ENGINEERING RESEARCH INSTITUTE THE UNIVERSITY OF MICHIGAN ANN ARBOR

Progress Report

THE EFFECT OF VARIATIONS IN AMOUNT OF WATER ADDED TO FLY-ASH CONCRETE MIXES

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SYNOPSIS

In previous investigations of the use of Detroit Edison Company fly ashes in portland-cement concrete it appeared possible that an increase in the mixing water, while producing a more fluid mix, did not produce a commensurate reduction in the compressive strength of the concrete. As a result, a study was made in which concrete mixes designed for a 4-inch slump were made with the water added at the mixer increased or decreased from the designed quantity to produce wet and dry consistency mixes. The base mixes were so selected that the concrete of a given consistency had approximately the same early compressive strength whether containing fly ash or not.

The results obtained indicate that there is no appreciable difference between the behavior of the mixes containing fly ash and the plain-cement mixes with respect to added water. The addition of extra water to produce the wetter mixes was equally detrimental to the compressive strength of both the fly-ash and plain-cement concrete.

OBJECTIVE

The purpose of this investigation is to study the properties of fly ash and concrete containing fly ash. The specific object of the research is to determine the effect of fly ash in concrete, as indicated by compressive strength, workability, durability in freezing and thawing, flexural strength, and other measures of concrete quality.

INTRODUCTION

This progress report is one of a series on the use of fly ash from The Detroit Edison Company power stations in portland-cement concrete. Presented herein are the results of a study to determine the effect of variations in the amount of water added to mixes of non-air-entrained concrete containing fly ash from the St. Clair station. The investigation was made by the Engineering Research Institute of The University of Michigan pursuant to a contract between the Institute and The Detroit Edison Company. This report covers only such matters as mix design, tests on the fresh concrete, and compressive strength through 90 days' age. Cylinders for testing at an age of one year were made and will be tested at the proper time.

Some matter pertaining to test procedures which was covered in earlier reports will be omitted here to avoid repetition.

This investigation was intended to be in the nature of a pilot study, making a minimum number of mixes, and the scope can be extended if the results warrant such action.

The method employed in this study consisted of making concrete of three consistencies from each of three mix designs by varying the amount of water added at the mixer. This gives the effect of a mix in which part of the water is omitted, making a drier mix, and one in which extra water is accidentally or intentionally added to give a wetter consistency. These can then be compared with the batch made of average consistency.

The case with too much water is the more prevalent, particularly in transit-mix operations, where some of the wash water may not be discharged prior to charging the mixer with a new batch, or where the purchaser may direct the driver to add more water to make the concrete more fluid for easy filling of the forms.

MIX DESIGN

The "Recommended Practice for Selecting Proportions for Concrete" (ACI 613-54), the standard of the American Concrete Institute, was used as the design basis for all the mixes, with a slight modification for the mixes containing fly ash. Increased coarse-aggregate contents over those recommended by the ACI were again used in the fly-ash mixes in the amounts found to be satisfactory in previous studies using the same materials.

Three mixes were investigated, namely, 5.5 sacks of cement with no fly ash and 4.5 sacks of cement with, respectively, 100 and 200 pounds of fly ash per cubic yard of concrete. The cement and fly-ash contents were selected to yield approximately equal strengths between mixes for a given slump in the range most normally employed in construction. All mixes were designed originally for a slump of about four inches.

MATERIALS

The concrete materials were the same as those used and reported previously. The coarse aggregate was one-inch-maximum-size natural gravel, the fine aggregate was a natural sand having a fineness modulus of 3.0, the cement consisted of a blend of equal amounts of Huron, Peninsular, and Peerless brands, and the fly ash was from the St. Clair station.

Results of tests on all of these materials have been reported previously, the fly ash in report No. 2211-8-P and the cement in report No. 2211-10-P.

FABRICATION OF SPECIMENS AND TEST PROCEDURES

The methods of mixing, fabricating, curing, and testing were generally the same as used previously. The principal variation was in the addition of water at the mixer. Three consecutive batches were made from the same design, using the same weights of sand, gravel, cement, and fly ash. In the first batch only enough water was added to give a slump of about one inch. In the second batch, water was added to give a slump of about four inches. The third batch was made quite wet, with water being added until a slump of about eight inches was obtained. Special care was taken with the third batch to minimize segregation while molding the cylinders.

The sand was used in the moist condition in all but the last few batches of concrete made in this study. The moisture content was determined in advance of making the mixes and the batch weights were adjusted to compensate for the contained moisture.

Ten cylinders were made from each batch, and there were two repeat mixes for each cement and fly-ash combination, providing six cylinders for each age of 1, 7, 28, 90, and 365 days. These cylinders were moist cured until time for testing.

DISCUSSION OF TEST RESULTS

A detailed tabulation of mix proportions, results of tests on fresh concrete, and compressive strengths of test cylinders is presented in Tables I-A, II-A, and III-A in the Appendix. Summaries of important aspects of these results will be presented in the body of the report.

1. CEMENT CONTENT

The mixes were designed to contain 5.5 sacks of cement per cubic yard in the concrete with no fly ash and 4.5 sacks in concrete containing fly ash. Increasing or decreasing the water from the design quantity without making compensating changes in the quantities of the other materials results in over- or under-yield and the actual cement content will be too low or too high. The quantities of sand, gravel, and water reported are based on the nominal yield, no adjustment being made for the over- or under-yield.

2. COMPRESSIVE STRENGTH

Average values of compressive strength are given in Table I. For a given slump, differences in the strength of the three mixes are not large at ages up to 28 days. At 90 days there is somewhat greater variation, particularly in the mixes of medium consistency. At the later age, the mixes with fly ash have higher strengths than the comparable mixes without fly ash but containing more cement.

At the outset of this study it was conjectured that the fly-ash mixes would not vary in compressive strength with a variation in water as much as the mixes without fly ash. Comparison can be made by determining the loss of compressive strengths between the dry and wet consistency mixes for each of the cement and fly-ash contents. These values are shown in Table II.

It appears from Table II that the fly-ash mixes are not superior to

the plain-cement mixes in preventing loss of strength with the addition of water. The variations in strength losses are no more than would be expected from such a variable material as concrete.

It may be noted that the concrete with 200 pounds of fly ash required slightly less water for the low-slump mixes, and slightly more water for the high-slump mixes, than did the concrete with no fly ash. This difference is very small, slightly more than one gallon per cubic yard of concrete.

The conclusion from this study is that the concrete producer must take as much care in controlling the water added to fly-ash concrete as he does with plain portland-cement concrete unless he is willing to accept a sizeable loss in strength with the wet consistency mixes.

The lack of strength improvement for the low-slump, 4.5-sack mix containing 200 pounds of ash with respect to the corresponding medium-slump mix at 90 days' age is perplexing. This somewhat parallels the paradoxical behavior noted in Progress Report 2211-6-P wherein a diminished amount of pozzolonic strength gain was observed in the mixes richer in cement. It may be necessary to postulate for mixes having higher ash contents that a certain minimum water content may be required to insure that pozzolonic strength gain will continue.

TABLE I

SUMMARY OF RESULTS

Nominal Cement	Actual	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	 Net Miving	Miving Weter	ر الم			10 O	*; } }	
Content, sk/cu.yd	Content, sk/cu yd	lb/cu yd	()	gal/sk	in。	1 Day	7 Days	Days 28 Days 90 Days	90 Days	l Year
	5.47 5.40 5.36	000	234 259 284	5.11 5.65 6.20	0.8	1482 1114 877	3213 2656 2133	4178 3562 3254	5238 4538 4069	
	4.54 4.51 4.44	100	254 258 288	6.24 6.86 7.68	0.0.5	1170 908 698	2963 2513 1966	4223 3646 3188	5547 4933 4233	
	4.56 4.14 4.14	000 000 000	230 264 295	6.14 7.04 7.86	1.2	1151 853 619	2753 2235 1773	4243 4010 2972	5459 5464 4541	

TABLE II

LOSS IN COMPRESSIVE STRENGTH OF NON-AIR-ENTRAINED
PLAIN AND FLY-ASH CONCRETE DUE TO ADDED WATER

Cement Content,	Fly Ash Content,	1		ve Strength	
sk/cu yd	lb/cu yd	l Day	7 Days	28 Days	90 Days
5.5	0	605	1080	924	1169
4.5	100	472	997	1035	1314
4.5	200	532	980	1271	918

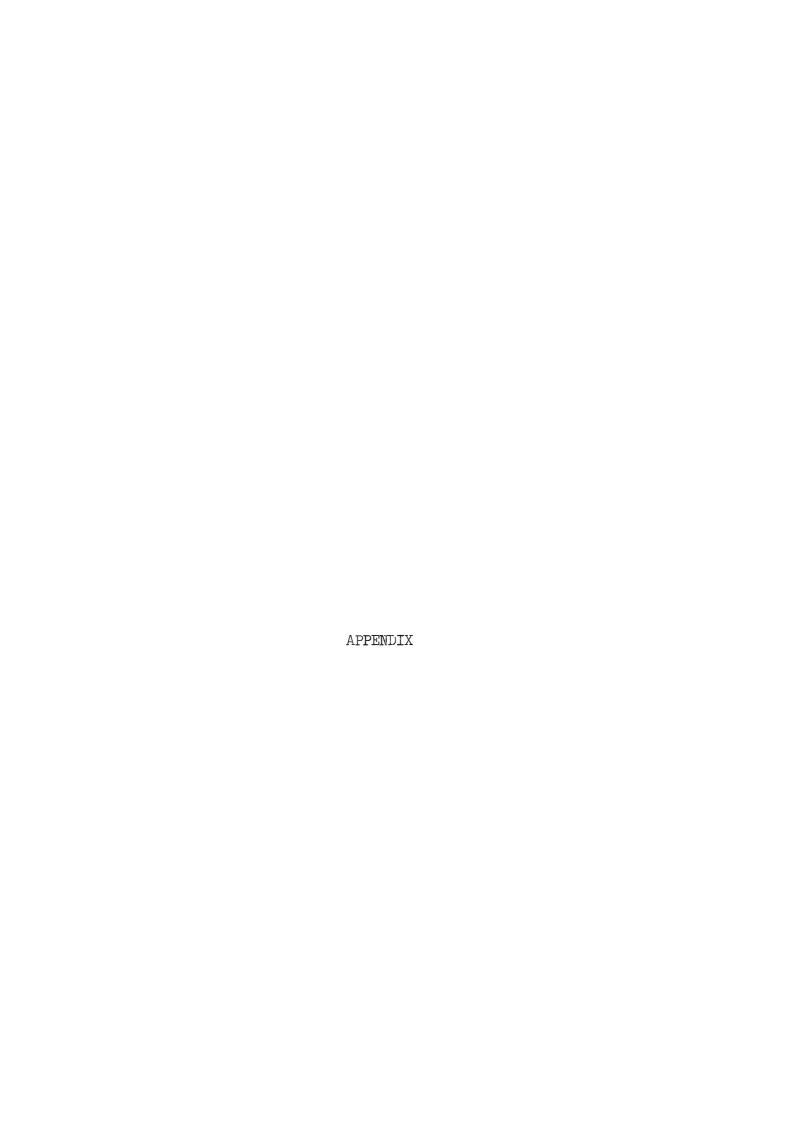


TABLE I-A

5.5-SACK REGULAR-CONCRETE DATA-LOW, MODERATE, AND HIGH SLUMPS

												-			
Batch			Actual Cement	*	Mate	Material Proportions	ortions	W/C,	Weight of Fresh	Slump,		Compress	ive Stre	Compressive Strength, psi	
No.	Date Made	lb/cu yd	נט	ທ >	Sand	Gravel M	Met Water	gal/sk	Concrete, lb/cu ft	in.	l Day	7 Days	28 Days	90 Days	l Year
371	3-28-56	0	5.47	0.64	1457	1815	228	4.97	149.8	1-1/4	1570	3425	7665	5740	
392	5-14-56	0	5.45	0.64	1481	1815	232	5.05	150.1	1/4	1285 1570	3250 2935	4575 3850	5565 5335	
, (, L	•	\ C	(\ I	1 (, (r (1580	5040	4275	5230	
990	2-SI-50	0	2.48	0.04	T##P	1815	245	5.30	150.1	7	1550 1535	3390	2 (10 3995	4940	
	Average	0	5.47	19.0	1461	1815	234	5.11	150.0	0.8	1482	3213	4178	5238	
372	3-28-56	0	5.46	0.64	1457	1815	243	5.31	150.0	5-1/4	975	2810	3920	0544	
											955	2630	3885	0644	
293	5-14-56	0	5.34	0.64	1481	1815	569	5.86	148.5	2-3/4	1140	2740	3250	4680	
											1125	2650	.3425	4700	
399	5-21-56	0	5.41	0.64	1446	1815	564	5.77	149.1	4-1/4	1235	2560	3620	74610	
											1255	2545	3270	4295	
	Average	0	2,40	0.64	1461	1815	259	5.65	149.2	4.1	1114	2656	3562	4558	
575	3-28-56	0	5.34	0.64	1457	1815	280	6.12	148.1	8-3/4	610	1855	3320	3815	
\ - \											999	1875	3305	4100	
394	5-14-56	0	5.34	0.64	14 81	1815	586	6.2 ⁴	149.0	7-1/4	925	2015	3215	4185	
											960	2155	3270	4115	
700	5-21-56	0	5.41	0.64	1446	1815	286	6.24	149.9	7-3/4	1000	2455	3445	4065	
											1090	2440	2970	4135	
	Average	0	5.36	0.64	1461	1815	- 182	6.20	149.0	7.9	877	2133	3254	690†	
									1						

**Denotes volume of dry-rodded coarse aggregate per unit volume of concrete.

TABLE II-A

4.5—SACK REGULAR-CONCRETE DATA—ST. CLAIR FLY ASH—LOW, MODERATE, AND HIGH SLUMPS

•	l Year												
ngth, ps.	90 Days	5370	5370 5865	5900 5405	5370 5547	0894	4560 4965	5055 5090	5245 4933	*ÒLL+1	4150	3940	4205 4233
Compressive Strength, psi	28 Days	4170	4275 4435	4415 3815	4225 4223	3535	3695 3655	3870 3695.	3425 3646	3550	3285 3320	3270 2825 <u>6</u>	2880 3188
Compres	7 Days	2970	3200 2845	3005 2 860	2895 2963	0448	2615 2510	2615 2510	2385 2513	2330	2330 1835	1875 1730	1695 1966
	1 Day	1185	1180	1075 1235	1245 1170	1020	955 820	920	875 908	830	760 550*	630	610 698
Slump	in.	3/4		႕	6.0	7	77	3-3/4	3.9	7-1/4	7-5/4	8-1/4	7.8
Weight of Fresh	Concrete, lb/cu ft	150.9	151.3	150.9	151.0	151.4	150.7	150.4	150.8	149.9	150.0	149.6	149.8
W/C,	gal/sk	6.71	5.97	6.03	6.24	7.30	24.9	6.82	98.9	7.91	7.34	7.79	7.68
ortions	Net Water	. 252	722	226	254	274	243	256	258	297	275	292	. 288
Material Proportions	Gravel	2126	2126	2126	2126	2126	2126	2126	2126	2126	2126	2126	2126
Mater	Sand	1132	1097	1108	1112	1132	1097	1108	1112	1132	1097	1108	1112
***	o.	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75
Actual Cement Content	sk/cu yd	64.4	4.57	4.55	45.4	84.4	4.54	4.50	4.51	4.41	4.48	4.44	4.44
	lb/cu yd	100	100	100	100	100	100	100	100	100	100	100	100
Date Made		4-2-56	4-18-56	5-9-56	Average	4-2-56	4-18-56	2-9-56	Average	4-2-56	4-18-56	5-9-56	Average
Batch	No.	374	380	289		375	381	390		376	382	391	

*Not included in average. **Denotes volume of dry-rodded coarse aggregate per unit volume of concrete.

TABLE III-A

4.5-SACK REGULAR-CONCRETE DATA-ST. CLAIR FLY ASH-LOW, MODERATE, AND HIGH SLUMPS

Batch	TO WORLD	Fly Ash,	Actual Cement	*	Mate	Material Proportions	rtions	W/C,	Weight of Fresh	Slump		Compress	sive Stre	Compressive Strength, psi	İ
No.	Dave Mane		sk/cu yd	ຜ	Sand	Gravel Ne	yu Net Water	gal/sk	Concrete, lb/cu ft	in.	1 Day	7 Days	28 Days	90 Days 1	Year
277	4-4-56	200	4.58	0.81	824	2296	219	5.85	151.1	1-1/4	1175	2685	0044	5300	
383	4-30-56	200	4.55	0.81	448	2296	243	6.47	151.8	1-1/2	1140 1085	2720 2880	4505 4150	5655 5740	
395	5-16-56	200	4.56	0.81	835	2296	229	0.10	151.2	3/4	1135	2825 2755	4100	5620 4945	
	Average	200	7.56	0.81	834	2296	230	6.14	151.4	1.2	1205 1151	2650 2753	4150 4243	5495 5459	
378	95-4-4	200	4.52	0.81	428	2296	254	6.78	150.6	3-3/4	795	2525*	0424	6005	
384	4-30-56	200	8t°t	0.81	448	2296	267	7.12	150.4	4-1/4	870 840	2210	4170 4010	5550 5530	
396	5-16-56	200	∠τ ,• τι	0.81	835	5296	271	7.22	149.7	4-1/4	8 8 8 7 6 7	2505 1945*	.4135 .2885 .785	5210 5230	
	Average	200	6ħ•ħ	0.81	854	2296	564	7.04	150.2	4.1	890	2190 22 35	3620 4010	5245 5464	
379	95-4-4	200	94.4	0.81	824	2296	284	7.56	149.4	œ	575	1625	2860	4550	
385	4-30-56	200	†† †	0.81	4448	2296	294	7.83	149.9	7-3/4	585 650	1855	26/0 3180	4415 4910	
397	5-16-56	200	4.41	0.81	835	2296	306	8.18	149.0	8-1/2	635	1875 1875	22'0 2740 3'50	00/4	
	Average	200	††' †	0.81	854	2296	295	7.86	149.4	8.1	619 619	1773	2110 2972	4400	
X															

^{*}Not included in average **Denotes volume of dry-rodded coarse aggregate per unit volume of concrete.