ARCH 324 - Structures 2, Winter 2009

von Buelow, Peter


<http://hdl.handle.net/2027.42/64938>
http://hdl.handle.net/2027.42/64938
Unless otherwise noted, the content of this course material is licensed under a Creative Commons Attribution 3.0 License. http://creativecommons.org/licenses/by/3.0/

© 2009, Peter Von Buelow

You assume all responsibility for use and potential liability associated with any use of the material. Material contains copyrighted content, used in accordance with U.S. law. Copyright holders of content included in this material should contact open.michigan@umich.edu with any questions, corrections, or clarifications regarding the use of content. The Regents of the University of Michigan do not license the use of third party content posted to this site unless such a license is specifically granted in connection with particular content. Users of content are responsible for their compliance with applicable law. Mention of specific products in this material solely represents the opinion of the speaker and does not represent an endorsement by the University of Michigan. For more information about how to cite these materials visit https://open.umich.edu/education/about/terms-of-use.

Any medical information in this material is intended to inform and educate and is not a tool for self-diagnosis or a replacement for medical evaluation, advice, diagnosis or treatment by a healthcare professional. You should speak to your physician or make an appointment to be seen if you have questions or concerns about this information or your medical condition. Viewer discretion is advised: Material may contain medical images that may be disturbing to some viewers.
Reactions:
\[ \Sigma M_{R_1} = 0 = 7.2(12) - R_2(24) + 6(28) \]
\[ R_2 = 10, 6 K \]
\[ \Sigma M_{R_2} = 0 = R_1(24) - 7.2(12) + 6(4) \]
\[ R_1 = 2, 6 K \]

Check: \[ \Sigma F_V = 10.6 + 2.6 - 7.2 - 6 = 0 \]

Shear:
- At 2.6 ft, \[ 11.267 \]
- At 8.67 ft, \[ -35.267 \]
- At 24 ft, \[ 6 K \]

Moment:
- At 4.88 ft, \[ 4.88 K' \]
- At 12 ft, \[ 12 K' \]
- At -24 K'

Design Moment = 24.0 K'
STRAIN COMPATIBILITY:

Assume wood controls strain:

\[ e = \frac{f}{E} = 1.5 \left( \frac{4}{12} \right)^2 = 0.0075 \]

\[ f = Ee = 30,000 \times (0.0075) = 225 \text{ psi} \]

Steel and wood have balanced strain as is.

Find moment carried by wood:

\[ M_{\text{wood}} = \frac{fS}{L} = 1.5 \left( \frac{4}{12} \right)^2 = 144 \text{ k}-\text{ft} \]

\[ = 12 \text{ k}-\text{ft} \]

Find moment carried by steel:

\[ M_{\text{steel}} = M_{\text{total}} - M_{\text{wood}} = 24 - 12 = 12 \text{ k}-\text{ft} \]

Find b of steel plates (two plates):

\[ M_b = \frac{fS}{2} = f \left( \frac{b(12)^2}{6} \right) \times 2 = 225 \left( \frac{b(12)^2}{6} \right) \times 2 = 12 \times 12 \]

\[ b = 1.33'' \]

Find length of plates (see moment diagram):

Wood will carry 12 k-ft alone, therefore find portion of moment diagram = 12 k-ft.

On cantilever:

\[ 6 \times l_1 = 12 \quad l_1 = 2 \\ \]

Between supports:

\[ 12 = l_2 (4.6 - 3.2) + l_2 (3.6) \]

\[ 0 = -0.15l_2^2 + 4.6l_2 - 12 \]

\[ l_2 = -4.6 \pm \sqrt{4.6^2 - 4(15)12} \]

\[ = 2.879' \]

\[ L_{\text{total}} = l_1 + l_2 \]

\[ = 2 + 2.88 = 4.88' \]