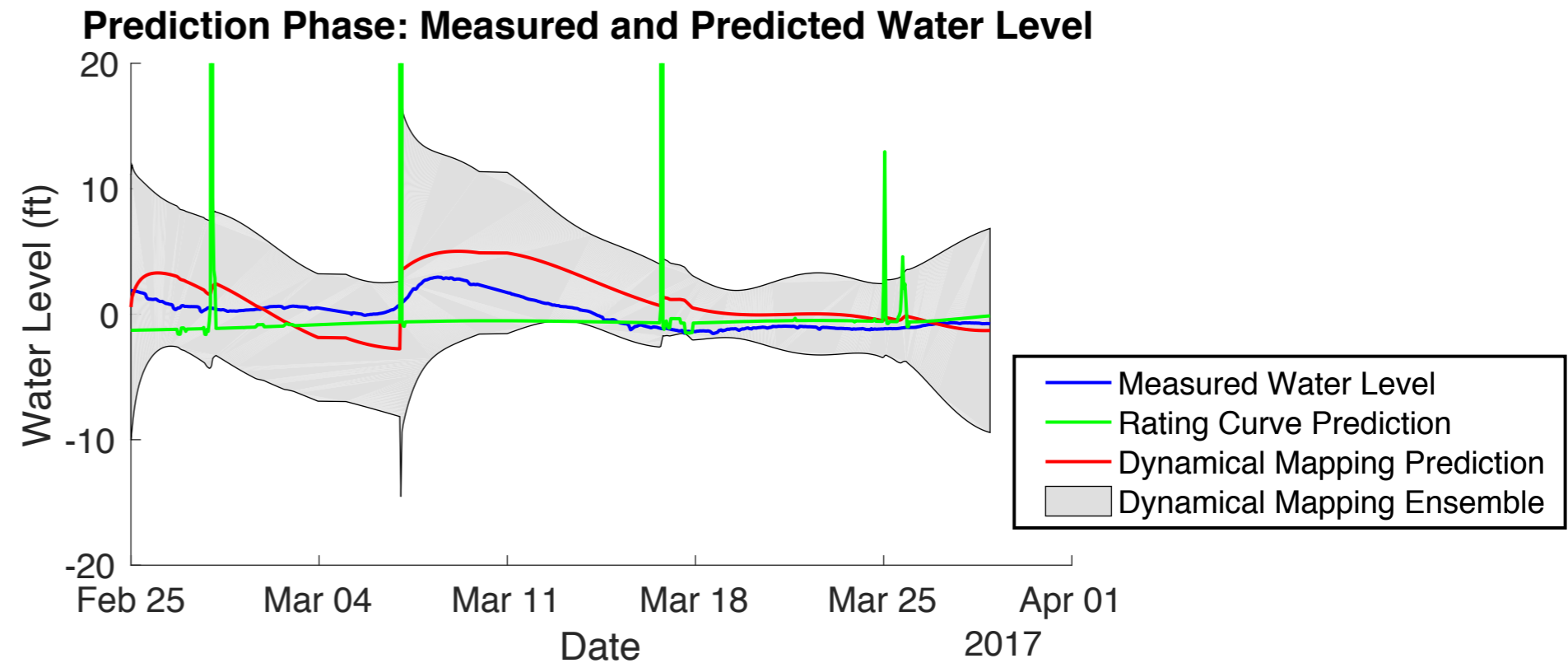
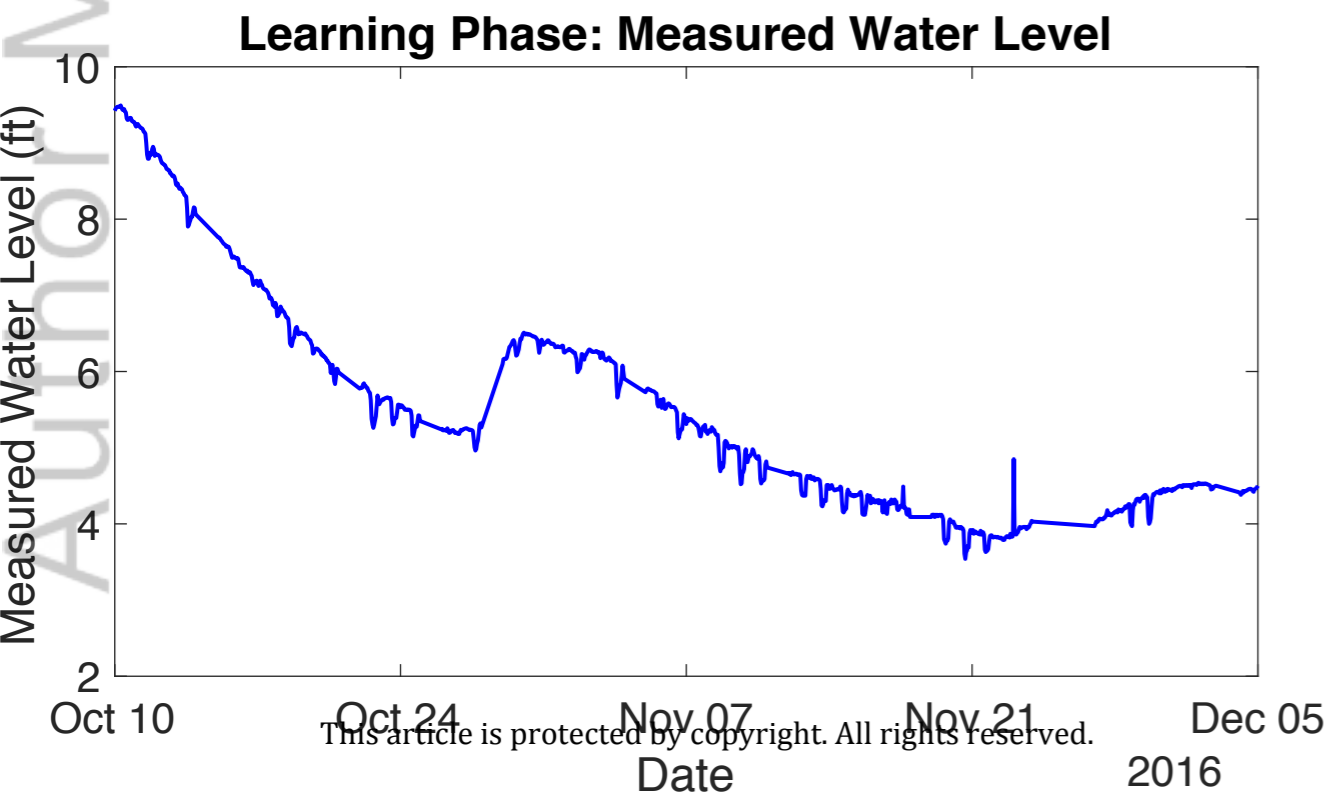
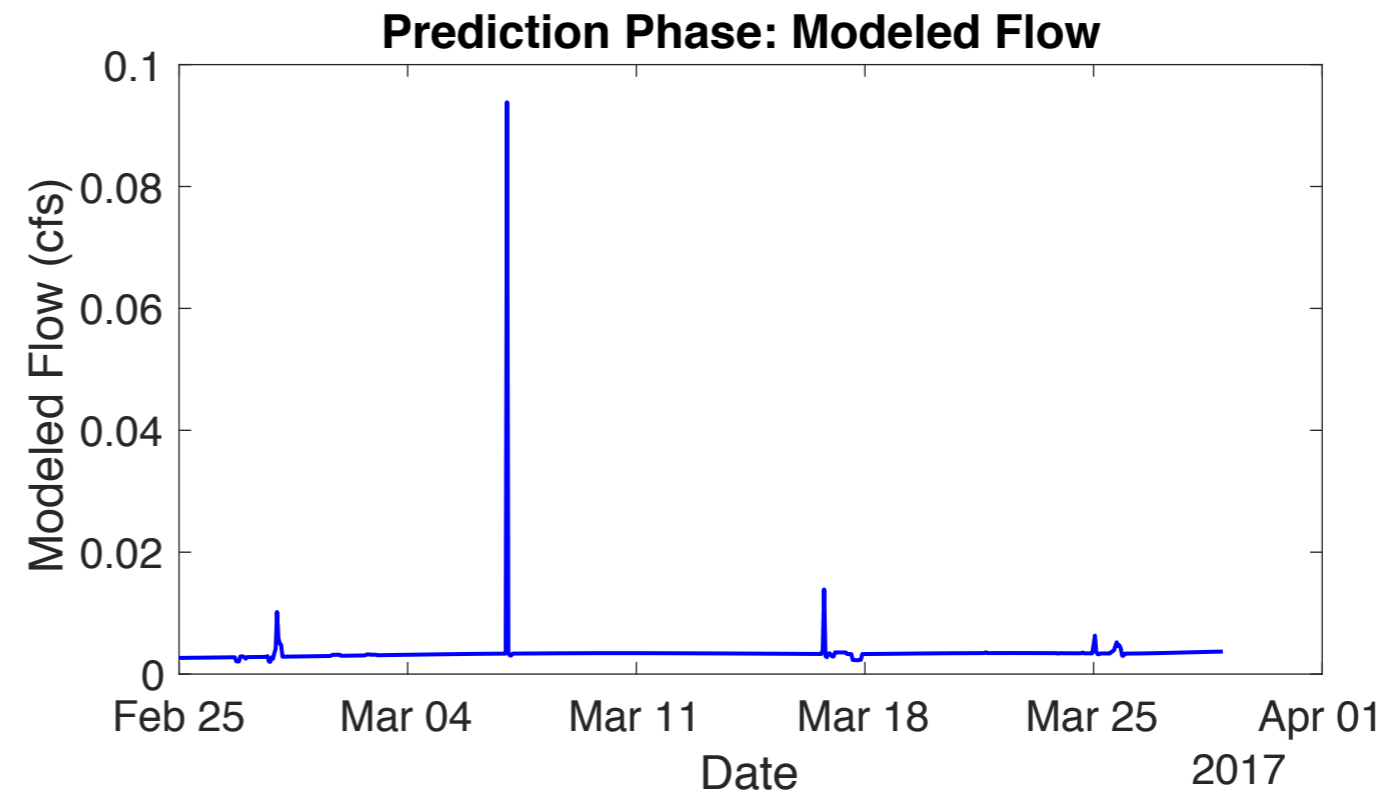
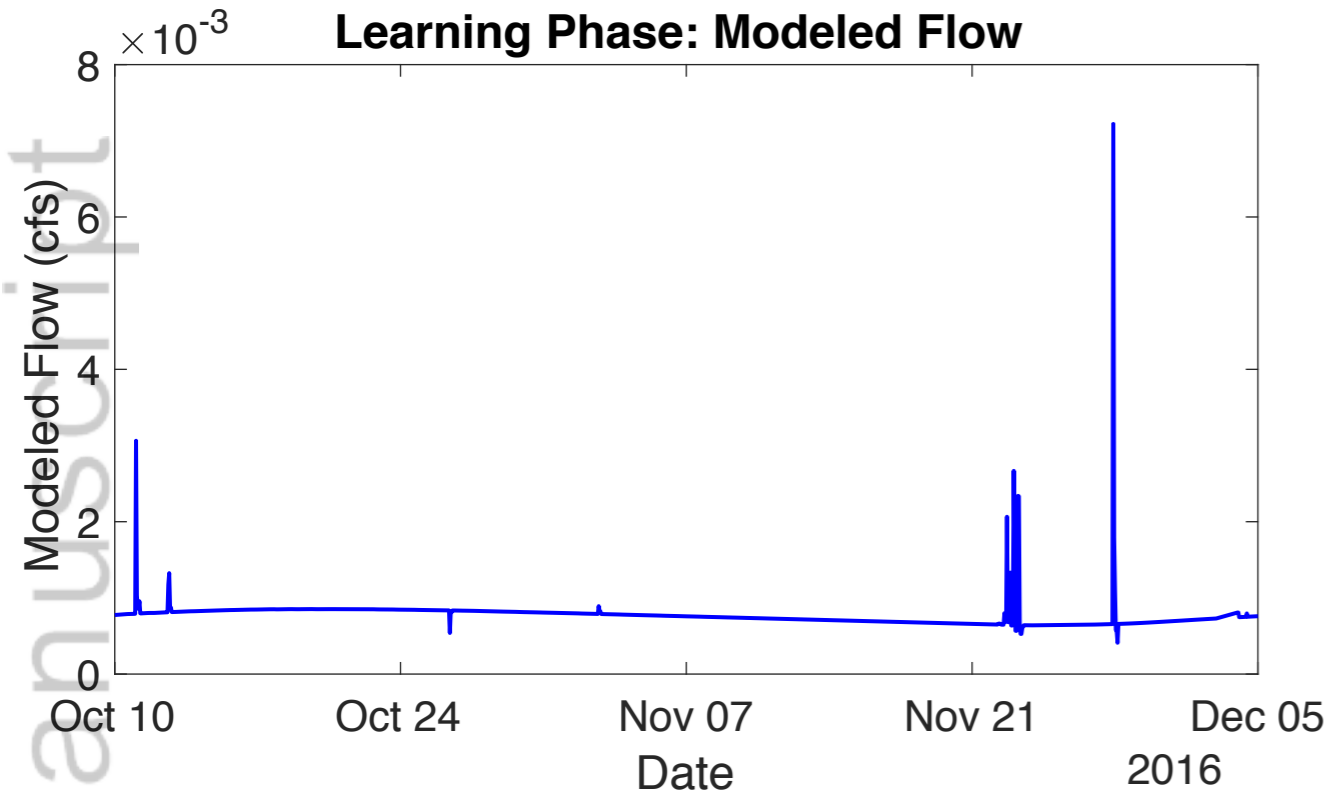
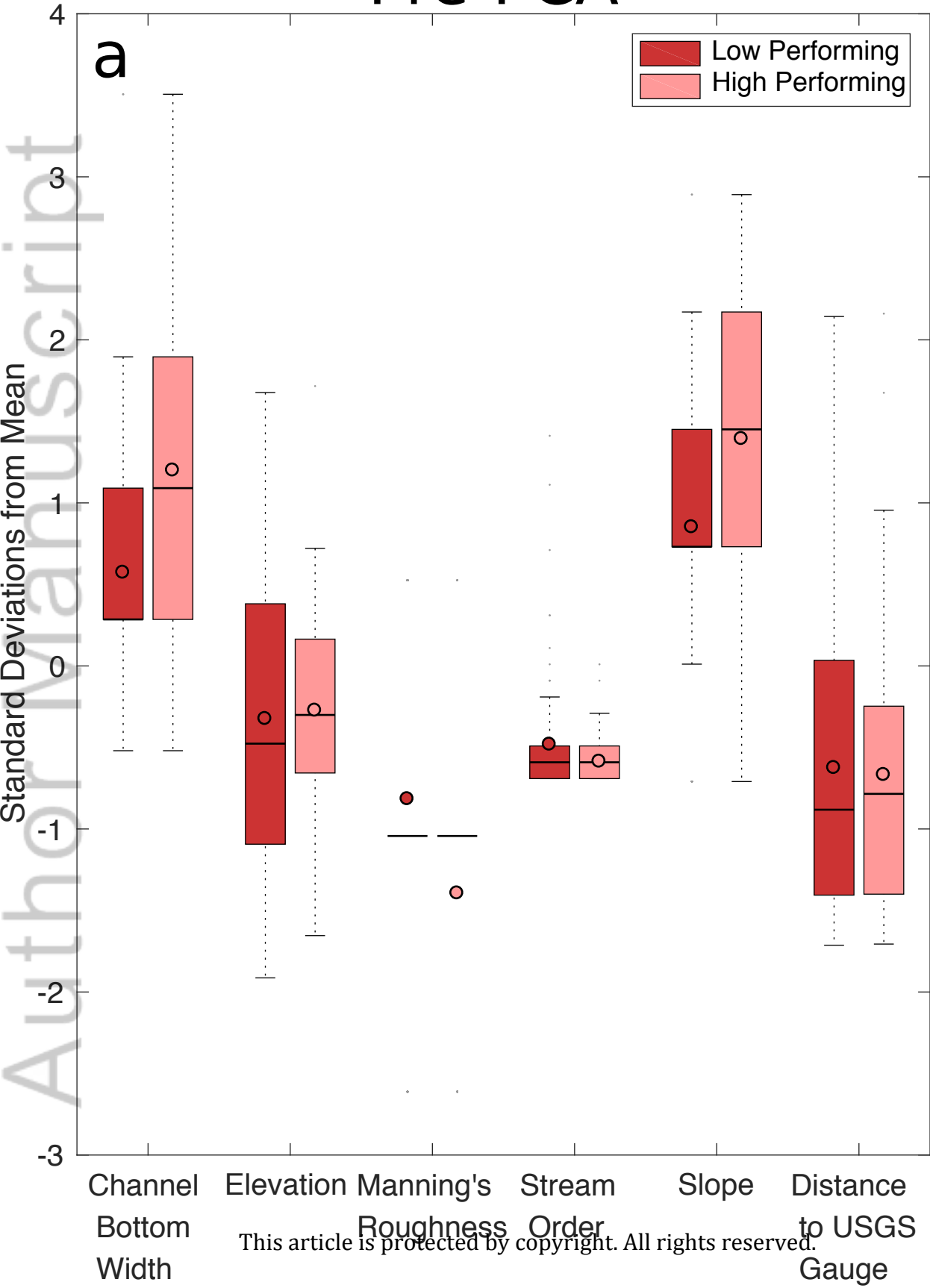


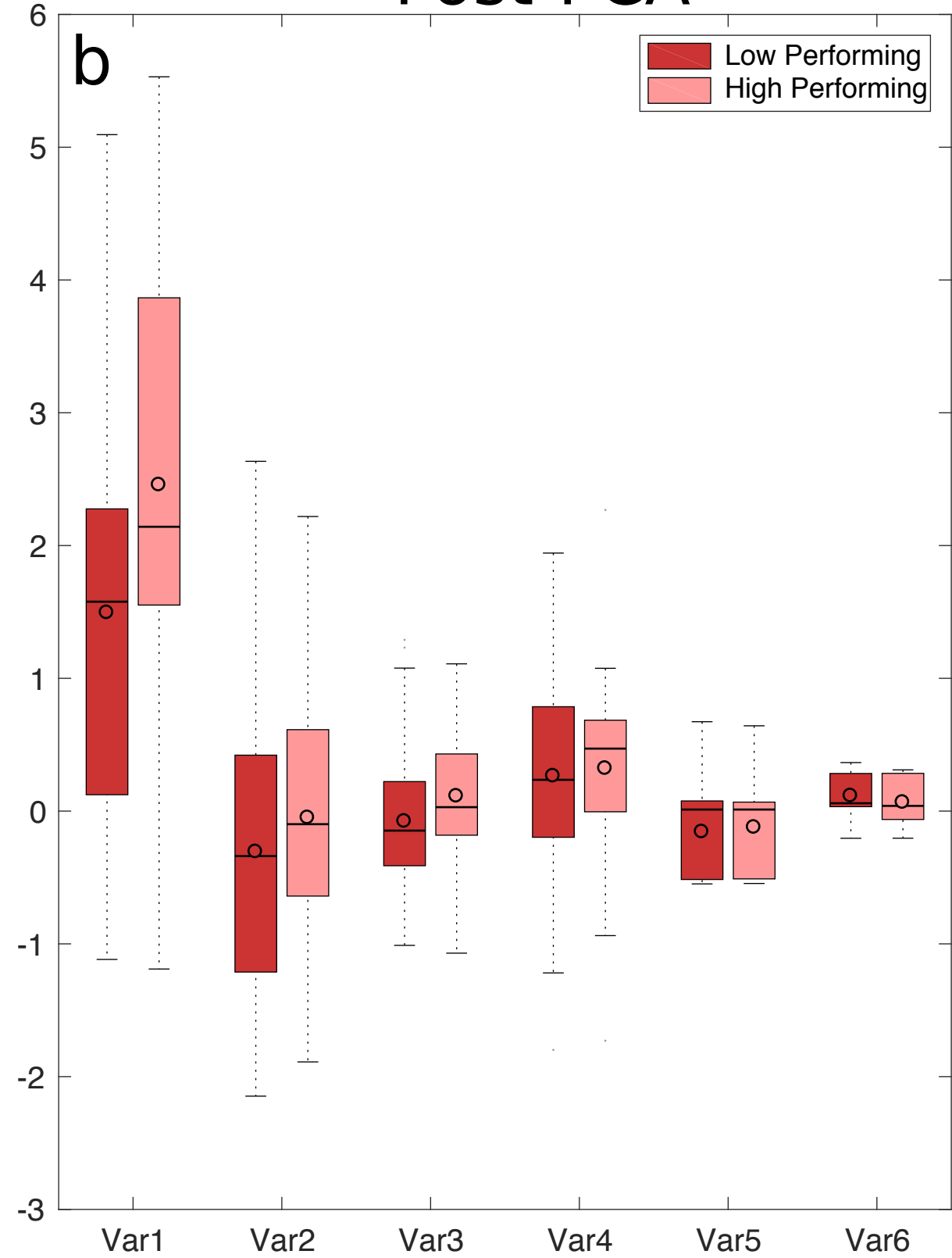
Figure 8.

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Pre-PCA



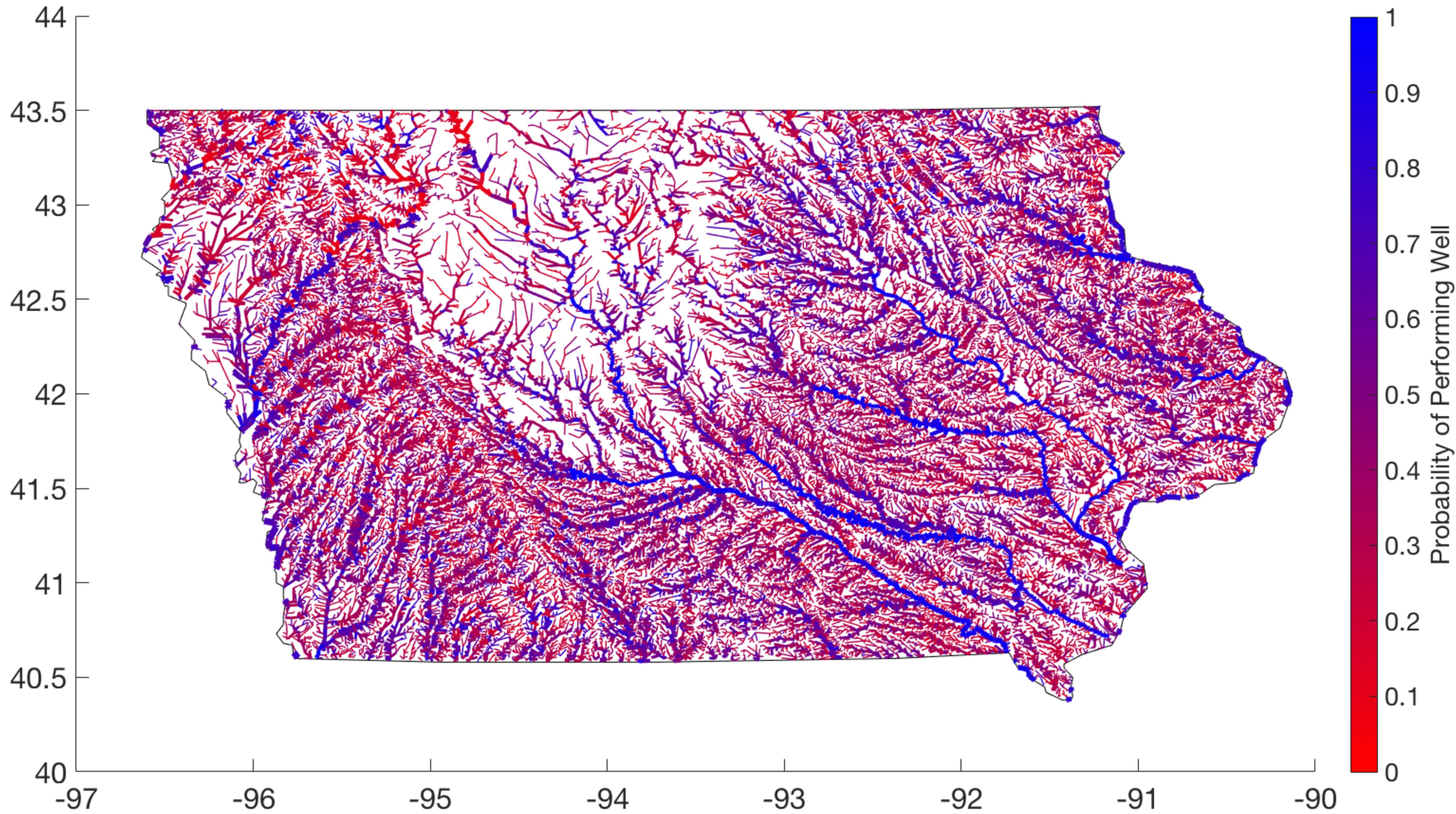
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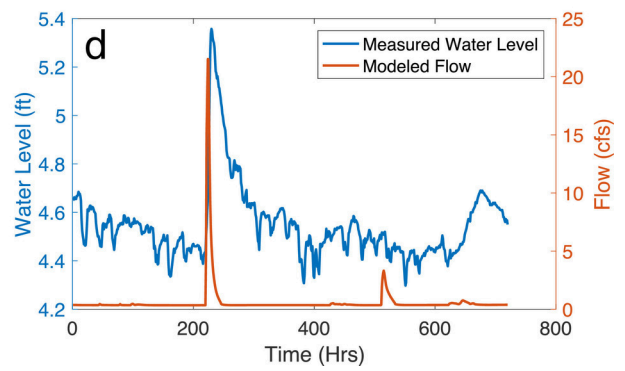
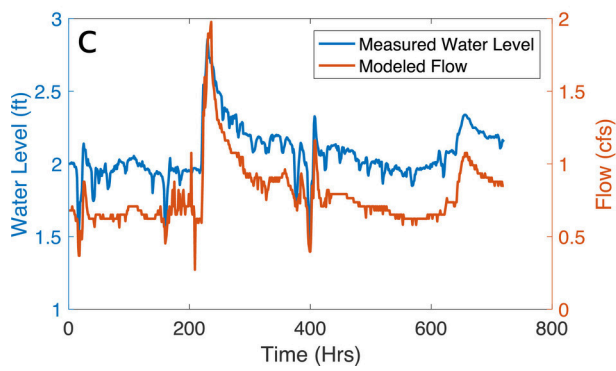
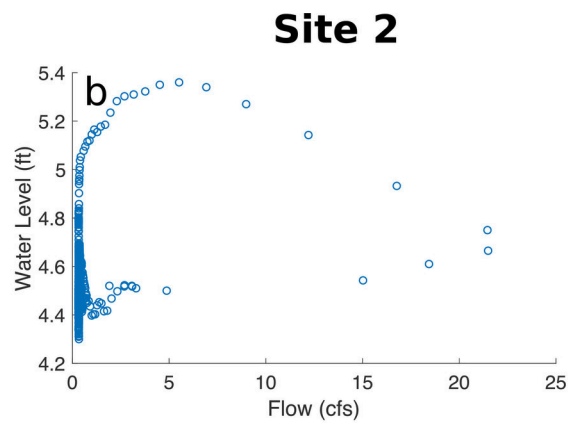
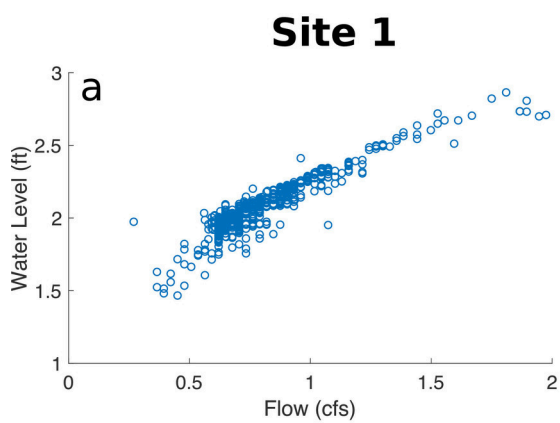


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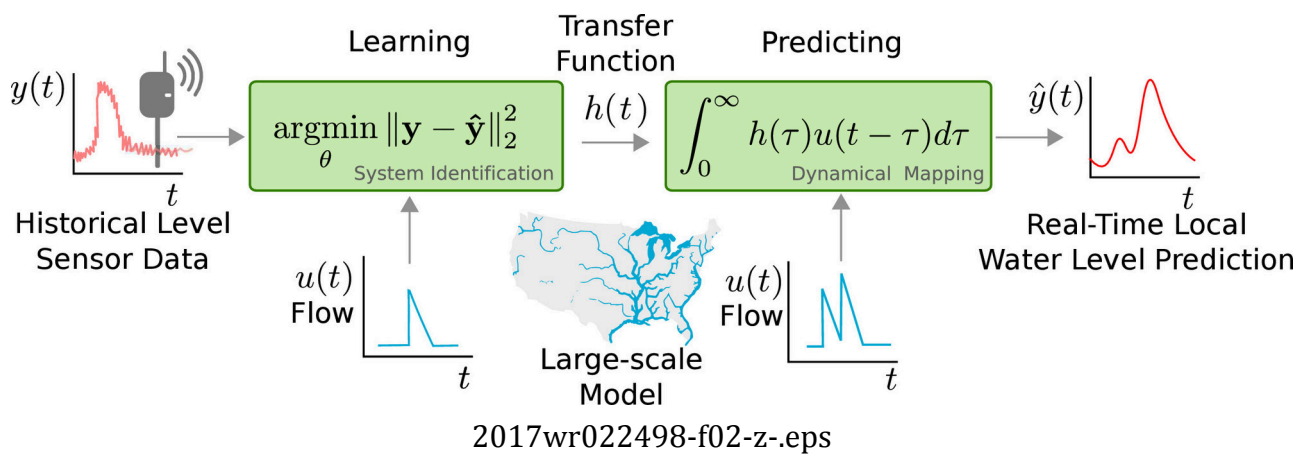
Figure 9.

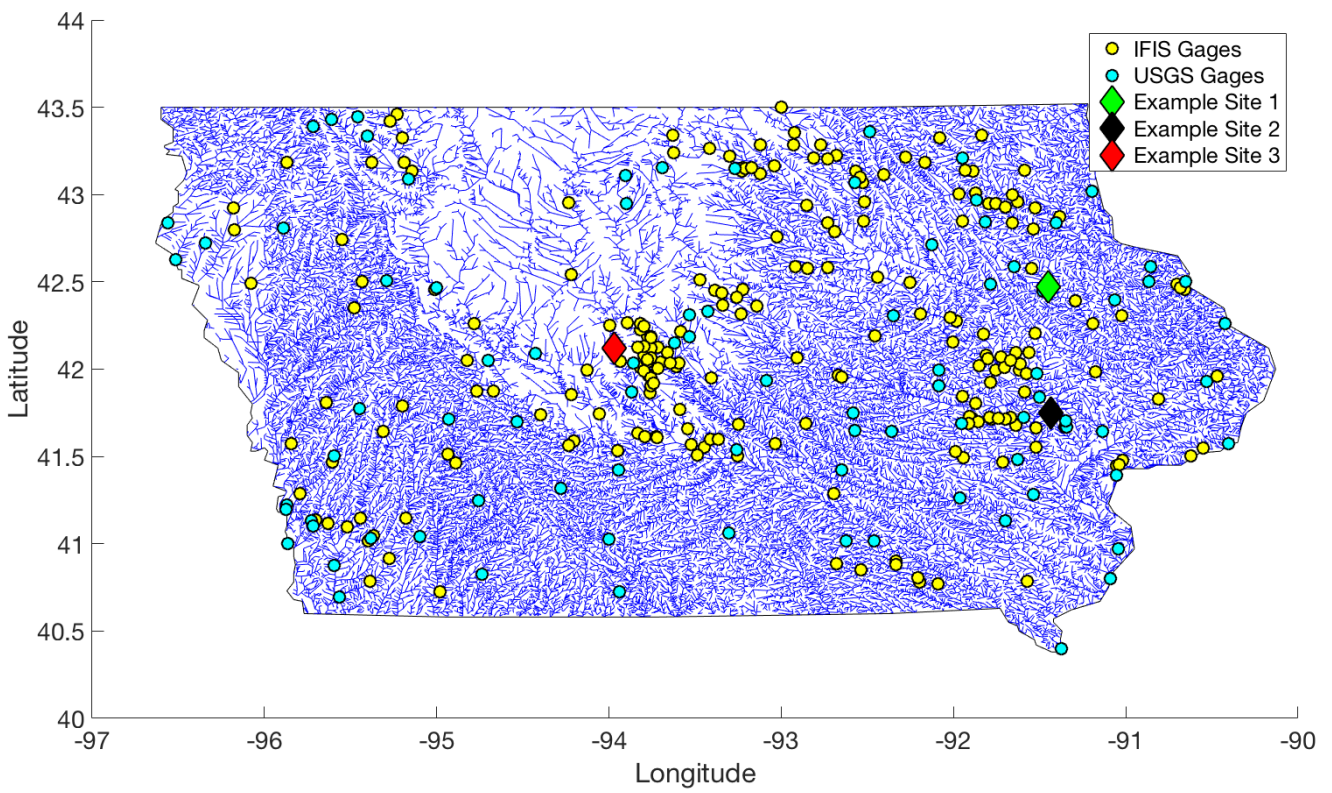
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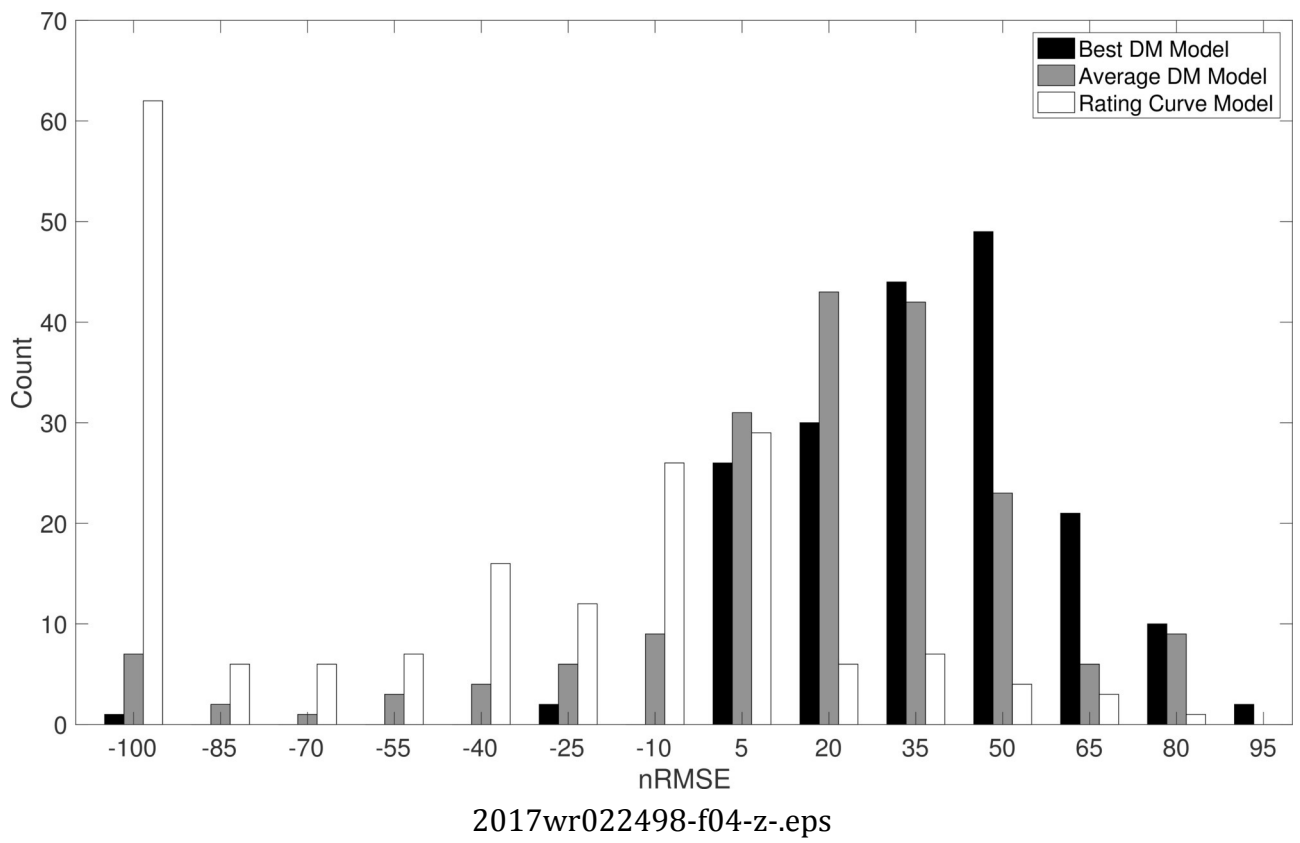


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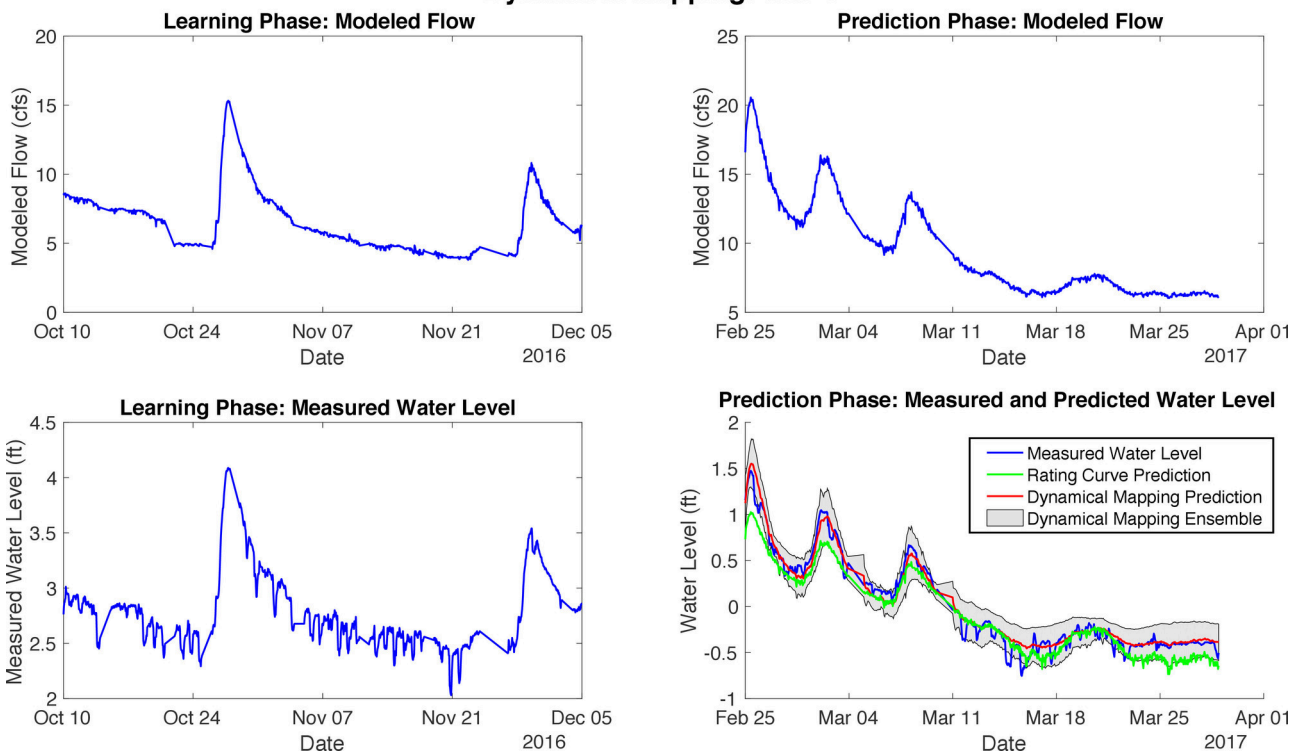




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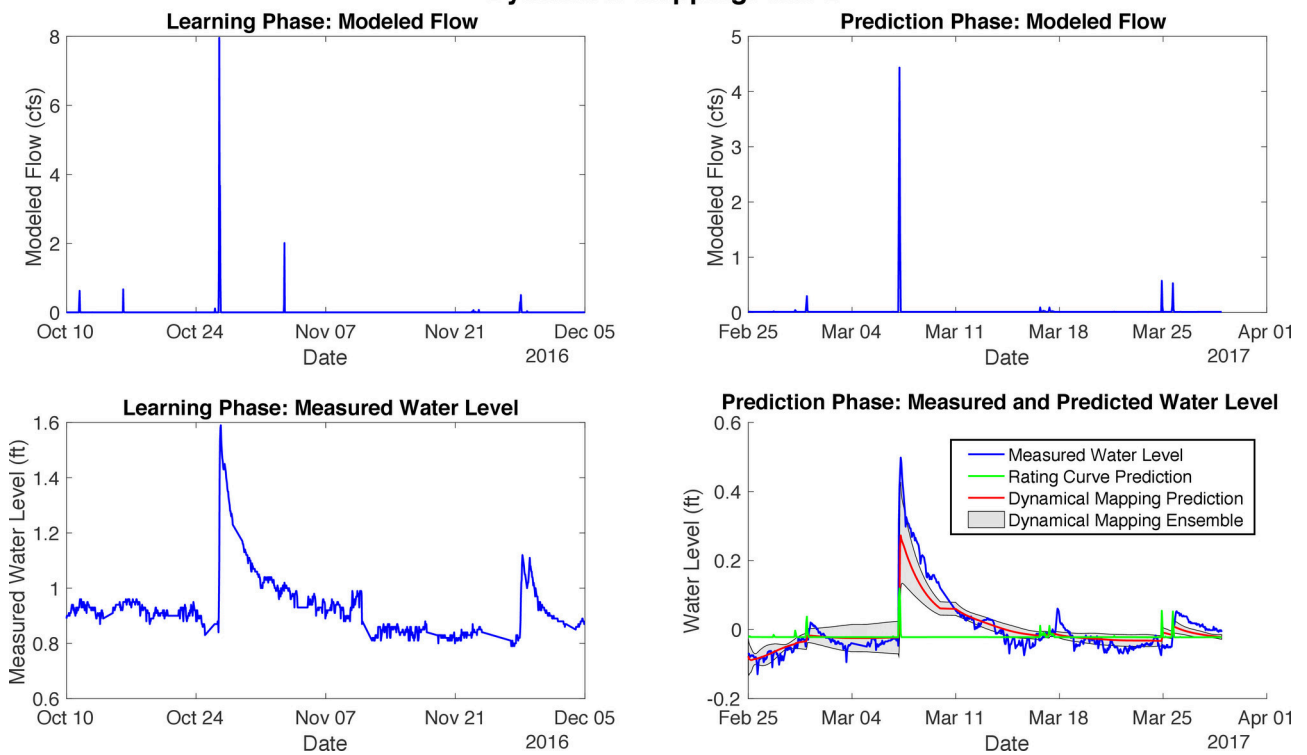


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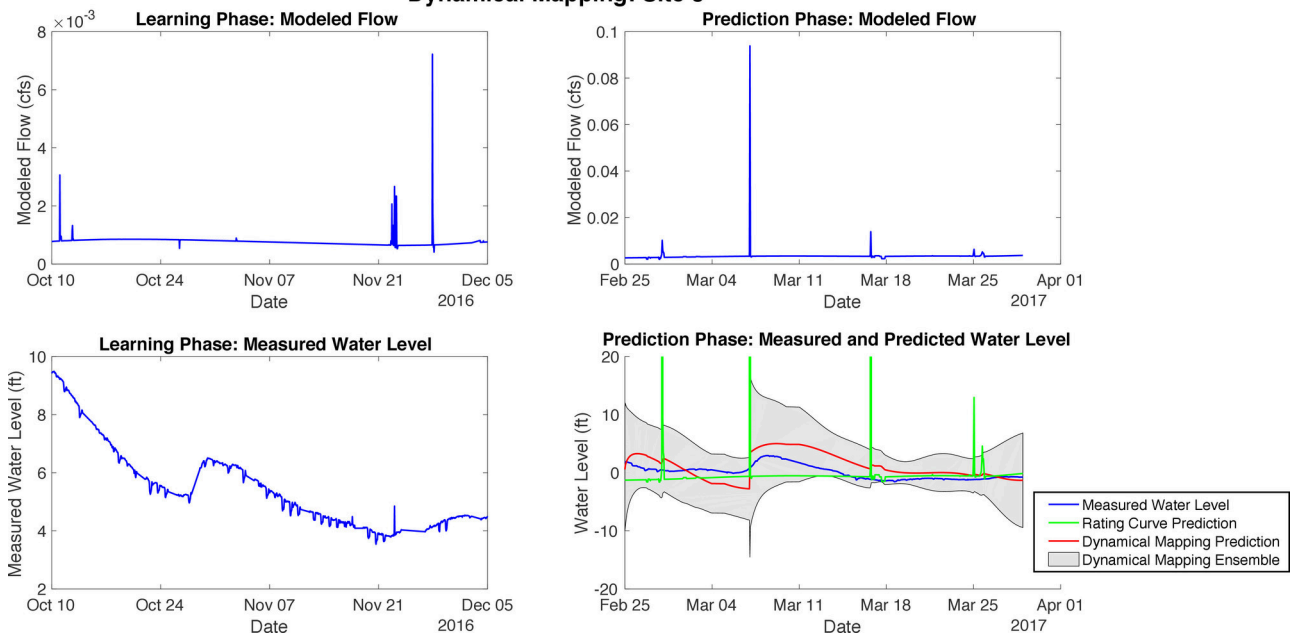
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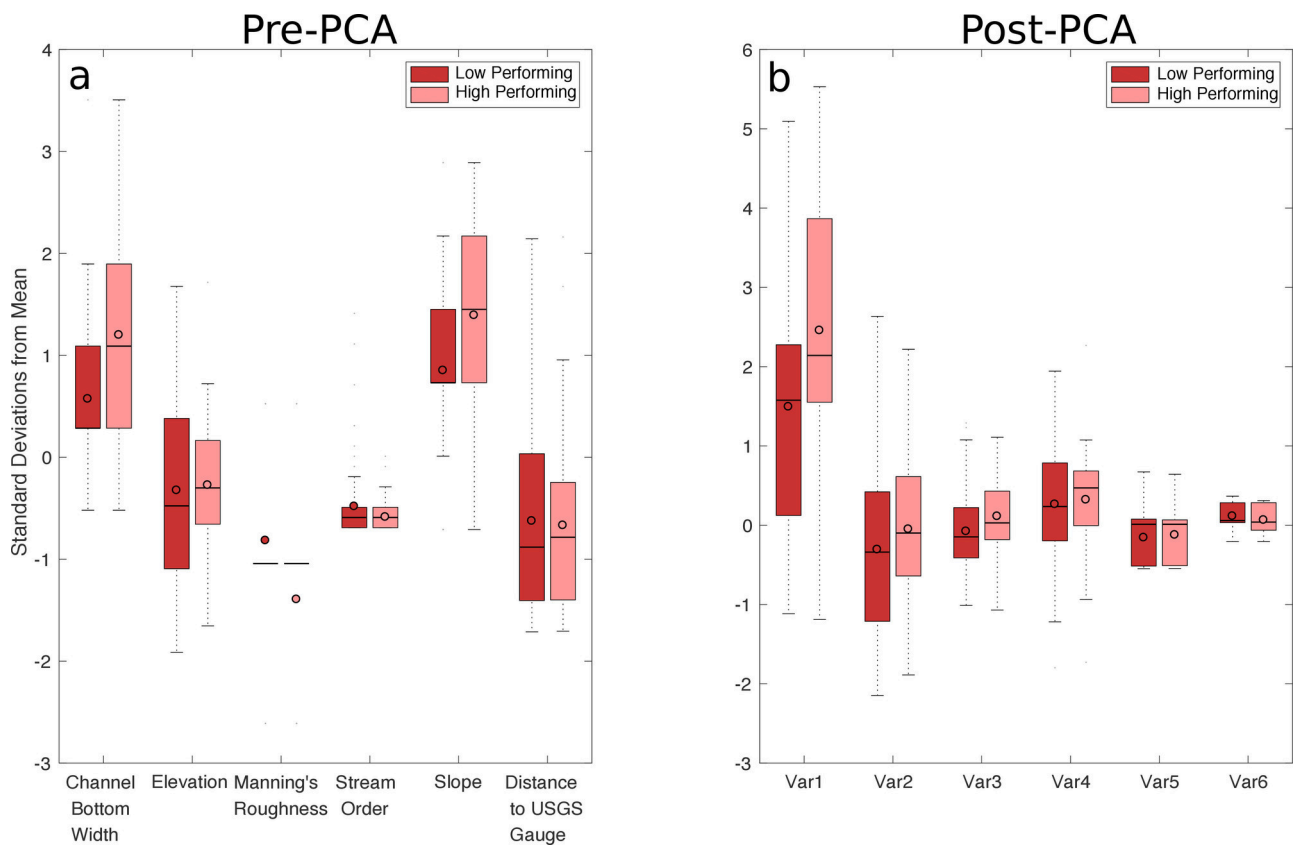


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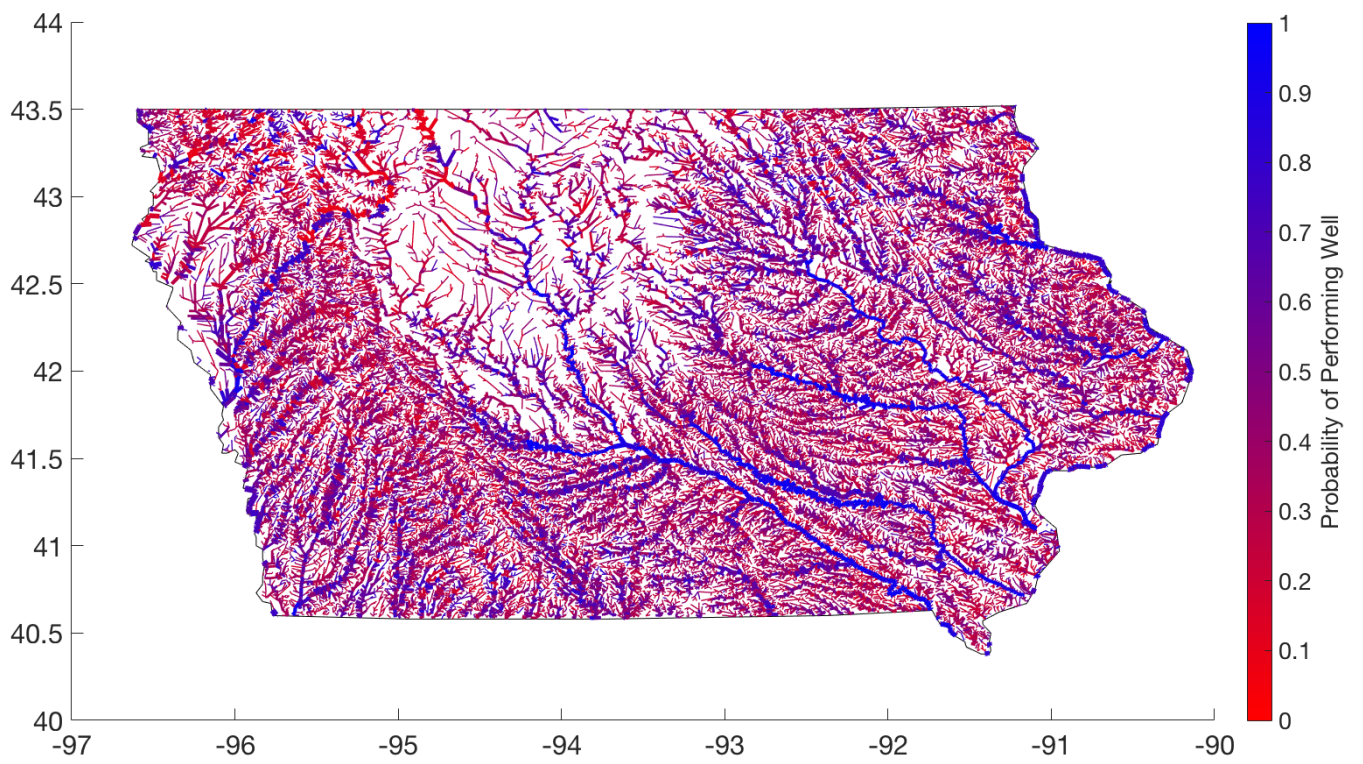
Dynamical Mapping: Site 3



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