ARCH 324 - Structures 2, Winter 2009

von Buelow, Peter

<http://hdl.handle.net/2027.42/64938>
http://hdl.handle.net/2027.42/64938
Unless otherwise noted, the content of this course material is licensed under a Creative Commons Attribution 3.0 License. http://creativecommons.org/licenses/by/3.0/

© 2009, Peter Von Buelow

You assume all responsibility for use and potential liability associated with any use of the material. Material contains copyrighted content, used in accordance with U.S. law. Copyright holders of content included in this material should contact open.michigan@umich.edu with any questions, corrections, or clarifications regarding the use of content. The Regents of the University of Michigan do not license the use of third party content posted to this site unless such a license is specifically granted in connection with particular content. Users of content are responsible for their compliance with applicable law. Mention of specific products in this material solely represents the opinion of the speaker and does not represent an endorsement by the University of Michigan. For more information about how to cite these materials visit https://open.umich.edu/education/about/terms-of-use.

Any medical information in this material is intended to inform and educate and is not a tool for self-diagnosis or a replacement for medical evaluation, advice, diagnosis or treatment by a healthcare professional. You should speak to your physician or make an appointment to be seen if you have questions or concerns about this information or your medical condition. Viewer discretion is advised: Material may contain medical images that may be disturbing to some viewers.
FRAMING PLAN

D.L + L.L = 200 psf

EFFECTIVE WIDTH = 90"

SECTION

n = \frac{1}{4} \left[ \frac{E_c}{E_s} \right]

f_{steel} = 24 ksi

f_{conc} = 1.35 ksi

LOADING DIAGRAM

W = 156k

W = (200 \text{ psf}) \times (13') = 2600 \text{ psf}

\Rightarrow W = 156k
Determine the most economical section (W-shape) to carry load without composite action.

For a simply supported uniformly loaded beam,

Max. Bending Moment,

\[ M = \frac{Wl}{8} = \frac{156^k \times 60'}{8} \]

\[ M = 1170^k \text{kip-ft} \]

Thus,

\[ S = \frac{M}{f} = \frac{1170^k \times 12''}{24 \text{ ksi}} \]

\[ S = 585 \text{ in}^3 \]

From Table D-35, for \( S_x = 585 \text{ in}^3 \), sections appropriate are,

- W 30 x 191 598 in\(^3\)
- W 33 x 201 684 in\(^3\)
- \( \rightarrow \) W 36 x 182\( ^* \) 623 in\(^3\)

Thus 'W 36 x 182' is used.

3) Transformed section

\[ h = \frac{1}{q} = \frac{E_c}{E_S} \]

```
\[ \therefore \text{Transforming the concrete to an equivalent area of steel by reducing the width we get,} \]
```
4. LOCATION OF NEUTRAL AXIS
(USING D-28)

\[ \bar{x} = \frac{EAx}{EA} \]

<table>
<thead>
<tr>
<th>A</th>
<th>( \bar{x} )</th>
</tr>
</thead>
<tbody>
<tr>
<td>50 in(^2)</td>
<td>2.5 in = 125 in(^3)</td>
</tr>
</tbody>
</table>

\[ \bar{x} = \frac{EAx}{EA} = \frac{1029 \cdot 1675}{89.7} \]

\[ \bar{x} = 11.47" \] (from top)

5. TRANSFORMED MOMENT OF INERTIA
(By Parallel Axis Theorem)

\[ I_{TR} = I + Ad^2 \]

<table>
<thead>
<tr>
<th>( d )</th>
<th>( I )</th>
<th>( Ad^2 )</th>
</tr>
</thead>
<tbody>
<tr>
<td>( 10 ) in</td>
<td>7800</td>
<td>4023</td>
</tr>
</tbody>
</table>

\[ I_T = I_g + A_d^2 \]

\[ I_g = 104.17 + 4023 = 4127.17 \]

\[ I_T = 7800 + 5073.78 = 12873.78 \]

\[ I_{TR} = 17000.99 \]
\[ M_c = \frac{f_c I_{TR}}{C_n} \]

\[ M_c = \frac{1.35 \times (17001)}{11.47 (1/9)} = 1800.8 \text{ k}^{-\text{in}} \]

\[ M_s = \frac{f_s I_{TR}}{C} \]

\[ M_s = \frac{24 \times (17001)}{29.08} = 14031.08 \text{ k}^{-\text{in}} \]

\[ f_s = \frac{24 \text{ ksi}}{1} \]

\[ f_c = \frac{M_c C_n}{I_{TR}} = \frac{14031.08 (11.47)^{1/9}}{17001} \]

\[ f_c = 1.052 \text{ ksi} \]